

## RFP # 17-019P **DESIGN - BUILD FREESTANDING STEEL BRIDGE CRANE** AND RELATED SERVICES **ADDENDUM No. 1** May 2, 2017

#### A. Clarifications

- Due date for Proposals is being extended to May 12, 2017 at 2:00 P.M.
- Shortlist Evaluation Meeting has been rescheduled to May 22, 2017 at 1:00 5:00 P.M.
- Supplier Presentation/Interview has been rescheduled to May 31, 2017 (Tentative)
- 4. Section 1.3, Item G (Power Volts): Requirements for Power volts will now accept single phase. (Section 1.8 systems options is correct)
- 5. Please see the attached links to directories providing Certified DBEs.
  - a. http://www3b.dot.state.fl.us/EqualOpportunityOfficeBusinessDirectory/CustomSearch.aspx
  - b. <a href="https://osd.dms.myflorida.com/directories">https://osd.dms.myflorida.com/directories</a>
  - c. https://tampa.diversitysoftware.com/?TN=tampa
- 6. Documents attached to this Addendum:
  - a. Dimension breakdown for Free Standing Bridge Crane (1 page)
  - b. Structural Drawings Maintenance (29 pages)
  - c. Maintenance Architectural Drawing MA0.00- MA1.13 (6 pages)
  - d. Maintenance Architectural Drawing MA2.01C- MA3.03 (7 pages)

#### **B.** Questions and Answers

Question: Can 3 phase power of some sort be made available to the area? If so, what type? Ex. 230-3-60, 460-3-1. 60, etc...

Response: Both 208 volt 3 phase and 480 volt 3 phase power can be provided. The panels serving this area have approximately 3 to 5 hp capacity. If a larger motor or feeder is required, a feeder from a panel further away can be provided.

2. Question: We would need to be released from the requirement on page twelve (12) item H. The Proposer must provide a 100% Performance Bond and a 50% Payment Bond.

Response: Please reference page 104 of the RFP document for Bonding requirements exceeding \$100,000.

Bonding for Construction Activities Exceeding \$100,000	% bid guarantee bond.  00% performance bond.  ayment bond equal to:  50% for contracts < \$1M.  40% for contracts > \$1M - < \$5M.  \$2.5M for contracts > \$5M.	§ 15.o(1)
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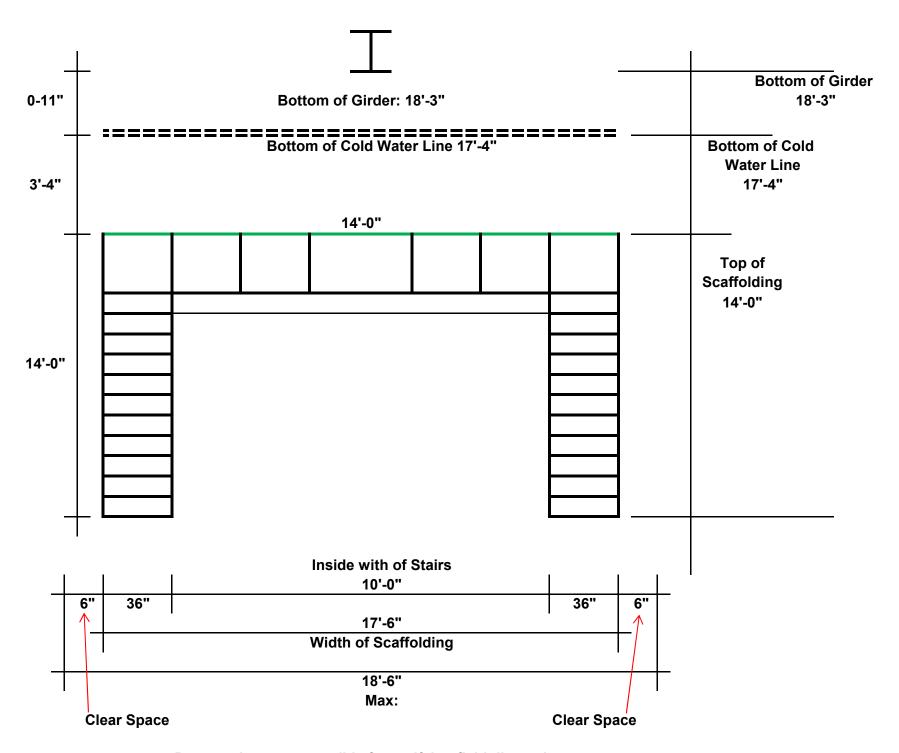
## All other Proposal terms and conditions originally issued remain unchanged.

REMINDER: Make sure you mark "Addendum No. 1" on Attachment "1" Acknowledgement of Addendum and remember to sign and return Acknowledgement Addendum form with your submittal package. Failure to do so may result in the disqualification of your proposal submittal.

The RFP is revised to the extent specifically amended by this Addendum #1. Otherwise, all provisions of the RFP remain in effect.

Eric L. Haubner Purchasing Agent II **Pinellas Suncoast Transit Authority** EHaubner@psta.net

# Dimension breakdown for Free Standing Bridge Crane



Proposer's are responsible for verifying field dimensions.

# Structural Drawings Maintenance



# **Pinellas Suncoast Transit Authority**



Administration Building: 3201 Scherer Drive Maintenance Building: 3101 Scherer Drive

Pinellas County, Florida Pinellas County, Florida

Volume 3 Maintenance Building







RECORD DOCUMENTS

THIS RECORD DOCUMENT HAS BEEN PREPARED BASED ON INFORMATION PROVIDED BY OTHERS. THE A/E HAS NOT VERIFIED THE ACCURACY AND/OR COMPLETENESS OF

SHEET CONTENT
MAINTENANCE BUILDING
PROJECT COVER SHEET

G0.03



#### LOCATION MAP:



SECTION 11 TOWNSHIP 30S RANGE 16E PINELLAS COUNTY, FLORIDA

#### PROJECT TEAM:

Architecture Mechanical Electrical Plumbing Fire Protection Interior Design



11757 Katy Freeway, Suite 600 Houston, Texas 77079 T: 281.558.7273 F: 281.558.7282

1831 Chestnut Street Saint Louis, Missouri 63103 T: 314.421.1476 F: 314.421.5664

BRINDLEY PIETERS & ASSOCIATES, INC.



401 CenterPointe Circle, Suite 1501 Altamonte Springs, Florida 32701 T: 407.830.8700 F: 407.830.8877



380 Park Place Blvd., Suite 300 Clearwater, Florida 33759 T: 800.861.8314 F: 727.539.1294

**RECORD DOCUMENTS** 

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NOTE: STRUCTURAL GENERAL NOTES & DETAILS ARE DUPLICATED IN VOLUMES 2, 3 AND 4. SOME DETAILS MAY NOT APPLY TO ALL VOLUMES.







V DESCRIPTION

PSTA
coast Transit Authority
Scherer Drive
s County, Florida 33771

RECORD DOCUMENTS

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DATE OF SEAL

SHEET CONTENT
PROJECT
INDEX TO DRAWINGS

DATE
SCALE
NO
DRAWN BY
CHECKED BY

PB PROJECT NUMBER

G1.10

DESIGN SUPERIMPOSED LOADS: LIVE LOAD DEAD LOAD MAINT. BLDG "B" STORAGE. 125 PSF CORRIDORS . 80 PSF MAINTENANCE MECH. PLATFORM, AT MAINTEN. BAT. 150 PSF TERRACE, ROOF [1], BRIVERS ROOM. 100 PSF ROOF TERRACE AT 2ND FLOOR MAINTEN. POD 100 PSF MECHANICAL PENTH. AT MAINTEN. POD "B". 150 PSF FOR SPECIFIC MECHANICAL & OTHER EQUIPMENT LOADS, SEE DWGS. DESIGN SUPERIMPOSED DEAD LOADS LISTED ABOVE DO NOT INCLUDE MASONRY OR STUD WALLS OR OTHER CONCENTRATED LOADS. SEE ARCHITECTURAL DRAWINGS & MEP DWGS. FOR THESE LOADS. OFFICES INCLUDE 20 PSF ALLOWANCE FOR PARTITIONS DESIGN WIND LOADS
GOVERNING CODE .....FLORIDA BUILDING CODE 2001.......
BASIC WIND SPEED (3-SECOND GUST)..... .....V = 123 MPH BUILDING CATEGORI (1982)
IMPORTANCE FACTOR BUILDING CATEGORY (ASCE 7-98 TABLE 1-1) ..... .... | = 1.0 ......B ......33 FEET ......Kd =0.85 .....Kzt=1.0 ......GCp: = 0.18

#### ADJACENT BUILDINGS AND PROPERTY

THE CONTRACTOR SHALL PROTECT ADJACENT PROPERTY, HIS OWN WORK AND THE PUBLIC FROM HARM. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR CONSTRUCTION MEANS AND METHODS, AND JOBSITE SAFETY INCLUDING ALL OSHA REQUIREMENTS. THE CONTRACTOR SHALL ENSURE THAT ALL CONSTRUCTION METHODS USED WILL NOT CAUSE DAMAGE TO THE ADJACENT BUILDINGS AND PROPERTY. THIS SHALL INCLUDE ALL FOUNDATION INSTALLATION, THE CONTRACTOR IS ADVISED TO PERFORM ALL PHOTOGRAPHIC SURVEYS AND

....GCp: = 0.55

OTHER DOCUMENTATION OF THE ADJACENT BUILDINGS BEFORE THE START OF

#### REINFORCED CONCRETE (REF. SECTION 03300)

COMPLY WITH ACI 301 AND 318 AND SPECIFICATION SECTIONS
PROVIDE STRUCTURAL CONCRETE WITH A MINIMUM ULTIMATE COMPRESSIVE DESIGN STRENGTH fc' . IN 28 DAYS AS FOLLOWS:

FOOTINGS, PIT SLAB 3000 PSI 4000 PSI 3000 PSI 4000 PSI SLABS ON GRADE AT MAINTEN/OPERATIONS TYPICAL SLAB ON DECK ELEVATED SLAB TILT UP WALLS 5000 PSI ELEVATED SLAB, OVER MAINTEN. PIT
C.I.P. COLUMNS, BEAMS, WALLS
USE NORMAL WEIGHT CONCRETE 150PCF FOR ALL STRUCTURAL

MEMBERS WITH CARBONATE OR SILICEOUS AGGREGATE, U.N.O..

PROVIDE ASTM A-615 GRADE 60 REINFORCING STEEL. REINFORCING SHALL BE
ACCURATELY PLACED, RIGIDLY SUPPORTED AND FIRMLY TIED IN PLACE, WITH APPROPRIATE BAR SUPPORTS AND SPACERS. LAP CONT. REINE, AS NOTED IN LAF SPLICE SCHEDULE. LAP BOTTOM STEEL OVER SUPPORTS AND TOP STEEL AT MIDSPAN (U.O.N.), HOOK DISCONTINUOUS ENDS OF ALL TOP BARS AND ALL BARS IN WALLS. U.O.N. PROVIDE COVER OVER REINFORCING AS FOLLOWS

TOP FOOTINGS, FOUND WALLS .3" CONCRETE PIERS SLABS ON GRADE SLABS ABOVE GRADE 1 1/2" 2" SLABS EXPOSED TO WEATHER 1 1/2" 1 1/2" WALLS RETAINING FILL WALLS; TILT WALLS ABOVE GRADE 1 1/2" 1 1/2" 1 1/2" COLUMNS

WHERE SPECIFIED, PROVIDE PLAIN, COLD-DRAWN ELECTRICALLY-WELDED STEEL FABRIC CONFORMING TO ASTM A-185. SUPPLY IN FLAT SHEETS ONLY, LAP SPLICE ONE CROSS WIRE SPACING PLUS TWO INCHES

PROVIDE THE FOLLOWING TEMPERATURE REINF. FOR ONE-WAY SLABS, U.O.N.: SLAB THICKNESS REINFORCING

3 1/2" - 4 1/4" 4 1/2" - 6 1/2" #3 @ 12" 0/C #4 @ 18" 0/C 7"- 7 1/2" 8" - 9" #4 © 15" O/C #4 © 12" O/C

IN ADDITION TO SPECIFIED REINFORCING PROVIDE 5 TONS OF REINFORCING BARS TO BE DETAILED, FABRICATED, DELIVERED TO SITE AND PLACED AS DIRECTED BY THE ARCHITECT/ENGINEER TO ACCOUNT FOR UNFORESEEABLE CONDITIONS.

#### REINFORCED CONCRETE (CONT)

UTILITIES SHALL NOT PENETRATE BEAMS OR COLUMNS BUT MAY PASS THROUGH SLABS AND WALLS INDIVIDUALLY, U.O.N. FOR OPENINGS 24" LONG OR LESS, CUT REINE AND REPLACE ALONGSIDE OPENING WITH SPLICE BARS OF FOLIVALENT AREA WITH 48 BAR. DIA. LAP. PREPARE AND SUBMIT SHOP DRAWINGS FOR OPENINGS LONGER THAN 24". FOR RECTANGULAR OPENINGS 12" LONG OR LONGER, ADD 1#5 x 6' MID DEPTH DIAGONAL AT ALL 4 CORNERS.

WHERE REINFORCING STEEL CONGESTION PERMITS, CONDUIT AND PIPES UP TO 1 WHITE REINFORCING SIELE CONGESTION FERMINS, CONDOIT AND FIRES OF TO FORM THE STATE OF THE STATE BE REQUIRED. REQUESTS TO EMBED LARGER PIPES SHOULD BE ACCOMPANIED BY A

DETAILED DESCRIPTION AND BE SUBMITTED TO THE ARCHITECT FOR EVALUATION PROVIDE CONSTRUCTION JOINTS IN ACCORDANCE WITH ACI 318, SECTION 6.4. PROVIDE KEYWAYS AND ADEQUATE DOWELS. SUBMIT DRAWINGS SHOWING LOCATION OF CONSTRUCTION JOINTS AND DIRECTION OF POUR FOR REVIEW.

PROVIDE 3/4" CHAMFER FOR ALL EXPOSED CORNERS. IN WALLS, GRADE AND TIE BEAMS PROVIDE CORNER BARS TO MATCH HORIZONTAL

WALL BARS AT ALL CORNERS.

PROVIDE REINFORCING STEEL PLACER WITH A SET OF STRUCTURAL DRAWINGS FOR FIELD REFERENCE. INSPECT REINFORCING STEEL PLACING FROM STRUCTURAL

COORDINATE EXACT SIZES AND LOCATIONS OF HOUSEKEEPING PADS, SUMP PITS TRENCH DRAINS, SLEEVES AND EMBEDS WITH OTHER TRADES AND WITH ARCH/MEP DWGS.

USE WEDGE-TYPE EXPANSION ANCHORS SUCH AS THE HILTI KWIK BOLT II, ITW RAMSET RED HEAD TRUBOLT WEDGE, POWERS RAWL POWER-STUD, OR ACCEPTED EQUIVALENT. FOLLOW MANUFACTURER'S SPECIFICATIONS FOR USE AND INSTALLATION.

CONFIRM THE ABSENCE OF REINFORCING STEEL BY DRILLING A 1/4" DIAMETER PILOT HOLE FOR EACH ANCHOR. DO NOT CUT REINFORCING STEEL WITHOUT APPROVAL OF THE STRUCTURAL ENGINEER.

PROVIDE ANCHOR EMBEDMENT, SPACING AND EDGE DISTANCE AS SHOWN ON THE

#### SHALLOW FOUNDATIONS

GEOTECHNICAL INVESTIGATION REPORT, PROPOSED BUSINESS ADMINISTRATION,
OPERATIONS, & MAINTENANCE FACILITY, ST. PETERSBURG, FLORIDA, PROJECT NO.
T-02-2003 BY MC SQUARED, INC., DATED JULY 2002 AND ANY SUBSEQUENT AMENDMENTS. THE CONSTRUCTION MANAGER SHOULD OBTAIN A COPY OF THE REPORT AND IT'S AMENDMENTS AND FOLLOW ALL RECOMMENDATIONS WITHIN

PARTICULARLY DURING FOUNDATION & SLAB ON GRADE CONSTRUCTION.
FOOTING SIZES AND REINFORCING ARE BASED ON AN ASSUMED ALLOWABLE SOIL BEARING CAPACITY OF MIN. 2500 PSF. AT A BEARING ELEVATION OF 5' BELOW

EXTERIOR GRADE ON COMPACTED SOIL AS SPECIFIED IN THE GEOTECH REPORT.

CENTER ALL FOOTINGS UNDER THEIR RESPECTIVE COLUMNS OR WALLS, U.O.N.

TOP OF ALL FOOTINGS IS A MINIMUM OF 18" BELOW THE GRD. FLOOR LEVEL, U.O.N.

SIDES OF THE FOOTINGS SHALL BE FORMED. EXCAVATED SIDES TO FORM THE FOOTINGS IS NOT ACCEPTABLE.

#### EXCAVATION, BACKFILL AND DEWATERING

THE CONTRACTOR IS SOLELY RESPONSIBLE FOR ALL EXCAVATION PROCEDURES INCLUDING LAGGING, SHORING, AND PROTECTION OF ADJACENT PROPERTY, STRUCTURES, STREETS AND UTILITIES IN ACCORDANCE WITH THE REQUIREMENTS OF THE LOCAL BUILDING DEPARTMENT AND OSHA REGULATIONS. DO NOT EXCAVATE WITHIN ONE FOOT OF THE ANGLE OF REPOSE OF ANY SOIL BEARING FOUNDATION UNLESS THE FOUNDATION IS PROPERLY PROTECTED AGAINST SETTLEMENT.

2. DO NOT BACKFILL AGAINST WALLS UNTIL 7 DAYS AFTER THE WALLS ARE BRACED BY

THE STRUCTURE OR ARE TEMPORARILY BRACED. DO NOT BACKFILL CANTILEVERED RETAINING WALLS UNITL CONCRETE IS 14 DAYS OLD. DO NOT BACKFILL UNTIL AFTER COMPLETION AND INSPECTION OF ANY WATERPROOFING.

3. THE CONTRACTOR IS RESPONSIBLE FOR THE DISPOSAL OF ALL ACCUMULATED WATER

IN A MANNER THAT DOES NOT INCONVENIENCE OR DAMAGE THE WORK.

GROUND WATER: THE CONTRACTOR'S ATTENTION IS DRAWN TO GROUND WATER ELEVATIONS DISCUSSED IN THE GEOTECHNICAL REPORT. TEMPORARY WELL POINTS WILL BE NEEDED SO THAT THE GROUND WATER LEVEL IS MAINTAINED AT LEAST TWO (2) FEET BELOW BOTTOM OF FOOTINGS DURING COMPACTION AND CONSTRUCTION AT DEEP EXCAVATIONS.

#### SLABS ON GRADE

REFER TO GEOTECHNICAL REPORT FOR SUBGRADE PREPARATION MORE THAN 12" BELOW BOTTOM OF SLAB.

2. ABOVE SUBGRADE, USE FILL CONTAINING NOT MORE THAN 10% PASSING #200 SIEVE AND MAXIMUM 1 INCH DIAMETER. COMPACT TO 95% OF MAXIMUM DRY DENSITY AS DETERMINED BY MODIFIED PROCTOR ASTM D-1557. EACH LAYER OF FILL SHALL NOT

EXCEED 6" LOOSE THICKNESS. COMPACT PRIOR TO PLACEMENT OF THE NEXT LAYER. FILL PLACEMENT AND COMPACTION SHALL BE MONITORED AND ACCEPTED BY THE TESTING AGENCY. TAKE A MIN. OF ONE FIELD DENSITY TEST (ASTM D-1556 OR D-2922) FOR EACH 2,500 SQUARE FEET OF EACH LAYER. THE TESTING AGENCY SHALL RANDOMLY SELECT TEST LOCATIONS.

A. FOR INTERIOR SLABS PLACE 8 MIL POLYETHYLENE SHEETING BETWEEN SOIL AND BOTTOM OF SLAB. DO NOT USE ANY SHEETING BELOW EXTERIOR CONCRETE SLABS.

5. SLAB THICKNESS AND REINFORCING ARE; U.N.O. ON PLANS, AS FOLLOWS: ARFA THICKNESS REINFORCING

6X6 W2.9XW2.9 TOP AND BOT. PARKING AREAS ELECTRICAL AND MECHANICAL ROOMS 6X6W2.9XW2.9 TOP AND BOT STORAGE ROOMS, SERVICE CORRIDORS 6X6 W2.9XW2.9 TOP AND BOT. ELEVATOR LORRY 6X6 W2.9XW2.9 TOP TYPICAL & OFFICE 6X6 W2.0XW2.0 TOP 6X6 W2.9XW2.9 TOP AND BOT. OPERATIONS BLDG "B" MAINTENANCE BLDG "A&C" REFER TO DWGS. FUEL\WASH BAYS 8" #4@12" TOP AND BOT.
PLACE CONCRETE IN LONG-STRIP CONSTRUCTION METHOD. PROVIDE CRACK CONTROL JOINTS AT 21'-4" FEET MAX. TO LIMIT AREAS BETWEEN JOINTS TO 400 SQ. FT. IN ALL FLOATING SLABS ON GRADE. LOCATE TO CONFORM TO BAY SPACING WHENEVER POSSIBLE, ADD CRACK CONTROL JOINTS AT RE-ENTRANT CORNERS WHICH TEND TO INVITE CRACKS.

IN SIDEWALKS AND WALKWAYS, LOCATE ISOLATION JOINTS AT 20 FT. O.C. MAX. SCORE AND TOOL BETWEEN ISOLATION JOINTS IN EQUAL BAYS OF 5 FT. OR LESS.

SEE THE ARCHITECTURAL/MEP DRAWINGS FOR SLAB ON GRADE, SIZE & LOCATION OF DEPRESSIONS, SLOPE, TRENCH DRAINS, SUMPS, HOUSEKEEPING PADS AND OTHER

#### CONCRETE MASONRY

CONSTRUCT MASONRY IN ACCORDANCE WITH SPECIFICATION SECTIONS/04810 AND 04230; ACI 530/ASCE 5, "BUILDING CODE REQUIREMENTS FOR CONCRETE MASONRY STRUCTURES"; AND ACI 530.1/ASCE 6, "SPECIFICATIONS FOR THE DESIGN AND CONSTRUCTION OF LOAD-BEARING CONCRETE MASONRY".

THE STRUCTURE CONSISTS OF A SKELETON FRAME. FRECT MASONRY AFTER STRUCTURAL FRAMING SUPPORTING THE LEVEL ABOVE IS IN PLACE. SECURE
MASONRY TO COLUMNS WITH GALVANIZED DOVETAIL ANCHORS (OR APPROVED EQUAL) IN EVERY SECOND COURSE. DO NOT POUR TIE COLUMNS UNTIL ALL SHORING AND RE-SHORING IN THAT STORY HAS BEEN REMOVED.

USE 50% SOLID, NOMINAL 8X8X16, CONCRETE MASONRY UNITS CONFORMING TO ASTM

C90. LAY UP UNITS IN RUNNING BOND. SAWCUT UNITS WHICH ARE NOT IN MULTIPLES OF 8". UNITS SHALL BE AT LEAST 8" LONG. BOND CORNERS BY LAPPING ENDS 8" IN SUCCESSIVE VERTICAL COURSES. DESIGN OF WALLS IS BASED ON A f m' OF 1500 PSI.

USE TYPE S MORTAR IN ACCORDANCE WITH ASTM C270 EXCEPT USE TYPE M OSE TIPE S MORTAR IN ACCURDANCE WITH ASIM CZ/U EXCEPT USE TIPE M MORTAR BELOW GRADE. HEAD AND BED JOINTS SHALL BE 3/8" FOR THE THICKNESS OF THE FACE SHELL. WEBS ARE TO BE FULLY MORTARED IN ALL COURSES OF PIERS, COLUMNS AND PILASTERS; IN THE STARTING COURSE; AND WHERE AN ADJACENT CELL IS TO BE GROUTED. REMOVE MORTAR PROTRUSIONS EXTENDING 1/2" OR MORE INTO CELLS TO BE GROUTED.

USE STANDARD (9 GAUGE) HORIZONTAL JOINT REINFORCING CONFORMING TO ASTM A-82 IN EVERY OTHER COURSE. OVERLAP DISCONTINUOUS ENDS 6". USE PREFABRICATED CORNERS AND TEES. USE TRUSS TYPE, EXCEPT USE LADDER TYPE FOR ALL SINGLE WYTHE IN WALLS WITH VERTICAL REINFORCING.
USE FINE GROUT CONFORMING TO ASTM C-476, WITH A MINIMUM COMPRESSIVE

STRENGTH OF 2500 PSI IN 28 DAYS. AGGREGATE TO CONFORM TO ASTM C404 FOR FINE GROUT, WITH SLUMP OF 8" TO 10". GROUT ALL MASONRY CONTAINING REINFORCING, ALL CELLS OF 4 HOUR RATED WALLS, AND WHERE INDICATED ON THE DRAWINGS. ALLOW MORTAR TO CURE 24 HOURS PRIOR TO GROUTING. PROVIDE CLEANOUT OPENINGS AT THE BASE OF CELLS CONTAINING REINFORCING STEEL TO CLEAN THE CELL AND TO TIE THE VERTICAL BAR TO THE DOWEL. IN HIGH-LIFT GROUTING, USE 5'-0" (MAX.) LIFTS, WITH 1/2 HOUR TO 1 HOUR BETWEEN LIFTS.
WIBRATE EACH LIFT AND RECONSOLIDATE THE PREVIOUS LIFT.
USE ASTM A-615 GRADE 60 REINFORCING STEEL. REINFORCE WALLS WHERE

INDICATED ON THE DRAWINGS AND 1#5 VERTICAL AT ALL INTERSECTIONS, EACH SIDE OF OPENINGS AND AT THE ENDS OF WALLS. USE BAR SPACERS © 10 FT. O.C.

WHERE GROUT POUR HEIGHT EXCEEDS 10'.
AT BOND BEAM CORNERS AND INTERSECTIONS, PLACE 1 #5 x 5' T & B CORNER BAR, WITH 30" LEGS EACH WAY, AT THE EXTERIOR FACE.
REINFORCED MASONRY WALL CONSTRUCTION SHALL BE INSPECTED BY AN ENGINEER

OR ARCHITECT IN ACCORDANCE WITH ACI 530.1/ASCE6.
WHERE ANCHOR BOLTS, WEDGE ANCHORS OR ANCHORS SET IN EPOXY ARE SET IN A
MASONRY WALL, FILL CELLS WITH GROUT FOR BOLTED COURSE, ONE COURSE

ABOVE AND TWO COURSES BELOW. PROVIDE LINTELS OR HEADERS WITH MIN. 8" BEARING OVER ALL MASONRY OPENINGS.

USE PRESSURE-TREATED WOOD FOR WOOD IN CONTACT WITH MASONRY. CONTROL JOINTS SHALL BE PROVIDE IN ALL CONCRETE MASONRY CONSTRUCTION AT LOCATIONS INDICATED ON THE ARCHITECTURAL DRAWINGS AND SPECIFICATIONS. THE SPACING OF CONTROL JOINTS SHALL NOT EXCEED 20'-0".

HORIZONTAL WALL REINFORCING SHALL BE STOPPED EACH SIDE OF CONTROL JOINTS.
SEE ARCHITECTURAL DRAWINGS FOR SEALANT REQUIREMENTS AT CONTROL JOINTS.

REINFORCE MASONRY WALLS WITH #5@48" MAX VERTICAL (U.N.O.). PROVIDE CONTINUOUS HORIZONTAL BOND BEAM COURSE W/2 #5 HORIZONTAL EVERY 10'-O" OF HEIGHT AND AT TOP OF WALL. ALL LAPS 48 BAR DIAMETERS. PROVIDE DOWELS TO MATCH WALL REINFORCEMENT.

STEEL ROOF DECK (REF. SECTION 05300)

1. ROOF DECK SHALL BE AS FOLLOWS:

A. ALL METAL ROOF DECK EXCEPT WHERE SHOWN ON PLAN SHALL BE 1 1/2" DEEP

A. ALL METAL ROOF DECK EXCEPT WHERE SHOWN ON PLAN SHALL BE 1 1/2" DEEP 22 GAGE WIDE RIB, MIN. PROPERTIES:

Sp. 0.186IN Sn=.192 IN , I=0.189IN., GALVANIZED G90.

B. ROOF DECK SHALL BE PLACED IN AT LEAST TWO SPAN SEGMENTS. NO SINGLE SPAN CONDITIONS SHALL BE USED.

C. STEEL DECK SHALL CONFORM TO ASTM A446 GRADES A, B, C, D, E OR F FOR GALVANIZED DECK, MINIMUM YIELD STRENGTH OF 33,000 PSI.

D. STEEL DECK SHALL BE GALVANIZED WITH A PROTECTIVE ZINC COATING CONFORMING TO ASTM A525 G90 CLASS.

1. WELDING:

a) ROOF DECK UNITS SHALL BE WELDED TO EACH STRUCTURAL SUPPORT ROUF DECK UNITS SHALL BY WELDED TO EACH STRUCTURAL SUPPORT MEMBER USING 5/8" DIAMETER PUDDLE WELDS AT ALL RIBS (36/7 FASTENER LAYOUT). WELD METAL SHALL PENETRATE ALL LAYERS OF DECK MATERIAL AT END LAPS AND SIDE JOINTS AND SHALL BE COMPLETELY FUSED TO THE SUPPORTING MEMBERS.

COMPLETELY FUSED TO THE SUPPORTING MEMBERS.

b) SIDE LAPS OF ADJACENT UNITS SHALL BE FASTENED BY WELDING (ON 18 GAUGE OR HEAVIER DECK ONLY) OR SHEET METAL SCREWS, SO THAT SPACING BETWEEN FASTENERS AND BETWEEN THE FIRST FASTENER AND SUPPORT DOES NOT EXCEED IS INCHES.

c) AT ALL ROOF OPENINGS AND PERIMETER ROOF EDGE CONDITIONS, ROOF

c) AT ALL ROOF OPENINGS AND PERIMETER ROOF EDGE CONDITIONS, RC
DECK UNITS SHALL BE WELDED TO EDGE STEEL USING 5/8"DIAMETER
PUDDLE WELDS AT 6" O.C.

2. PROVIDE A MINIMUM END BEARING OF 2" OVER SUPPORTS.

3. END LAPS OF SHEETS SHALL BE A MINIMUM OF TWO INCHES AND SHALL OCCUR
OVER SUPPORTS. ROOFS SHALL BE ERECTED BEGINNING AT THE LOW SIDE TO
INSURE THAT END LAPS ARE SHINGLE FASHION.

4. ATTACH DECK TO EDGE SUPPORT WITH 5/8" DIA PUDDLE WELD @ 6" O.C.

METAL FORM DECK (REF. SECTION 05600)
. FORM DECK SHALL BE AS FOLLOWS:

FORM DECK SHALL BE AS FOLLOWS:

A. ALL METAL FORM DECK EXCEPT WHERE SHOWN ON PLAN SHALL BE 1 1/2" DEEP 24 GAGE WIDE RIB, GALVANIZED G90.

B. FORM DECK SHALL BE PLACED IN AT LEAST TWO SPAN SEGMENTS. NO SINGLE SPAN CONDITIONS SHALL BE USED.

C. STEEL DECK SHALL CONFORM TO ASTM A446 GRADES A, B, C, D, E OR F FOR GALVANIZED DECK, MINIMUM YELD STRENGTH OF 33,000 PSI.

D. STEEL DECK SHALL BE GALVANIZED WITH A PROTECTIVE ZINC COATING CONFORMING TO ASTM A525 G90 CLASS.

a) FORM DECK UNITS SHALL BE WELDED TO EACH STRUCTURAL SUPPORT FORM DECK OWINS SHALL BE WELDED ID EACH STROCTURAL SUPPORT MEMBER USING 5/8" DUMETER PUDDLE WELDS AT 12" SPACING MAX. WELD METAL SHALL PENETRATE ALL LAYERS OF DECK MATERIAL AT END LAPS AND SIDE JOINTS AND SHALL BE COMPLETELY FUSED TO THE SUPPORTING MEMBERS.

b) SIDE LAPS OF ADJACENT UNITS SHALL BE FASTENED BY WELDING (ON 18 GAUGE OR HEAVIER DECK ONLY) OR SHEET METAL SCREWS, SO THAT SPACING BETWEEN FASTENERS AND BETWEEN THE FIRST FASTENER AND SUPPORT DOES NOT EXCEED 18 INCHES.

c) AT ALL ROOF OPENINGS AND PERIMETER ROOF EDGE CONDITIONS, ROOF

C) AT ALL ROUF OPENINGS AND PERIMETER ROUF EUGE CONDITIONS, ROUDECK UNITS SHALL BE WELDED TO EDGE STEEL USING 5/8" DIAMETER PUDDLE WELDS AT 12" O.C.

PROVIDE A MINIMUM END BEARING OF 2" OVER SUPPORTS.

END LAPS OF SHEETS SHALL BE A MINIMUM OF TWO INCHES AND SHALL OCCUR OVER SUPPORTS. ROOFS SHALL BE RECTED BEGINNING AT THE LOW SIDE TO INSURE THAT END LAPS ARE SHINGLE FASHION.

4. ATTACH DECK TO EDGE SUPPORT WITH 5/8" DIA PUDDLE WELD @ 12" O.C.

SHOP DRAWINGS AND OTHER SUBMITTALS (REF. SECT. 01330) REFER TO DIVISION 1 OF THE SPECIFICATIONS FOR SUBMITTAL PROCEDURES AND REQUIREMENTS. REFER TO THE APPLICABLE SPECIFICATION SECTIONS FOR TECHNICAL CONTENT REQUIREMENTS. INCOMPLETE SUBMITTALS WILL BE RETURNED

WITHOUT REVIEW. WITHOUT REVIEW.

SUBMIT SECIFIC COMPONENTS, SUCH AS COLUMNS, FOOTINGS, ETC., IN A SINGLE PACKAGE. SUBMIT SIMILAR FLOORS TOGETHER.

DO NOT USE OR REPRODUCE DESIGN DRAWINGS AS PART OF SHOP DRAWINGS

WITHOUT THE WRITTEN PERMISSION OF THE ARCHITECT. WHEN CAD FILES OR COPIES OF THE DESIGN DRAWINGS ARE MADE AVAILABLE, IT IS UNDER THE FOLLOWING

ALL INFORMATION CONTAINED IN THE CAD FILES OR COPIES OF THE DESIGN DRAWINGS ARE INSTRUMENTS OF SERVICE OF THE ARCHITECT AND SHALL NOT BE USED FOR OTHER PROJECTS, ADDITIONS TO THE PROJECT OR THE COMPLETION OF THE PROJECT BY OTHERS. CAD FILES AND COPIES OF THE DESIGN DRAWINGS REMAIN THE PROPERTY OF THE ARCHITECT AND IN NO CASE SHALL THEIR TRANSFER BE CONSIDERED A SALE;

B. CAD FILES OR COPIES OF THE DESIGN DRAWINGS ARE NOT CONTRACT

DOCUMENTS. IN THE EVENT OF A CONFLICT, THE DESIGN DRAWINGS SHALL

THE USE OF CAD FILES OR COPIES OF THE DESIGN DRAWINGS SHALL NOT N ANY WAY RELIEVE THE CONTRACTOR RESPONSIBILITY FOR PROPER CHECKING AND COORDINATION OF DIMENSIONS, DETAILS, SIZES AND QUANTITIES OF MATERIALS AS REQUIRED FOR THE PREPARATION OF SHOP DRAWINGS THAT ARE COMPLETE AND ACCURATE; AND,

D. THE CONTRACTOR SHALL REMOVE INFORMATION THAT IS NOT REQUIRED FOR THEIR WORK FROM THE CAD FILES OR COPIES OF THE DESIGN DRAWINGS, INCLUDING THE TITLE BLOCK.

CLEARLY FLAG AND CLOUD ALL CHANGES AND ADDITIONS MADE ON RESUBMITTALS.

COUNT THOSE ITEMS CLOUDED WILL BE REVIEWED.

SUBMITTALS FOR CUSTOM DESIGNED, LOAD—CARRYING ITEMS THAT ARE REQUIRED BY
CODES OR STANDARDS TO RESIST FORCES MUST BE PREPARED BY, OR UNDER THE DIRECT SUPERVISION OF, A DELEGATED ENGINEER. EXAMPLES INCLUDE PRECAST CONCRETE, STRUCTURAL LIGHT GAGE STEEL FRAMING, EXTERIOR ENCLOSURE SYSTEMS, AND SHORING AND RESHORING.

A DELEGATED ENGINEER IS DEFINED AS A REGISTERED PROFESSIONAL ENGINEER WHO

A DELEGATED ENGINEER IS DETINED AS A REDISTRED PROFESSIONAL ENGINEER SPECIALIZES IN AND UNDERTAKES THE DESIGN OF STRUCTURAL COMPONENTS OR STRUCTURAL SYSTEMS INCLUDED IN A SPECIFIC SUBMITTAL PREPARED FOR THIS PROJECT AND IS AN EMPLOYEE OR OFFICER OF, OR CONSULTANT TO, THE CONSTRUCTION MANAGER OR FABRICATOR RESPONSIBLE FOR THE SUBMITTAL. THE DELEGATED ENGINEER SHALL SIGN, SEAL AND DATE THE SUBMITTAL, INCLUDING CALCULATIONS AND DRAWINGS. SEE SPECIFICATIONS FOR MORE SPECIFIC CRITERIA.

THE CONTRACTOR SHALL REVIEW AND APPROVE SUBMITTALS AND SHALL SIGN AND DATE EACH DRAWING PRIOR TO SUBMITTING TO THE ARCHITECT. THIS APPROVAL IS TO CONFIRM THAT THE SUBMITTAL IS COMPLETE, COMPLIES WITH THE SUBMITTAL REQUIREMENTS AND IS COORDINATED WITH FIELD DIMENSIONS, OTHER TRADES, ERECTIO SEQUENCING AND CONSTRUCTABILITY.

THE STRUCTURAL ENGINEER REVIEWS SUBMITTALS TO CONFIRM THAT THE SUBMITTAL IS IN GENERAL CONFORMANCE WITH THE DESIGN CONCEPT PRESENTED IN THE CONTRACT DOCUMENTS. QUANTITIES AND DIMENSIONS ARE NOT CHECKED. CONSTRUCTABILITY IS THE SOLE RESPONSIBILITY OF THE CONSTRUCTION MANAGER/FABRICATOR.

THE STRUCTURAL ENGINEER'S REVIEW OF DELEGATED ENGINEER SUBMITTALS IS LIMITED TO VERIFYING THAT THE SPECIFIED STRUCTURAL SUBMITTAL HAS BEEN FURNISHED AND SEALED BY THE DELEGATED ENGINEER AND THAT THE DELEGATED FORMISHED AND SEARCH BY THE DELEGATED ENGINEER AND THAT THE DELEGATED ENGINEER HAS UNDERSTOOD THE DESIGN INTENT AND USED THE SPECIFIED STRUCTURAL CRITERIA. NO DETAILED CHECK OF CALCULATIONS WILL BE MADE. ALL COMMENTS BY THE STRUCTURAL ENGINEER WILL BE MADE ON THE SHOP DRAWINGS. CALCULATIONS ARE FOR ARCHITECT'S AND ENGINEER'S RECORDS AND ARE NOT APPROVED NOR RETURNED.

10. ALL SHOP DRAWINGS MUST BE REVIEWED AND STAMPED BY THE CONTRACTOR PRIOR TO TO SUBMITTAL TO THE ARCHITECT/ENGINEER.

THE CONTRACTOR SHALL SUBMIT FOR THE ARCHITECTS/ENGINEER'S REVIEW SHOP DRAWINGS FOR THE FOLLOWING ITEMS
REINFORCING STEEL

PRECAST CONCRETE SHOP DWGS.(\*) STRUCTURAL PRECAST CONCRETE ENGINEERING CALCULATION (#)

FORMWORK SHORING PLANS(\*)(#)
CONCRETE MIX DESIGNS
CONSTRUCTION JOINT LOCATIONS IN STRUCTURAL FLOORS

PRE-ENGINEERED METAL BLDGS.(\*)(#)
TILT UP CONCRETE WALL SHOP DWGS WITH REINF. & EMBED PLACEMENTS. ERECTION & SHORING PLANS (\*)(#)TILT WALL BRACING DESIGN FOR TEMPORARY & CONSTRUCTION LOADS (\*)(#)

STRUCTURAL STEEL SHOP DWGS (\*)
STRUCTURAL STEEL CONNECTIONS ENGINEERING CALCULATIONS (\*)

ITEMS MARKED (\*) SHALL HAVE SHOP DRAWINGS SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF FLORIDA, ITEMS MARKED (#) SHALL BE SUBMITTED FOR ENGINEERS RECORD ONLY.

COMPOSITE METAL DECK & SLAB (REF. SECTION 05600)

THE METAL DECK SHOWN ON THE DRAWINGS IS THE MINIMUM REQUIRED FOR UNSHORED CONSTRUCTION FOR THE TYPICAL CONDITIONS OF TWO CONTINUOUS SPANS OR MORE. THE DECK SUPPLIER SHALL INCREASE THE GAGE THICKNESS, IF NECESSARY, FOR SINGLE SPAN CONDITIONS THE DECK GAGE AND DEPTH HAVE BEEN SELECTED BASED ON THE WET WEIGHT OF

CONCRETE AND THE FINAL DESIGN LOADS ONLY. CONSTRUCTION MATERIALS MAY NOT BE PLACED ON THE BARE METAL DECK.

3. THE FINAL SLAB THICKNESS SHALL BE NO LESS THAN CALLED FOR ON THE PLANS.

4. CONTRACTOR IS TO PROVIDE ADDITIONAL CONCRETE REQUIRED DUE TO THE DEFLECTION

OF UNSHORED BEAMS AND DECK.

OF UNSHORED BEAMS AND DECK.

S. STEEL DECK SHALL CONFORM TO ASTM A446 GRADES A, B, C, D, E OR F FOR CALVANIZED DECK, MINIMUM YELD STRENGTH OF 33,000 PSI.

6. STEEL DECK SHALL BE GALVANIZED WITH A PROTECTIVE ZINC COATING CONFORMING TO

ASTM A525 G90 CLASS







V	DESCRIPTION	DATE

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RECORD **DOCUMENTS** 

THIS RECORD DOCUMENT HAS BEEN PREPARED BASED ON INFORMATION PROVIDED BY OTHERS. THE A/E HAS NOT /ERIFIED THE ACCURACY AND/OR COMPLETENESS OF AND/OR COMPLETENESS OF THIS INFORMATION AND SHAL NOT BE RESPONSIBLE FOR AN ERRORS OR OMISSIONS THAT MAY BE INCORPORATED HEREIN AS A RESULT.

DATE OF SEAL

SHEET CONTENT
GENERAL NOTES
ABBREVIATIONS
SYMBOLS DRAWN BY T.F

CHECKED BY

**S0.01** 

P.A

#### STRUCTURAL STEEL (REEF SECTION 05120) STEEL SHALL CONFORM TO THE FOLLOWING ASST. SPECIFICATION: A. W-SHAPES (O.N.O..), A572(GR50) B. STRUCTURAL STEEL PIPE (FY-35) C. STRUCTURAL STEEL TUBE (FY-46) D. BASE PLATES AND CONNECTION PLATES (FY=36) E. ALL OTHER STEEL (FY=36) 2. BOLTED CONNECTIONS: A. ALL TRUSS CONNECTIONS, A325-SC, SLIP CRITICAL, SIZES AS SHOWN IN THE DETAILS B. LOAD WASHERS MUST BE USED FOR FIELD CONNECTIONS. C. ALL BEAM TO COLUMN CONNECTIONS, BRACE CONNECTIONS AND MOMENT

- CONNECTED MEMBERS SHALL HAVE A325-N D. ALL OTHER CONNECTIONS (UNLESS NOTED OTHERWISE) 3/4" DIAMETER, A325 N E. OVERSIZED AND LONG-SLEEVED HOLES ARE PERMITTED ONLY WHEN DESIGN BY
- CONNECTION DELEGATED ENGINEER IS SUBMITTED AND APPROVE PRIOR TO SUBMISSION OF STEEL CONNECTION SHOP DWGS. STRUCTURAL DRAWINGS.
- F. THE SHOP DRAWINGS SHALL CLEARLY INDICATE THE TYPE OF BOLT USED IN EACH CONNECTION AND THE ALLOWABLE VALUES USED FOR THE VARIOUS BOLT TYPES.
- G. THE FOLLOWING MINIMUM STANDARDS APPLY: (U.N.O.)
- a) MINIMUM PLATE THICKNESS" 3/8"
- b) MINIMUM BOLT DIAMETER" 3/4"
- c) MINIMUM WELD" THICK THROAT 3/16" d) MINIMUM DESIGN LOAD ON ANY CONNECTION 10 kips
- 5. WELDED CONNECTIONS:

  A. ALL SHOP AND FIELD WELDING SHALL CONFORM TO WAS STRUCTURAL WELDING
- B. MINIMUM WELD: 3/16" THICK THROAT.
  C. ALL STEEL TO STEEL CONNECTIONS NOT SHOWN BOLTED SHALL BE WELDED TO DEVELOP FULL SHEAR CAPACITY OF CONNECTING MEMBERS AS PER AISC SPECIFICATIONS. MINIMUM SIZE OF FILLET WELD (UNLESS NOTED OTHERWISE ON DRAWINGS):
  - ATERIAL THICKNESS OF MINIMUM SIZE OF THICKER PART JOINED FILLET WELD: TO 1/4" INCLUSIVE 1/8" ALL AROUND
  - TO 1/4" TO 1/2" 3/16" ALL AROUND OVER 1/2" TO 3/4" 1/4" ALL AROUND
- OVER 3/4" TO 1 1/2" 5/16" ALL AROUND
  OVER 3/4" TO 1 1/2" 5/16" ALL AROUND
  4. SPLICING OF STRUCTURAL STEEL WHERE NOT DETAILED IS PROHIBITED WITHOUT PRIOR WRITTEN APPROVAL OF THE ARCHITECT.
- BEAM CONNECTIONS:

  A. DESIGN CONNECTIONS FOR BEAMS TO SUPPORT 0.55 OF THE UNIFORM CAPACITY SHOWN IN AISC "TABLES FOR ALLOWABLE LOADS ON BEAMS" FOR THE GIVEN SECTION AND SPAN UNLESS OTHERWISE SHOWN.
- OALVANIZING:

  A. HOT-DIP GALVANIZE AFTER FABRICATION ALL STRUCTURAL STEEL ITEMS AND THEIR
  CONNECTIONS PERMANENTLY EXPOSED TO THE OUTSIDE.

  B. EXAMINE DRAWINGS FOR OTHER ITEMS TO BE GALVANIZED.
- ANCHOR BOLTS EXCEPT FOR STEEL TRUSSES:
- A. ALL ANCHOR BOLTS SHALL BE MADE FROM THREADED ROUND STOCK, ASTM A36.
  ALL ANCHOR BOLTS, NUTS AND WASHERS USED WITH GALVANIZED BASE PLATES
- SHALL BE HOT DIP GALVANIZED AFTER FABRICATION.
- B. NUTS SHALL BE HEX HEAD, ASTM A563.
  C. WASHERS FOR ALL BASE PLATES SHALL BE 1/4" THICK PLATES EXTENDING MINIMUM 1" FROM EDGE OF BASE PLATE HOLES ON EACH SIDE WITH HOLES 1/16" LARGER THAN THE NOMINAL BOLT DIAMETER AND CONFORM TO ASTM A36.

  D. ALL ANCHOR BOLTS SET IN CONCRETE SHALL UTILIZE TEMPLATES.

- TEMPLATES SHALL BE DETAILED ON THE SHOP DRAWINGS.

  NON-SHRINK GROUT FOR BASE PLATES AND BEARING PLATES

  A. NON-METALLIC, SHRINKAGE RESISTANT, PREMIXED, NON-CORROSIVE, NON-STAINING PRODUCT CONTAINING PORTLAND CEMENT SILICA SANDS SHRINKAGE COMPENSATING AGENTS, AND FLUIDITY IMPROVING COMPOUNDS, AND SHALL CONFORM TO CORPS OF ENGINEERS SPECIFICATION FOR NON-SHRINK GROUT,
- B. TWENTY-EIGHT DAY COMPRESSIVE STRENGTH SHALL BE A MINIMUM OF
- 6,000 PSI. SUBMITTALS:

CRD-C621-83.

- A. SEE INDIVIDUAL SPECIFICATION SECTIONS FOR DESIGN RESPONSIBILITIES. AND THE STRUCTURAL ENGINEER OF RECORD'S REVIEW RESPONSIBILITIES WITH REGARD TO SUBMITTAL REVIEW.
- REQUIREMENTS AND PROCEDURES.

  C. REPRODUCTION OF STRUCTURAL DRAWINGS FOR SHOP DRAWINGS WILL NOT BE PERMITTED.
- D. REVIEW OF SUBMITTALS OR SHOP DRAWINGS BY THE STRUCTURAL ENGINEER DOES NOT RELIEVE THE CONTRACTOR OF THE SOLE RESPONSIBILITY TO REVIEW AND CHECK ALL SUBMITTALS AND SHOP DRAWINGS BEFORE SUBMITTING TO THE STRUCTURAL ENGINEER. CONTRACTOR REMAINS SOLELY RESPONSIBLE FOR ERRORS AND OMISSIONS ASSOCIATED WITH THE PREPARATION OF SHOP DRAWINGS AS THEY PERTAIN TO MEMBER SIZES, DETAILS, AND DIMENSIONS SPECIFIED IN THE

## ENGINEERED LIGHT-GAGE METAL FRAMING (REF SECTION 05400)

- DESIGN OF ENGINEERED LIGHT-GAGE METAL FRAMING IS THE SOLE RESPONSIBILITY OF THE METAL SUPPLIER. SUBMIT SHOP DRAWINGS SEALED BY AN ENGINEER (DELEGATED ENGINEER) LICENSED IN THE STATE OF FLORIDA REVIEW OF SHOP DRAWINGS SHALL BE FOR CONFORMANCE WITH THE CONTRACT DOCUMENTS REGARDING ARRANGEMENT AND SIZES OF MEMBERS AND THE CONTRACTOR'S INTERPRETATION OF THE DESIGNS LOADS AND CONTRACT DOCUMENT DETAILS. SUCH REVIEW DOES NOT RELIEVE THE CONTRACTOR OF FULL
  RESPONSIBILITY FOR THE DESIGN OF THE ENGINEERED LIGHT-GAGE METAL FRAMING AND THEIR CONNECTIONS.
- TIGHT-GAGE METAL FRAMING SHALL BE DESIGNED IN ACCORDANCE WITH THE DESIGN/ CODE INFORMATION LISTED AT THE BEGINNING OF THESE STRUCTURAL GENERAL NOTES.
- DESIGN OF LIGHT GAGE METAL FRAMING SHALL CONFORM TO THE LATEST EDITION OF SPECIFICATION FOR THE DESIGN OF COLD—FORMED STRUCTURAL STEEL MEMBERS, AISI

#### OPEN WEB STEEL JOISTS (REF. SECTION 05210)

- . ALL STEEL JOISTS SHALL CONFORM TO THE STANDARDS OF THE STEEL JOIST
- 2. THE JOIST MANUFACTURER SHALL REVIEW THE DRAWINGS AND PROVIDE JOISTS CAPABLE OF CARRYING THE FOLLOWING LOADS:
- A. ROOF DEAD LOAD......10 PSF B. SUPERIMPOSED DEAD LOAD......10
- REFER TO GROSS UPLIFT DIAGRAMS, DWG SO.03 AND SUPERIMPOSED DEAD LOADS TABULATED FOR NET UPLIFT DESIGN OF JOISTS BY FABRICATOR, BUT NOT LESS THAN 20 PSF
- 4. THE JOISTS SHOWN ON THE PLANS ARE THE MINIMUM SIZE REQUIRED.
- DEPTHS SHOWN MAY NOT BE EXCEEDED.

  5. JOISTS SHOULD BE CAMBERED IN ACCORDANCE WITH S.J.I. STANDARD CAMBERS.
- 5. JOISTS SHALL BE WELDED TO ALL SUPPORTING BEAMS WITH A MINIMUM OF TWO 3/16 INCH BY 2 INCH LONG FILLET WELDS, OR FOR THE UPLIFT FORCE WHICHEVER IS GREATER.
- 7. PROVIDE A MINIMUM END BEARING ON STEEL SUPPORTS AS REQUIRED BY SJI.
- PROVIDE A MINIMUM END BEARING UN SIEL SUPPORTS AS REQUIRED BY SJI.
  STAGGER THE ENDS OF JOISTS IF NECESSARY.
  PROVIDE HORIZONTAL OR DIAGONAL TYPE BRIDGING FOR ALL JOISTS AS REQUIRED
  BY SJI SPECIFICATION. THE ENDS OF ALL BRIDGING LINES TERMINATING AT
  WALLS OR BEAMS SHALL BE ANCHORED THERETO AT TOP AND BOTTOM CHORDS.
  PROVIDE ALL REQUIRED BRIDGING ANCHORS.
  PROVIDE ALL JOISTS AND ACCESSORIES WITH ONE SHOPCOAT OF PAINT AS
  SPECIFICION IS USEDANDADED DEPENDE SUPPAGE AS BEGINNER BY BAINT.
- SPECIFIED IN SJI STANDARDS. PREPARE SURFACE AS REQUIRED BY PAINT MANUFACTURERS RECOMMENDATIONS AND PROVIDE A CONTINUOUS DRY PAINT THICKNESS OF NOT LESS THAN 1 MIL. STEEL JOISTS THAT ARE TO RECEIVE SPRAYED FIREPROOFING. PAINT ADHESION SHALL CONFORM TO THE SPECIFICATIONS OF THE UNDERWRITERS LABORATORIES.
- 10. REFER TO THE SPECIFICATIONS FOR ALL OTHER REQUIREMENTS.
  11. EXAMINE ARCHITECTURAL, STRUCTURAL AND MECHANICAL DRAWINGS FOR ANY ACCESSORIES OR ATTACHMENTS REQUIRED TO BE PROVIDED FOR STEEL JOISTS.

- PRE-ENGINEERED METAL BUILDINGS (REF. SECTION 13125) DESIGNS OF PRE ENGINEERED SYSTEMS SPECIFIED IN THE CONTRACT DOCUMENTS WHICH ARE DESIGNED/ ENGINEERED BY OTHERS IS THE SOLE RESPONSIBILITY OF THE PEMB SUPPLIER. SUBMITTALS OF SUCH SYSTEMS SHALL BE SEALED BY AN ENCINEER LICENSED IN THE PROJECT STATE.
  REVIEW OF SUBMITTALS BY THE STRUCTURAL ENGINEER SHALL BE FOR
  CONFORMANCE WITH THE CONTRACT DOCUMENTS WITH REGARD TO THE ARRANGEMENT AND SIZES OF MEMBERS SHOWN ON THE STRUCTURAL CONTRACT DOCUMENTS AND THE CONTRACTOR'S INTERPRETATION OF THE DESIGN INFORMATION INCLUDED IN THE CONTRACT DOCUMENTS. SUCH REVIEW BY THE STRUCTURAL ENGINEER SHALL NOT IMPLY ANY RESPONSIBILITY FOR THE ACTUAL DESIGNS OF SUCH SYSTEMS.

  CONTRACTOR HAS FULL RESPONSIBILITY FOR DIMENSIONAL ACCURACY AND CONFORMANCE WITH THE INFORMATION CONTAINED IN THE CONTRAC
- DOCUMENTS.

  2. SEE SPECIFIC SECTIONS OF STRUCTURAL GENERAL NOTES BELOW AND SPECIFICATIONS FOR THE APPROPRIATE DESIGN RESPONSIBILITIES OF THE CONTRACTOR
- A CERTIFIED TESTING AGENCY SHALL BE ENGAGED BY CONTRACTOR TO PERFORM INDUSTRY STANDARD INSPECTIONS TO ENSURE CONFORMANCE WITH PLANS AND SPECIFICATIONS (IF PROVIDED). SUBMIT REPORTS TO ARCHITECT AND
- ENGINEER.
  4. ALL STRUCTURAL STEEL SHALL BE MINIMUM ASTM A36 UNLESS NOTED OTHERWISE.
- 5. STRUCTURAL SUBMITTALS ARE REQUIRED FOR REVIEW AS AN INDICATION THAT INTENT HAS BEEN UNDERSTOOD AND THAT SPECIFIED CRITERIA HAVE BEEN USED. 6. STRUCTURAL SUBMITTALS SHALL IDENTIFY THE PROJECT AND LIST LOADING AND OTHER DESIGN CRITERIA. FABRICATION AND ERECTION DRAWINGS SHALL INDICATE IN DETAIL THE CONSTRUCTION OF THE STANDARD STRUCTURE USED OR AS MODIFIED TO COMPLY WITH THE REQUIREMENTS OF THE PARTICULAR PROJECT. THEY SHALL INDICATE ALL CONNECTION DETAILS, OPENINGS AND PROJECT: INFECT SHALL INICIATE ALL SHOW THE MAGNITUDE AND LOCATION OF BUILDING REACTIONS ON THE FOUNDATION UNDER ALL DESIGN CONDITIONS. CALCULATIONS SUPPORTING THE DESIGN SHALL BE SUBMITTED NOT ONLY FOR THE STANDARD STRUCTURE BUT FOR MODIFICATIONS AND FOR RELATED
- THE STANDARD STRUCTURE BUT FOR MODIFICATIONS AND FOR RELATED
  COMPONENTS REQUIRING STRUCTURAL DESIGN.

  COLUMN REACTIONS AND ANCHOR BOLT GROUP DESIGN SHALL BE DETERMINED BY METAL BUILDING
  MANUFACTURER. TO TRANSMIT LOADS TO THE FOUNDATIONS. THESE DESIGNS SHALL BE SUBMITTED TO
  THE ENGINEER FOR FINAL COORDINATION WITH FOUNDATION. NO FABRICATION OF PEMB SHALL BEGIN
  PRIOR TO APPROVED ANCHOR BOLT DESIGNS.

  MOMENT FRAME BRACING LOCATIONS AS LATERAL RESISTING ELEMENTS ARE SHOWN ON THE DEPONTATION.
- SUBMIT LOCATION OF ALL LATERAL RESISTING ELEMENTS FOR APPROVAL. NO X-BRACING IS PERMITTED UNLESS LOCATIONS ARE APPROVED BY ARCHITECT.

  9. PROVIDE MIDHEIGHT HORIZONTAL STRUCTURAL MEMBER TO BRACE ALL EXTERIOR CMU AND TILT WALLS
- EXCEEDING 19ET HEIGHT

#### TILT-UP CONCRETE PANELS (REF. SECTION 03470)

- REINFORCING STEEL SHOWN ON PLANS IS FOR THE DESIGN OF SLABS INPLACE. ANY ADDITIONAL REINFORCING STEEL REQUIRED FOR LIFTING, TOGETHER WITH THE NECESSARY LIFTING INSERTS AND FASTENERS. IS TO
- BE PROVIDED BY THE TILT-SLAB CONTRACTOR—ENGINEER.

  2. THE PANELS SHALL BE SUPPORTED TEMPORARILY IN PLACE AND BRACED
  TO WITHSTAND ALL LOADS UNTIL THEY ARE SUPPORTED BY THE STRUCTURAL STEEL MEMBERS AND ROOF DECKING.
- 3. ALL PANELS SHALL BE ALIGNED AND CONNECTED TOGETHER.

  4. GROUTING UNDER PANELS SHALL BE CONTINUOUS AND ACROSS FULL WIDTH OF PANELS, AFTER PANELS ARE PLUMB AND SET FOR THEIR FINAL POSITION.
- 5. FOR SIZE AND LOCATION ON EMBEDS FOR BEAM CONNECTIONS, SEE FLOOR AND ROOF PLANS AND CONNECTION DETAILS.
- 6. CONCRETE FOR ALL PANELS 'C AT 28 DAYS = 5000 PSI, fy = 60 KSI.
  LOCATE ALL EMBEDS CORRECTLY IN THE CAST PANELS.

  7. PANEL DIMENSIONS TO BE CHECKED FOR ACCURACY BY CONTRACTOR.
- 8. VERIFY WITH MECHANICAL, ELECTRICAL, AND PLUMBING FOR OPENINGS IN
- CONCRETE TILT-UPS.

  9. ALL PANELS SHALL BE 9 1/4" THICK UNLESS NOTED OTHERWISE.
- 10. ALL PANELS SHALL BE REINFORCED WITH #4's ⊕12"o.c. HORIZONTAL AND VERTICAL, U.N.O. 2-#5 CONTINUOUS AT BOTTOM
- OF PANEL, 2-#5 CONTINUOUS AT EACH SIDE AND TOP.

  11. SEE STRUCTURAL DETAILS FOR DOWELS TO SLAB AND INSERTS AND PLATES FOR SUPPORT OF JOISTS AND BEAMS AND PANEL CONNECTIONS
- 12. PANELS SHALL NOT BE LIFTED UNTIL CONCRETE HAS ATTAINED A MINIMUM
- COMPRESSIVE STRENGTH f'c OF 2500 PSI.
- DELEGATED ENGINEER SHALL PROVIDE SHORING PLANS SIGNED AND SEALED BY FLORIDA PE FOR THE ERECTION OF THE PANELS.

   CONTRACTOR TO PROVIDE PANEL SHOP DRAWINGS WITH ALL EMBEDS. ALL PANEL DIMENSIONS SHALL BE PROVIDED ON SHOP DRAWINGS. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO LOCATE ALL EMBEDS CORRECTLY IN THE CAST PANELS.







DESCRIPTION

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#### RECORD **DOCUMENTS**

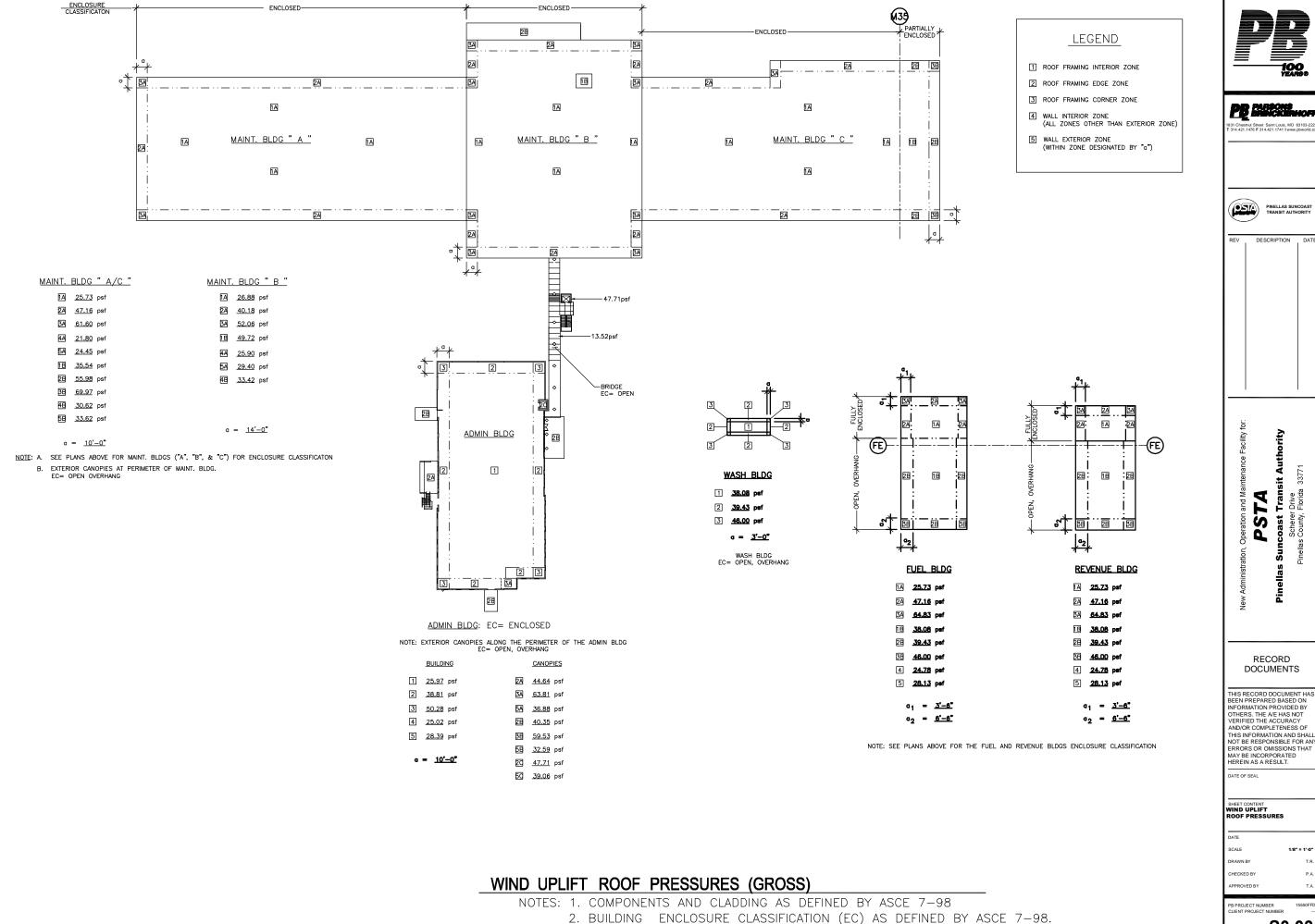
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DATE OF SEAL

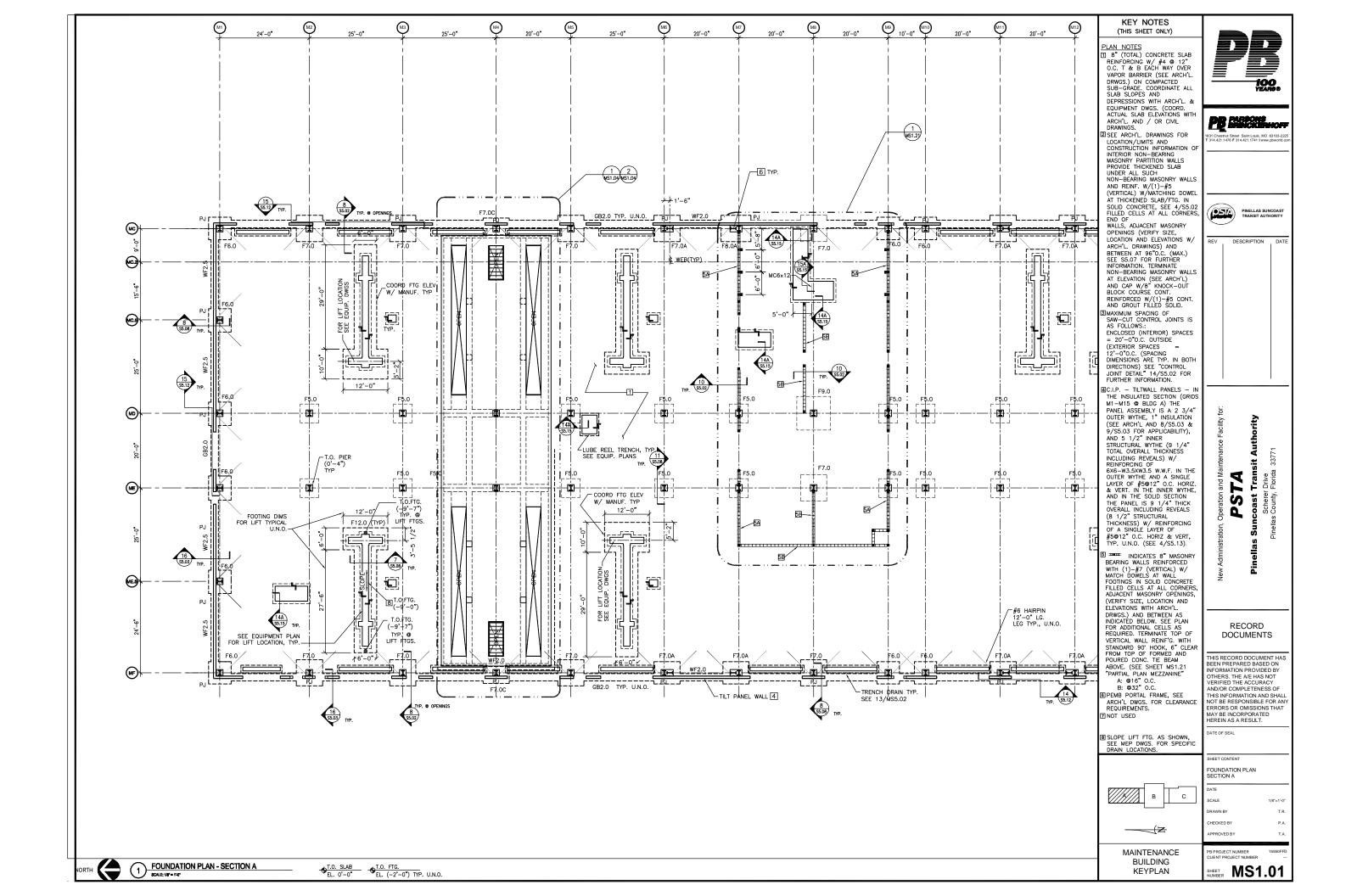
SHEET CONTENT
GENERAL NOTES
ABBREVIATIONS
SYMBOLS

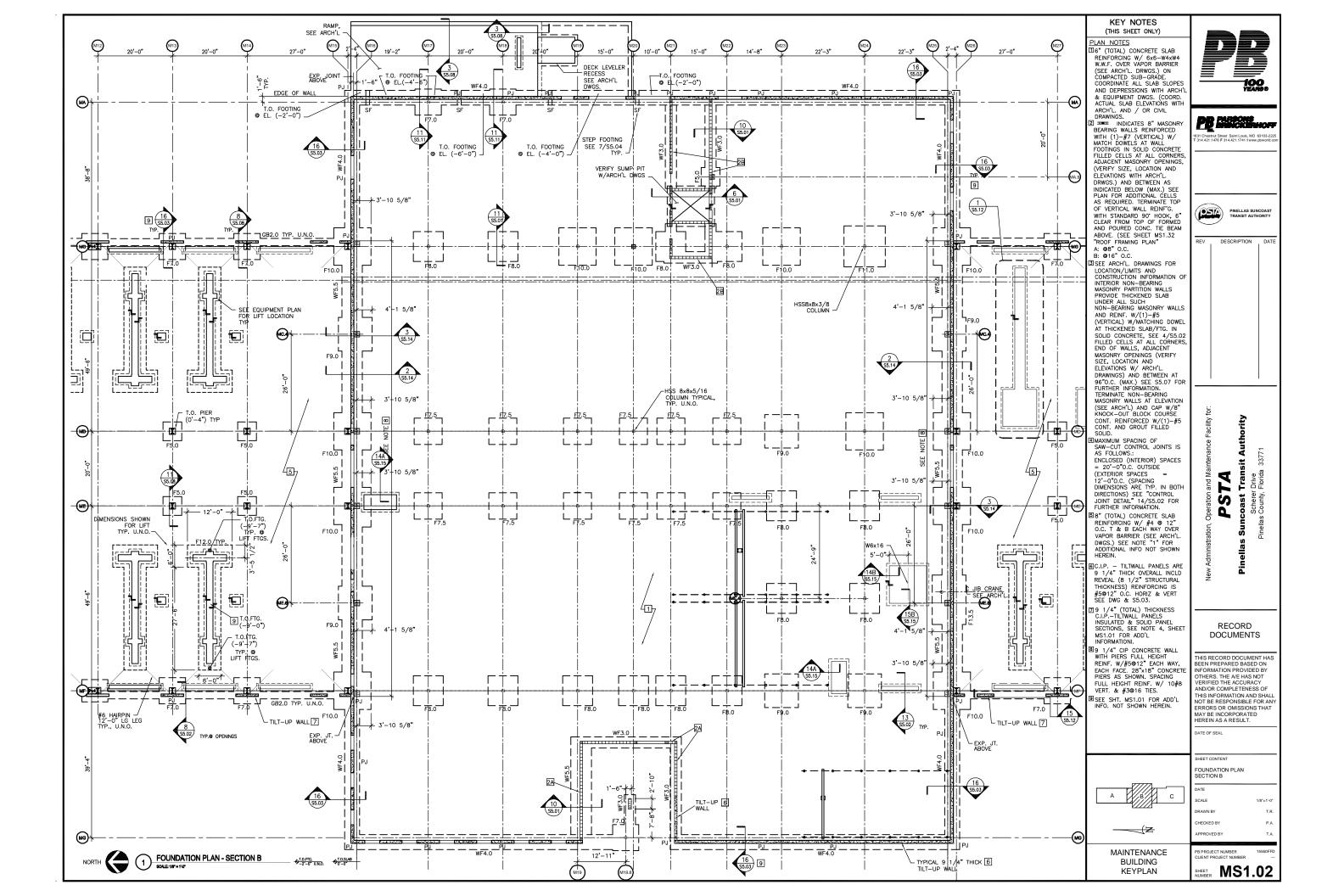
DRAWN BY T.R. CHECKED BY P.A.

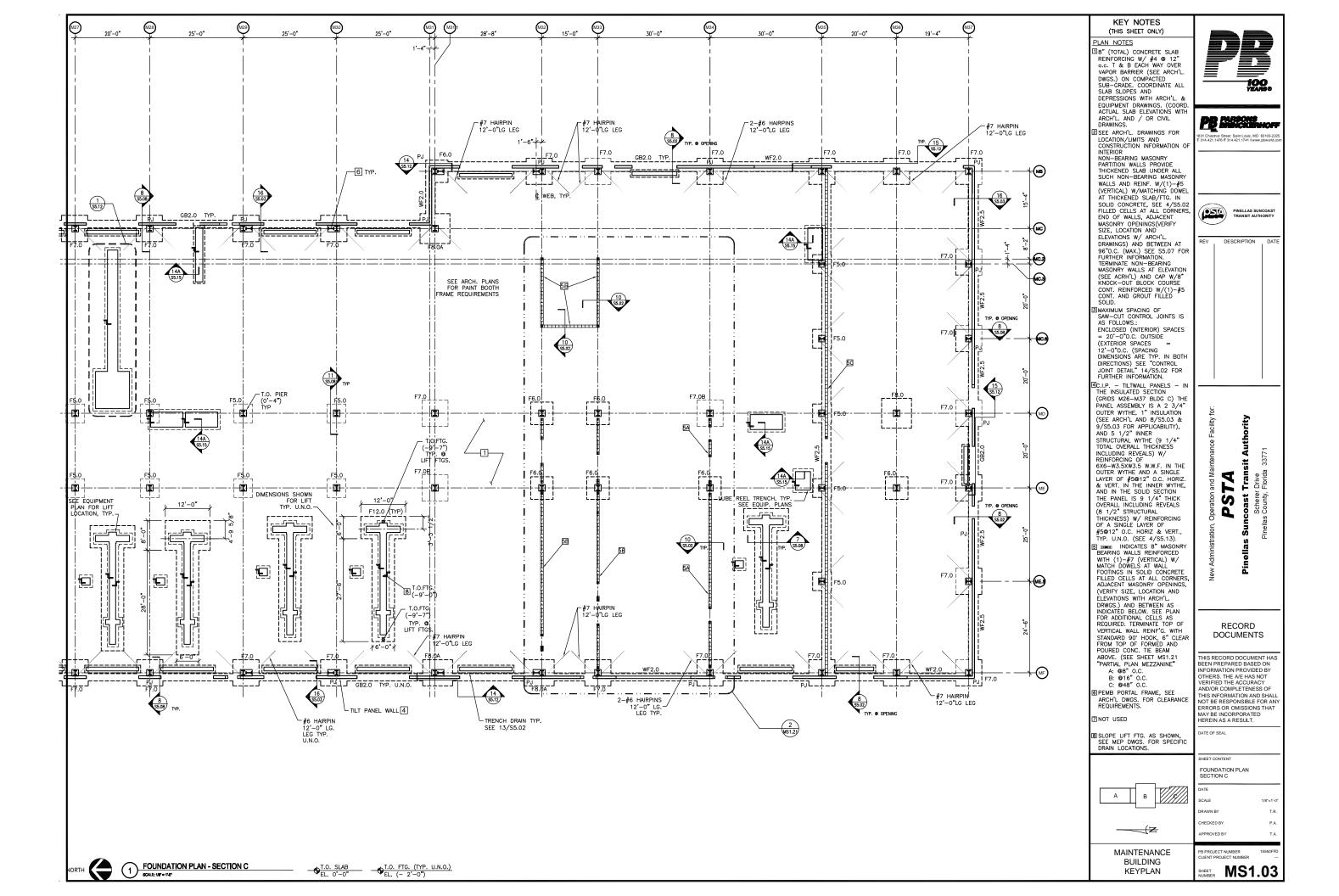
**S0.02** 

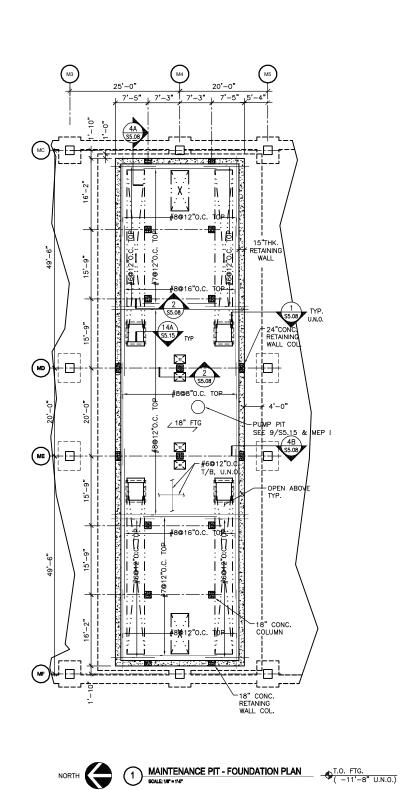


**S0.03** 

















KEY NOTES (THIS SHEET ONLY)

1 SEE MECHANICAL DWGS. FOR SIZE & LOCATION OF MECHANICAL OPENINGS.





PINELLAS SUNCOAST TRANSIT AUTHORITY

EV DESCRIPTION

**57A** st Transit Authority

PSTA
as Suncoast Transit

RECORD DOCUMENTS

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DATE OF SEAL

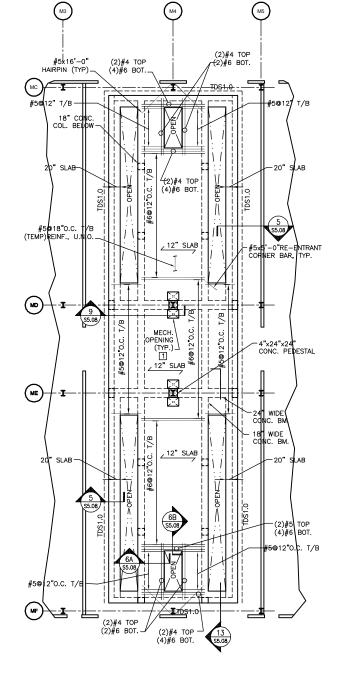
SHEET CONTENT

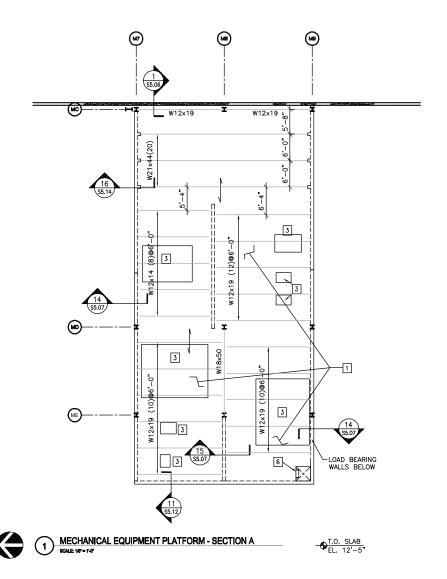
MAINTENANCE PIT FOUNDATION & SLAB PLANS

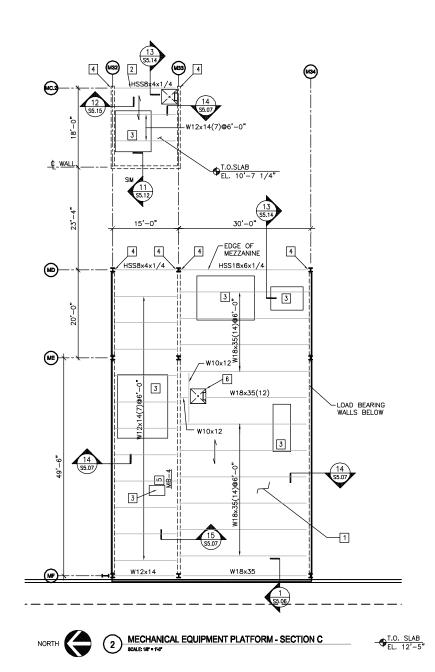
DATE
SCALE
DRAWN BY

PB PROJECT NUMBER CLIENT PROJECT NUMBER

SHEET MS1.04

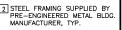






KEY NOTES (THIS SHEET ONLY)

1 MEZZANINE SLAB 1 MEZZANINE SLAB
CONSTRUCTION: 5" (TOTAL)
NORMAL WEIGHT CONCRETE
REINFORCED W/
6x6-W1.4W1.4 W.W.F. OVER
2V1/20 (2"DEEP, 20 GAGE)
COMPOSITE METAL DECK (SEE
GENERAL NOTES ON SHEET
SO.01 FOR FURTHER
INFORMATION) ATTACH
COMPOSITE DECK WITH 36/4
PATTERN OF WELDING
WASHERS. PERIMETER WELDS
AT 12" O.C. MAX. #10 TEK.
SCREWS OR EQUIV. SIDE LAP
BETWEEN EA. SUPPORT



3 PROVIDE 6" HOUSEKEEPING PAD, SEE 9/S5.02 AND MEP DWGS FOR SIZE AND LOCATION.

4 SPECIAL BEAM REACTION DUE TO HANDRAIL: VERTICAL = 12 KIPS, HORIZONTAL = 1 KIPS, TORSION = 3 KIP-FT.

5) SEE MASONRY LINTEL SCHEDULE (3/S5.12). TYPICAL LINTELS SPECIFIED BY LENGTH. SPECIAL LINTELS ARE NOTED ON PLANS.









DESCRIPTION

6 LADDERS, SEE ARCH'L.

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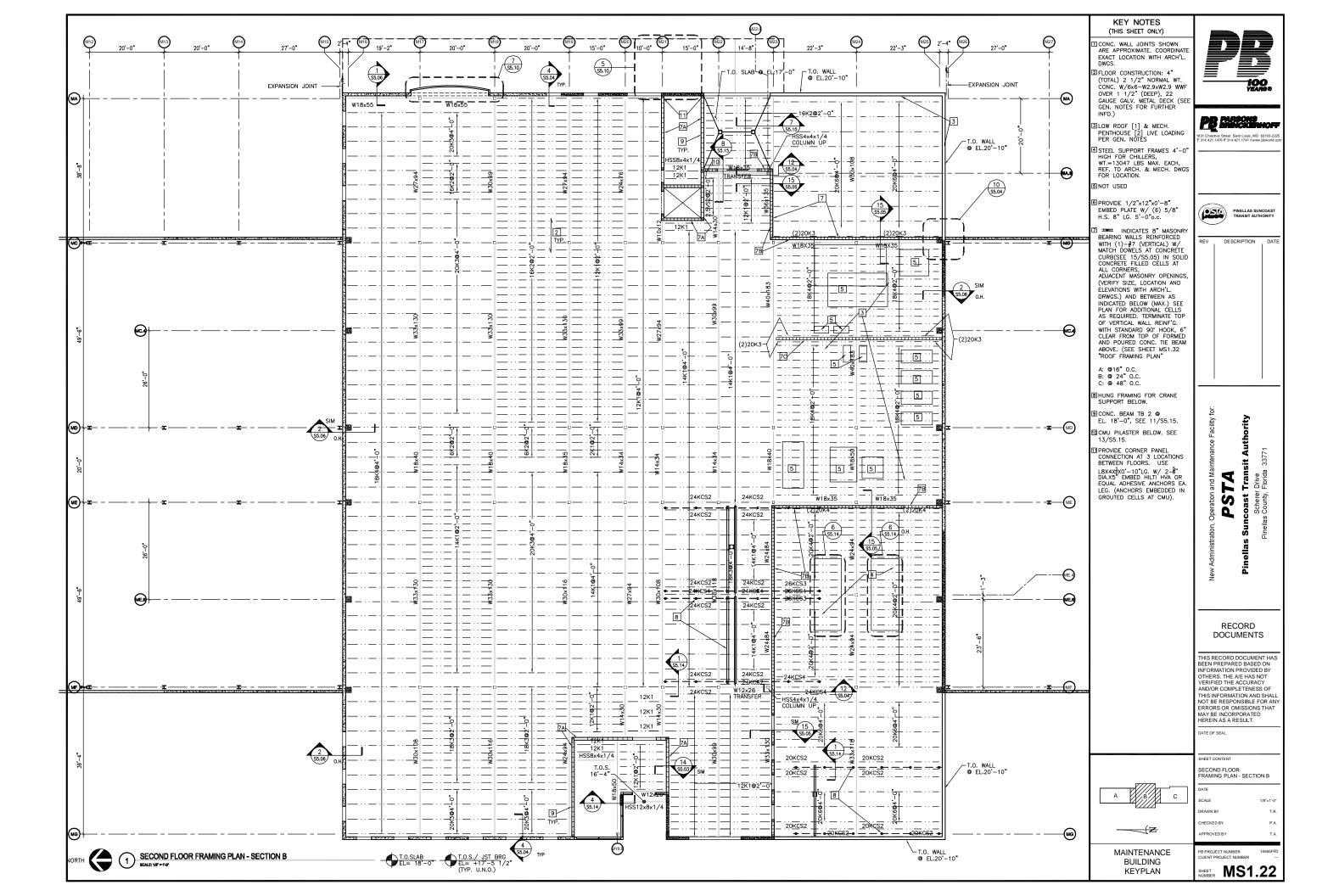
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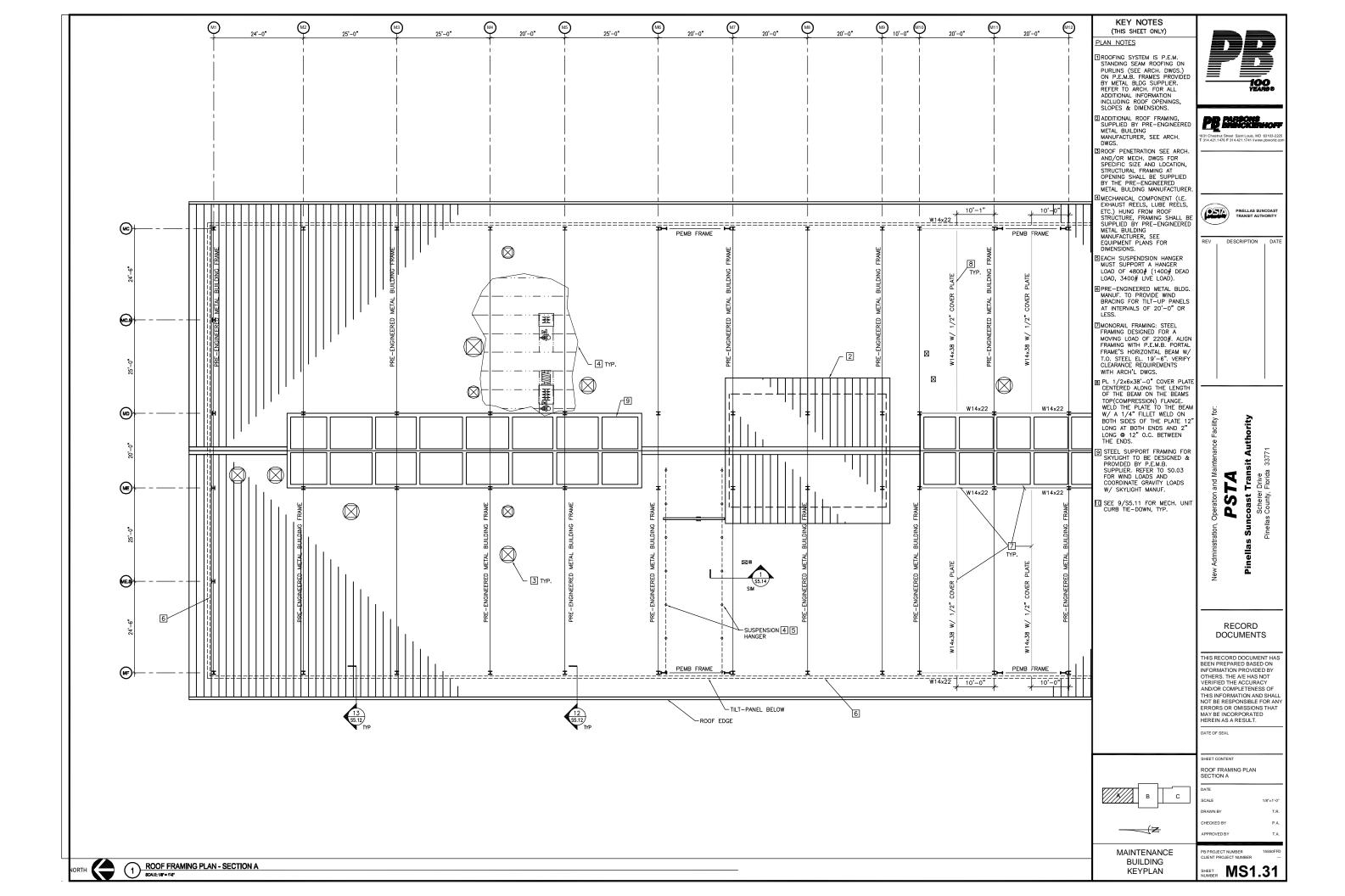
SHEET CONTENT
MECHANICAL EQUIPMENT
PLATFORM FRAMING
PLAN

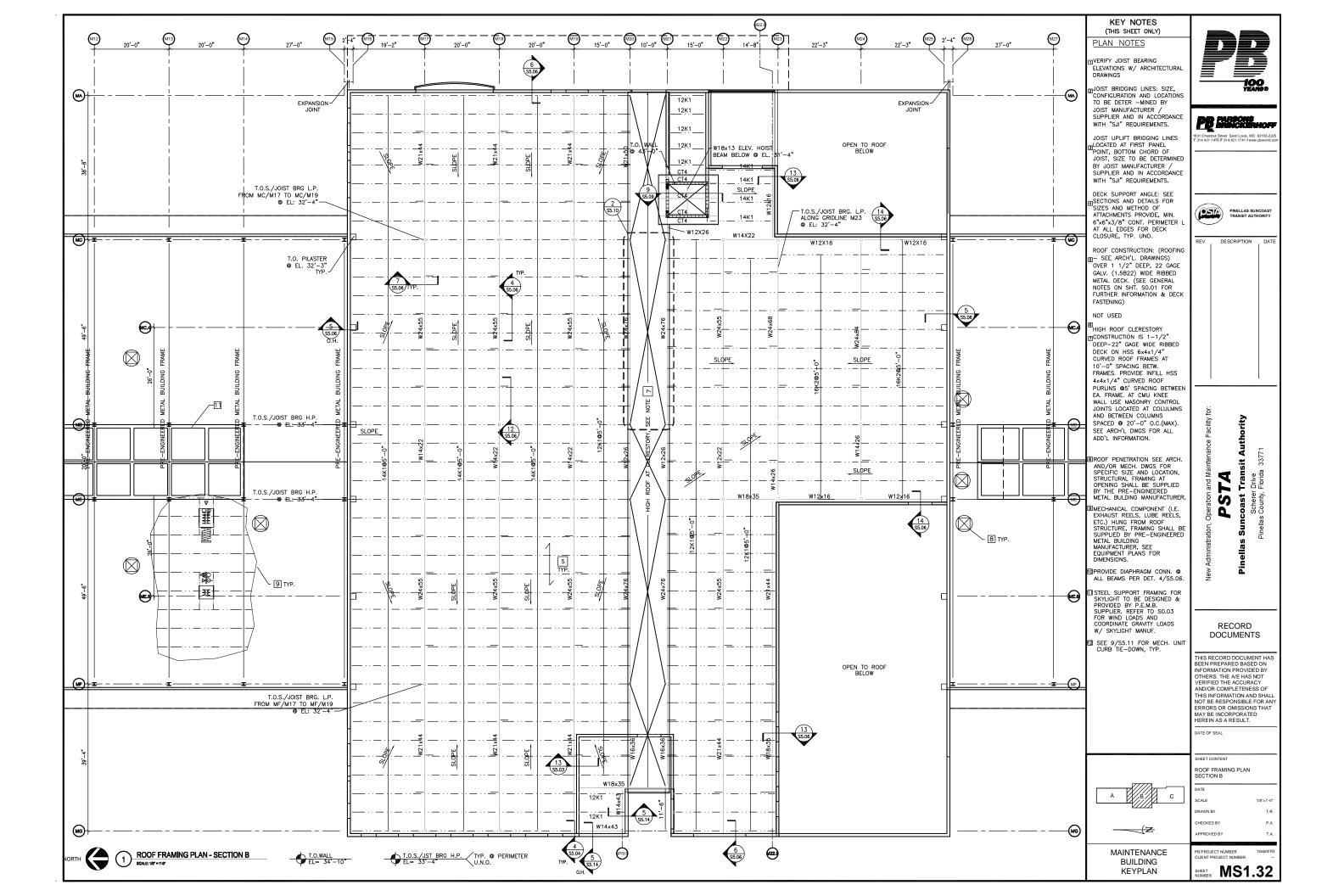
CHECKED BY

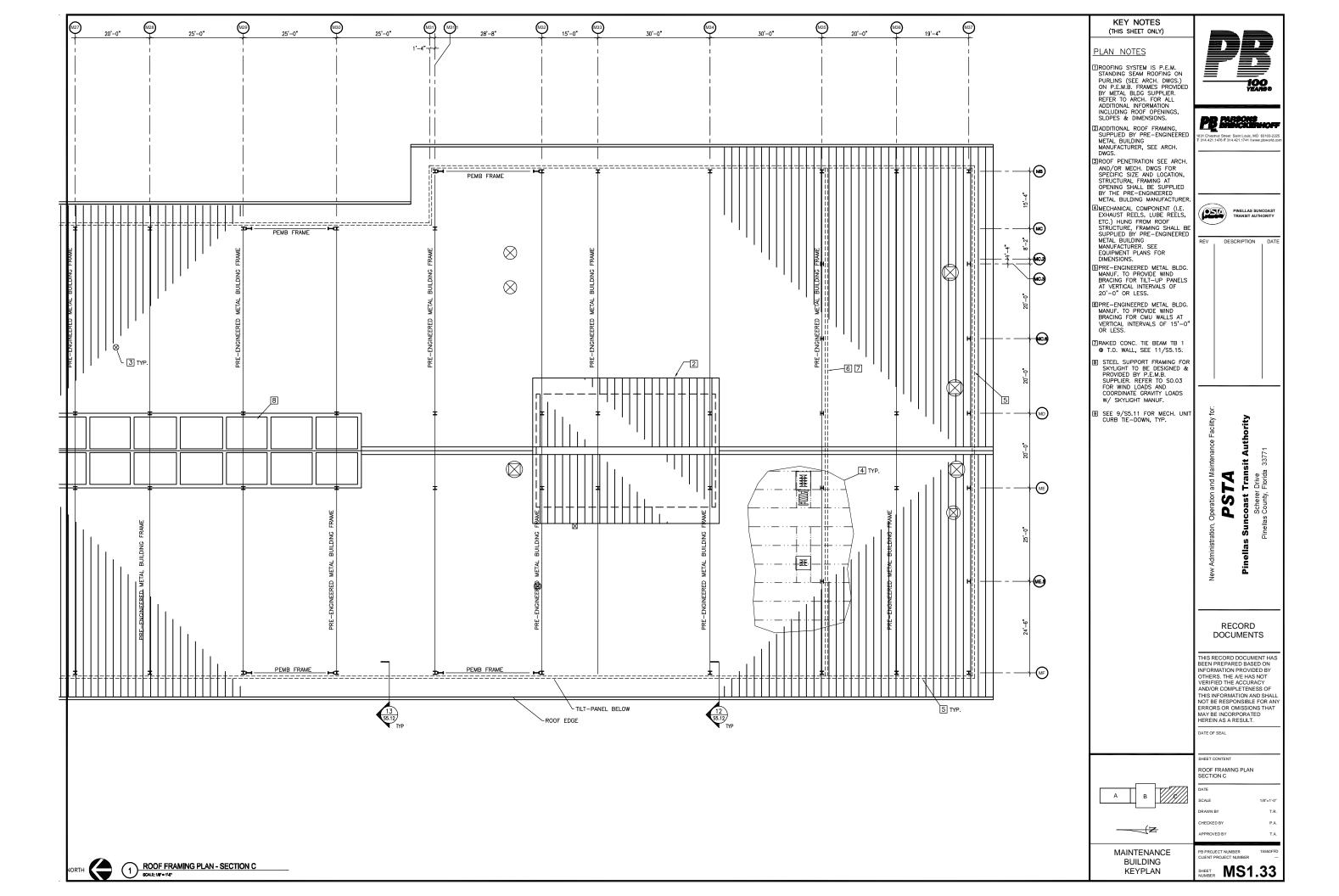
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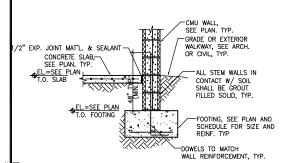
# TENSION DEVELOPMENT LENGTHS AND TENSION LAP SPLICE LENGTHS GRADE 60 REINFORCEMENT NORMAL WEIGHT CONCRETE | F'c = 3000|F'c = 4000 | F'c = 5000 | F'c = 6000 | F'c = 7000|F'c = 8000 | | 1.0Lg | 1.3Lg | 1.3L

MIE ADOVE DEVELOPMENT LENGTH AND LAP SPLICE LENGTH TABLE SHALL BE USED FOR HORIZONTA EMBERS (BEAMS, JOISTS, SLABS ETC.). USE FOR VERTICAL MEMBERS UNLESS NOTED OTHERWISE ON LAN OR DETAIL. THE ABOVE DEVELOPMENT LENGTH AND LAP SPLICE LENGTH TABLE SHALL BE LISED FOR HORIZONTAL

Ld = TENSION DEVELOPMENT LENGTH.
ALL SPLICE LENGTHS ARE IN INCHES.

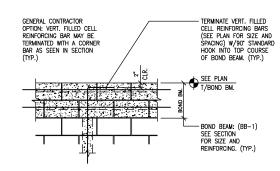
FOR TOP BARS, MULTIPLY SPLICE LENGTHS BY 1.30. TOP BARS REQUIRE A MINIMUM OF 12" FRESH INCRETE BELOW BAR.

13 TYPICAL TENSION DEV. AND SPLICE LENGTHS

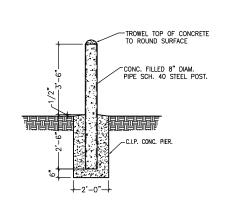


FER TO PLAN FOR SIZE AND SPACING VERTICAL REINFORCING. TYP.

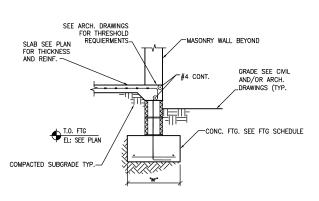
9 TYPICAL EXT. WALL FOOTING DETAIL



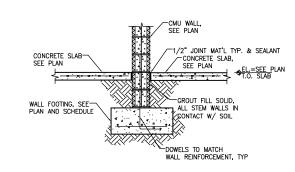
5 TYPICAL REBAR AT BOND BEAM



TYPICAL BOLLARD DETAIL

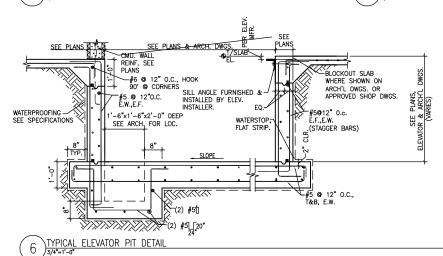


(14) TYPICAL EXT. WALL FOOTING DETAIL @ THRESHOLD



10 TYPICAL INTERIOR WALL FOOTING DETAIL

#5x3'-0"LG. CORNER BAR TOP AND BOT. TYP.-



-30"x30" PISTON BLOCK-OUT |

-(8) #5x4'-0" TOP &

TYPICAL REINF.@ ELEV. PISTON BLOCK-OUT.

PISTON HAS BEEN INSTALLED)

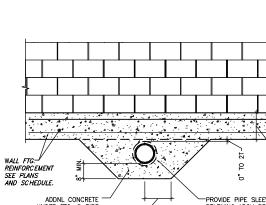
BRACE WALL AS REQUIRED UNTIL UPPER
 SLAB IS POURED AND HAS REACHED 2500 PSI MIN.

SLAB IS POURED AND HAS REACHED 2500 PSI MII

2. SLOPE PIT BOTTOM SLAB 1" TO SUMP PIT

3. GENERAL CONTRACTOR TO COORDINATE ELEVATOR
PIT SIZE, DEPTH, SILL DETAIL AND OTHER
REQUIREMENTS WITH ELEVATOR'S MFR'S
APPROVED SHOP DWGS.

4. PROVIDE GALVANIZED METAL LADDER IN TO
LOCATION AWAY FROM SUMP AS FOLLOWS:
HAND HOLD: 1 1/2"x1/4" BARS
RINGS: 1/2 ROUND.



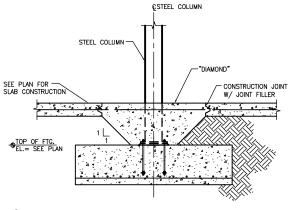
ADDITIONAL REINF. SEE SCHEDULE BELOW — VERT.CONSTRUCTION JOINT	L/2		ED SLAB REINF. DUS THRU JOINT
	72 2	2.00	

ADDN'L.REINFORCEMENT SCHED.  Fy = 60 ksi					
SLAB THICKNESS	REINFORCEMENT				
T ≤ 4"	#4 @ 18 x 2'-6"				
4" < T ≤ 6"	#4 @ 12 x 2'-6"				
6" < T <_8"	#5 @ 12 x 3'-0"				
8" < T < 12"	#6 @ 12 x 3'-8"				

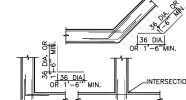
NOTES:

1. CONSTRUCTION JOINTS SHOULD BE LOCATED ON THE MIDDLE THIRD OF THE SLAB SPAN. 2. CONCRETE SURFACE AT CONSTRUCTION JOINT SHALL BE CLEAN AND FREE OF LAITANCE.

15) TYPICAL CONSTRUCTION JOINT IN STRUCTURAL SLAB.



1 1) STEEL COL. TO ISOLATED FOOTING DETAIL



FOOTING SCHEDULE

REINFORCEMENT

воттом

5#5 FW

7-#5 L.W. 5-#5 S.W 6#5 EW

5#6 EW

6#6 EW

7#5 EW

6-#6 E.W.

5-#6 L.W. 4-#6 S.W 7-#6 E.W.

5-#6 L.W. 3-#6 S.W. 8-#6 E.W.

8-#7 E.W.

9#6 EW

10#6 EW

10#8 EW

10#8 EW

#6@12'-0"o.c. EW

#4@12" o.c. EW

(2)\_#5 CONT.
#39 24°.c. TRANSVERSE
(3)\_#5 CONT.
#39 24°.c. TRANSVERSE
(4)\_#5 CONT.
#49 12°.c. TRANSVERSE
(5)\_#5 CONT.
#49 12°.c. TRANSVERSE
(5)\_#5 CONT.
#49 12°.c. TRANSVERSE
(4)\_#5 CONT.
#49 12°.c. TRANSVERSE
(4)\_#5 CONT.
#49 18°.c. TRANSVERSE

REINFORCEMENT

8-#6 E.W.

10#6 EW

#6@12'-0"o.c. EW

#4@12" o.c. EW

SIZE

WIDTH x LENGTH x DEPTH

4'-0"x4'-0"x1'-0"

4'-0"x7'-0"x1'-2

5'-0"x5'-0"x1'-0"

6'-0"x6'-0"x1'-0"

6'-0"x6'-0"x1'-3"

6'-6"x6'-6"x1'-0"

7'-0"X7'-0"X1'-0"

7'-0"x6'-0"x1'-0"

7'-0"X7'-0"X1'-6"

7'-0"x4'-6"x1'-0"

8'-0"x8'-0"x1'-6"

8'-0"x7'-0"x1'-4"

9'-0"x9'-0"x1'-6"

9'-6"x9'-6"x1'-6"

11'-0"x11'-0"x1'-8"

\* \* x1'-4"

13'-6"x9'-0"x1'-6"

17'-0"x8'-6"x1'-0"

2'-0"xCONT.x12"

2'-6"xCONT.x12"

3'-0"xCONT.x12"

4'-0"xCONT.x12"

4'-4"xCONT.x12" 5'-6"xCONT, X12"

\* FOR TYP. FTG. 'F' SEE DET. SLAB FTG. 'SF' SEE DET. \*\* IRREGULAR SHAPE. FOR DIMENSIONS SEE PLAN

MARK

F4.0

F4.0A

F5.0

F6.0

F6.0A

F6.5

F7.0

F7.0A

F7.0B

F7.0C

F7.5

F8.0

F8.0A

F9.0

F9.5

F10.0

F11.0 F12.0

F13.5

F17.0

WF-2.0

WF-2.5

WF-3.0

WF-4.0

WF-4.33

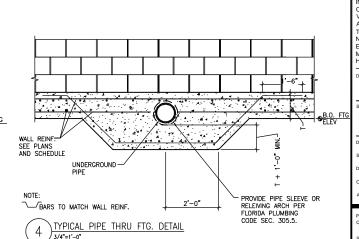
WF-5.5

12) FOOTING SCHEDULE

1.TYPICAL DETAIL OF SPLICES FOR HORIZONTAL REINFORCEMENT AT ALL CORNERS AND INTERSECTIONS OF ALL FOOTINGS UNLESS OTHERWISE INDICATED.

2. SPLICE BARS TO MATCH HORIZONTAL

8 TYPICAL REINFORCEMENT CORNERS/INTERSECTIONS









DESCRIPTION

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RECORD DOCUMENTS

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DATE OF SEAL

STRUCTURAL DETAILS

T.R CHECKED B

PROJECT NUMBER

S5.01

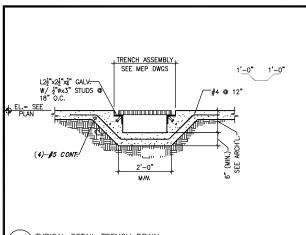
P.A.

B.O. FTG ELEV

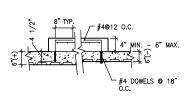
TYPICAL PIPE UNDER FTG. DETAIL

-(8) #5x2'-0" DOWELS MID DEPTH (PLACE & GROUT FILL AFTER ELEV.

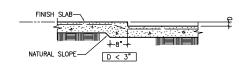
-PROVIDE PIPE SLEEVE OR RELEIVING ARCH PER FLORIDA PLUMBING CODE SEC. 305.5. ADDNL CONCRETE UNDER FTG. @ PIPE

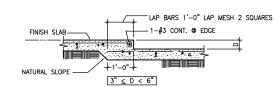


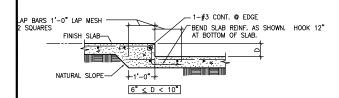
(13) TYPICAL DETAIL TRENCH DRAIN



9 TYPICAL HOUSE KEEPING PAD







COORDINATE DEPTHS & LOCATIONS OF ALL FLOOR DEPRESSIONS WITH ARCHITECTURAL DRAWINGS. . PROVIDE 1-#4x 4'-0" TOP AT INTERIOR CORNERS OF ALL DEPRESSIONS. SLAB DEPRESSIONS ARE TYPICALLY SHOWN ON PLAN THUS:  $x^{2^{-}}$ 

TYPICAL S.O.G. FLOOR DEPRESSIONS

## CONSTRUCTION JOINT NOTES

SEE PLAN FOR SLAB THICKNESS (T) AND REINFORCEMENT.
2. SLAB REINFORCEMENT SHALL BE CHAIRED BY SOIL SUPPORTED SLAB BOLSTERS 3'-0"O.C. EA. WAY. 3. BREAK BOND BETWEEN NEW AND PREVIOUSLY
PLACED SLAB BY SPRAYING OR PAINTING EXPOSED SIDE OF
SLAB AND DOWEL WITH A CURING COMPOUND, ASPHALTIC
EMULSION, OR FORM OIL

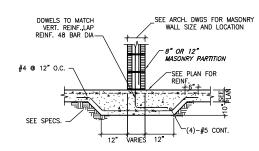
EMULSION, OR FORM OIL.

4. REFER TO GENERAL NOTES, GENERAL SPECIFICATIONS,
AND DRAWINGS FOR SUB-FLOOR DRAINAGE SYSTEM,
SUBGRADE PREPARATION AND/OR MUD SLAB REQUIREMENTS.
5. SUBGRADE SHALL BE FREE OF STANDING
WATER AT THE TIME OF CONCRETE PLACEMENT.

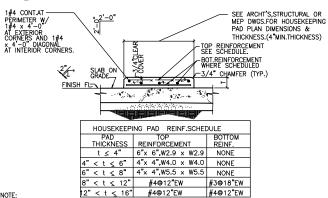
6. LONG STRIP CONSTRUCTION METHOD SHALL BE USED IN PLACING CONCRETE FOR ALL SLABS ON GRADE. SEE SCHEMATIC PLAN FOR CONCRETE

1. PROVIDE CONTROL AND/OR CONSTRUCTION
JOINTS AT EVERY COLUMN LINE AND IN
BETWEEN THE COLUMN LINES SUCH THAT
THE JOINT SPACING DOES NOT EXCEED 15
FEET, UNLESS OTHERWISE SHOWN ON THE
DEPARTMENT

14) TYPICAL CONSTRUCTION AND CONTROL JOINTS



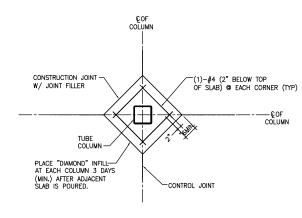
10 TYPICAL THICKENED SLAB UNDER LOAD BEARING CMU



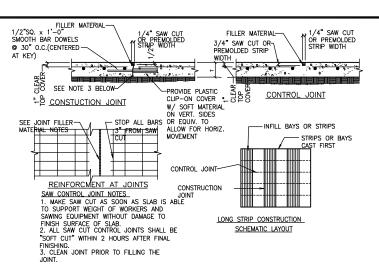
NOTE:

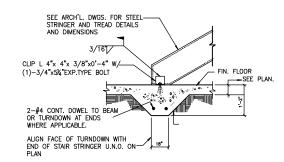
1. GENERAL CONTRACTOR TO COORDINATE WITH MECHANICAL DRAWINGS AND SPECS. TO DETERMINE REQUIREMENTS FOR HOUSEKEEPING PADS OVER SLAB ON GRADE AND PROVIDE WHERE REQUIRED WHETHER SHOWN ON STRUCTURAL DRAWINGS OR NOT. COORDINATE DIMENSIONS AND OTHER SPECIAL REQUIREMENTS WITH EQUIPMENT MANUFACTURERS AS REQUIRED.

TYPICAL HOUSEKEEPING PAD ON S.O.G. 6



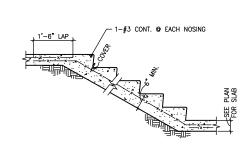
TYPICAL DIAMOND PLAN DETAIL FOR TUBE COL. & UTILITY BOX





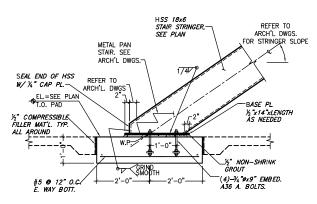
2. CLIP ANGLE TO BE HIDDEN BENEATH FIRST TREAD. WHERE OPEN RISERS ARE USED, COORDINATE THE LOCATION WITH THE ARCHITECT.

11) TYPICAL S.O.G. TURNDOWN AT STEEL STAIR



NOTES: 1. SEE ARCH'L. FOR STAIR

TYPICAL SLAB-ON-GRADE STAIRS



DETAIL OF EXTERIOR STAIR STRINGER T.O. FOUNDATION PAD

JOINT FILLER MATERIAL NOTES

1. FILLER MATERIAL USED SHALL HAVE A MINIMUM SHORE A HARDNESS OF 80, AND SHALL CONFORM TO ASTM D2240, JOINT FILLER SHALL BE APPROVED BY ENGINEER PRIOR TO APPLICATION. APPROVED JOINT FILLER IS MANUFACTURED BY METZGER MAGUIRE OR APPROVED EQUAL.

2. WHERE POSSIBLE, FILLER MATERIAL SHALL BE APPLIED WHEN BUILDING IS UNDER PERMANENT TEMPERATURE CONTROL. THIS SHALL BE EITHER AT THE END OF CONSTRUCTION OF THE COMPLETE BUILDING SHELL, OR A MINIMUM OF 90 DAYS AFTER SLAB CONSTRUCTION.

CONSTRUCTION.

3. FOLLOW STRICTLY THE MANUFACTURERS RECOMMENDED PROCEDURES FOR APPLYING THE JOINT FILLER.

FORMED CONTROL JOINT NOTES

1. FORM CONTROL JOINTS BY INSERTING PREMOLDED STRIP INTO FRESH
CONCRETE UNTIL TOP SURFACE OF STRIP IS FLUSH WITH SLAB SURFACE.

2. TOOL SLAB EDGES ROUND ON EACH SIDE OF INSERT.

DEBRIS.

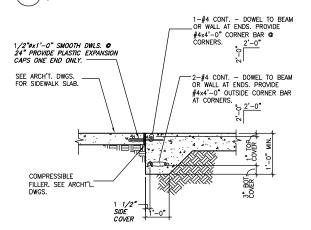
DOWEL NOTES

1. ALL DOWELS SHALL CONFORM TO ASTM A615.

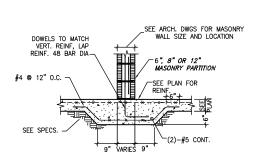
2. DOWELS SHALL BE CAREFULLY ALIGNED AND SUPPORTED DURING CONCRETING

-SEE ARCHITECTURAL DRAWINGS FOR WALL CONSTRUCTION (1)-#3 CONT. AT EDGE #3 CONT. @ 12" OUTSIDE FACE. #3 DOWELS @ 24" 4 1'-2" 10" < D < 2'-0" (2)-#5 CONT. WITH FACE OF WALL ABOVE.

(12) TYPICAL SLAB-ON-GRADE DEPRESSION
3/4"=1'-0"



TYPICAL TURNDOWN AT SLAB ON GRADE 8

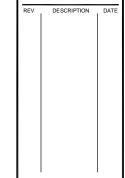


TYPICAL THICKEN SLAB UNDER NON-BEARING CMU









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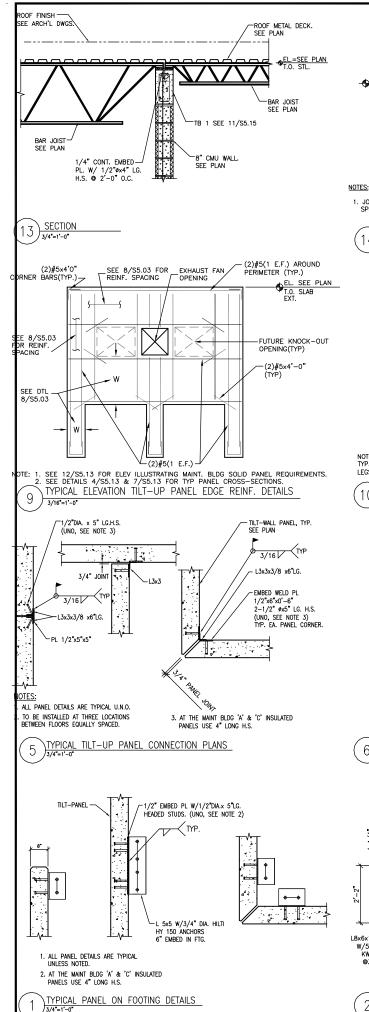
RECORD DOCUMENTS

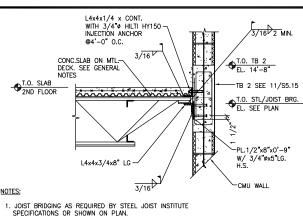
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DATE OF SEAL

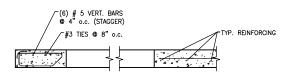
STRUCTRUAL DETAILS

CHECKED B P.A.



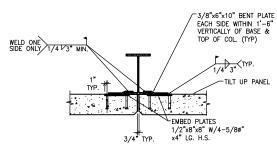


14 JOIST AT CMU WALL



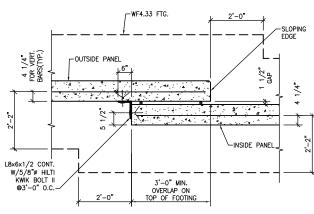
NOTE: TYP. SECTION FOR PANEL LEGS 2'-6" OR LESS

10 TYPICAL TILT-UP PANEL CONNECTIONS

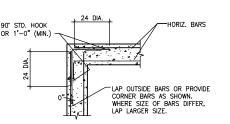


<u>PLAN</u>

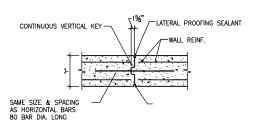
TYPICAL TILT-UP PANEL CONNECTION DETAIL @ COLUMN



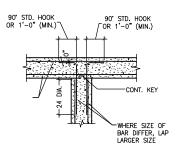
TILT-UP PANEL OVERLAP DETAIL REF: 6/AA3.2.1



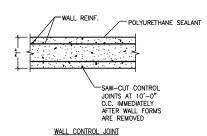
WALL CORNER



WALL CONSTRUCTION JOINT

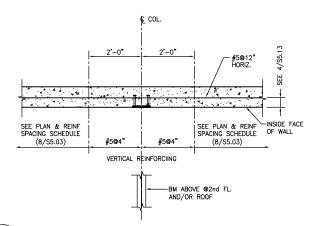


#### WALL INTERSECTION

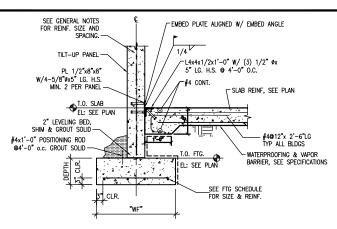


1. RETAINING WALL CONTROL JOINTS TO BE 10' - 0" O.C. UNLESS INDICATED OTHERWISE ON PLAN

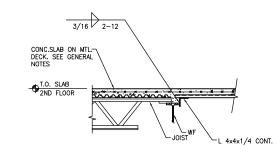
TYPICAL WALL DETAILS



TYP. REINFORCING @ COL. STRIPS OF TILT-UP PANEL



 $(16)^{\frac{1}{3}}$  PANEL ON 'WF' FTG. DETAIL



12) DECKING SUPPORT

SPANDREL WIDTH/ DEPTH	VERT/HORZ, REINF, MAX SPACING
W < 6'-0"	4" O.C.
6'-0" ≤ W < 10'-0"	8" O.C.
W ≥ 10'-0"	SEE PLAN

MAINTENANCE BLDGS "A" & "C"	
SPANDREL WIDTH/ DEPTH	VERT/HORZ. REINF. MAX SPACING
W < 6'-0"	8" O.C.
6'-0" < W < 10'-0"	10* D.C. (SEE NOTE 2)
W ≥ 10'-0"	SEE PLAN (SEE NOTE 2)

NOTES 1. SEE 4/55.13. FOR DEFINITION OF GROSS-SECTION.
2. APPELS TO INNER STRUIDEN, WITHE OF RESIATED PANEL M/ A SAGLE LAYER OF STRUCTURAL RENFORCEMENT.
3. SEE PLAN FOR OUTER (ARCH1) WITHE RENFORCEMENT OF INSLIATED PANEL.
4. SEE PLANTON 12/53.13 FOR REQUIRED RENFORMENT SOLV, ON/SILATED PANEL. ORIENTATION.

8 TYP. TILT-UP PANEL REINF. SPACING SCHEDULE











ority STA

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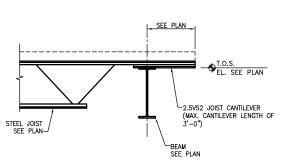
STRUCTURAL DETAILS

T.R P.A.

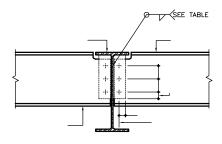
ALL BOLTS ~7" Ø A3250C @ BEAM TO GIRDER CONNECTION.

| SINGLE PLATE CONNECTION TABLE | 1"=1"-0"

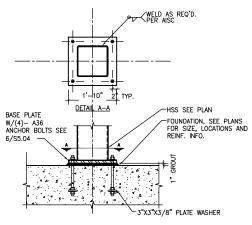
1"=1'-0"



9 TYPICAL JOIST TOP CHORD CANTILEVER EXTENSION



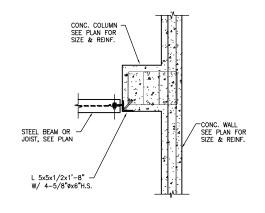
TYPICAL BEAM TO GIRDER CONNECTION 5 1"=1'-0"



TYPICAL HSS COLUMN BASE PLATE

< COLUMN HSS 8"x8" COLUMN-SEE 13/S5.04

BEAM CONNECTION TO COLUMN

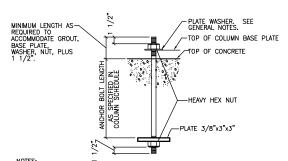


TYPICAL CORNER CONNECTION TO CONC. COLUMN 3/4"=1"-0"

COL SIZE	BASE PL SIZE	ANCHOR BOLT
HSS8x8	1"x16"x16"	7/8"ø x7" EMBED
HSS8x8	1 1/4"x16"x16"	7/8"ø x7 1/2" EMBED
HSS8x8	1"x16"x16"	7/8"ø x6" EMBED
HSS8x8	1"x16"x16"	7/8"ø x6" EMBED
HSS6x6	1 1/4"x14"x14"	7/8"ø x7 1/2" EMBED
HSS8x8	1 1/4"x16"x16"	7/8"ø x7 1/2" EMBED
HSS14x14	1 3/8"x22"x22"	1 1/4"ø x 13 1/2" EMBED
	HSS8x8 HSS8x8 HSS8x8 HSS8x8 HSS6x6 HSS8x8	HSS8x8 1"x16"x16" HSS8x8 1 1/4"x16"x16" HSS8x8 1"x16"x16" HSS8x8 1"x16"x16" HSS8x8 1"x16"x16" HSS6x6 1 1/4"x14"x14" HSS8x8 1 1/4"x16"x16"

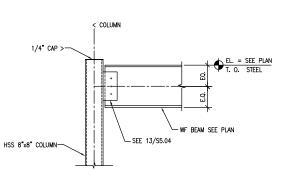
NOTE: SEE DETAILS 1/S5.04 & 2/S5.04 FOR ADD'L INFO.



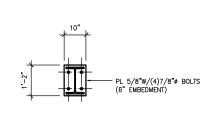


- SEE COLUMN SCHEDULE FOR ANCHOR BOLT SIZE.
   BOLT THREADS AT THE EMBEDDED END SHALL BE STRIKED AT TWO PLACES BELOW THE HEAVY HEX NUT.
- 3. UNLESS NOTED OTHERWISE, ALL BOLTS SHALL BE TIGHTENED TO A "SNUG TIGHT" CONDITION AS DEFINED BY AISC AFTER THE CONCRETE IS AT LEAST 14 DAYS OLD. THE HOLE IN THE PLATE WASHER SHALL BE 1/16" LARGER THAN THE BOLT DIAMETER.

TYPICAL TYPE B ANCHOR BOLT

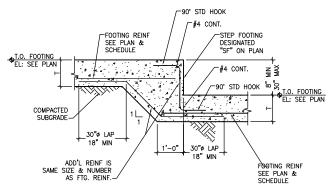


ROOF BEAM CONNECTION TO COLUMN



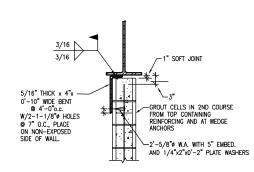
NOTE: FOR INFO NOT SHOWN SEE 1/S5.04 & 2/S5.04.

# 11) SOUTH ENTRY COLUMN BASEPLATE @ A4.1/ AF.1, AF.4, AF.8

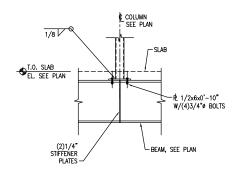


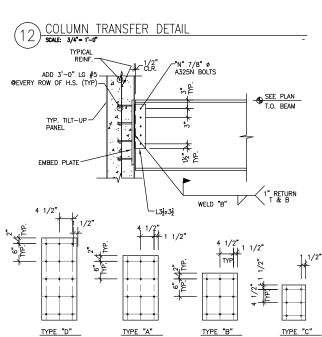
T = THICKNESS OF CONT. WALL FOOTING, SEE PLAN AND SCHEDULE.





TOP OF WALL BRACING (16)





PL ¾" x 12" x 2'-4" PL ¾" x 12" x 1'-10" PL %" x 12" x 1-4" PL %" x 8" x 1'-0" W/ 15- $\frac{3}{4}$ "ø x 5" HSA W/ 12- $\frac{3}{4}$ "ø x 5" HSA W/ 9- $\frac{3}{4}$ "ø x 5" HSA W/ 6- $\frac{3}{4}$ "ø x 5" HSA

BEAM SIZE	"N"	ANGLE SIZE	EMBED PL.	WELD "B"
W8-W10	2	(2) L3½"x 2½"x 5/16"	"C"	1/4
W12-W16	3	(2) L3½"x 3½"x 5/16	"B"	1/4
W18-W21	4	(2) L3½"x 3½"x 5/16"	"B"	1/4
W24-W30	5	(2) L3½"x 3½"x 3/8"	"A"	5/16
W24x94 & HEAVIER	6	(2) L3½"x 3½"x 3/8"	"D"	5/16
W33-W36	6	(2) L3½"x 3½"x 3/8"	"D"	5/16

USE 5/16" BENT PLATE @ SKEWED CONDITION.

TYPICAL "W" CONNECTION DETAIL TO CONCRETE



PB BARRONS



DESCRIPTION

STA 

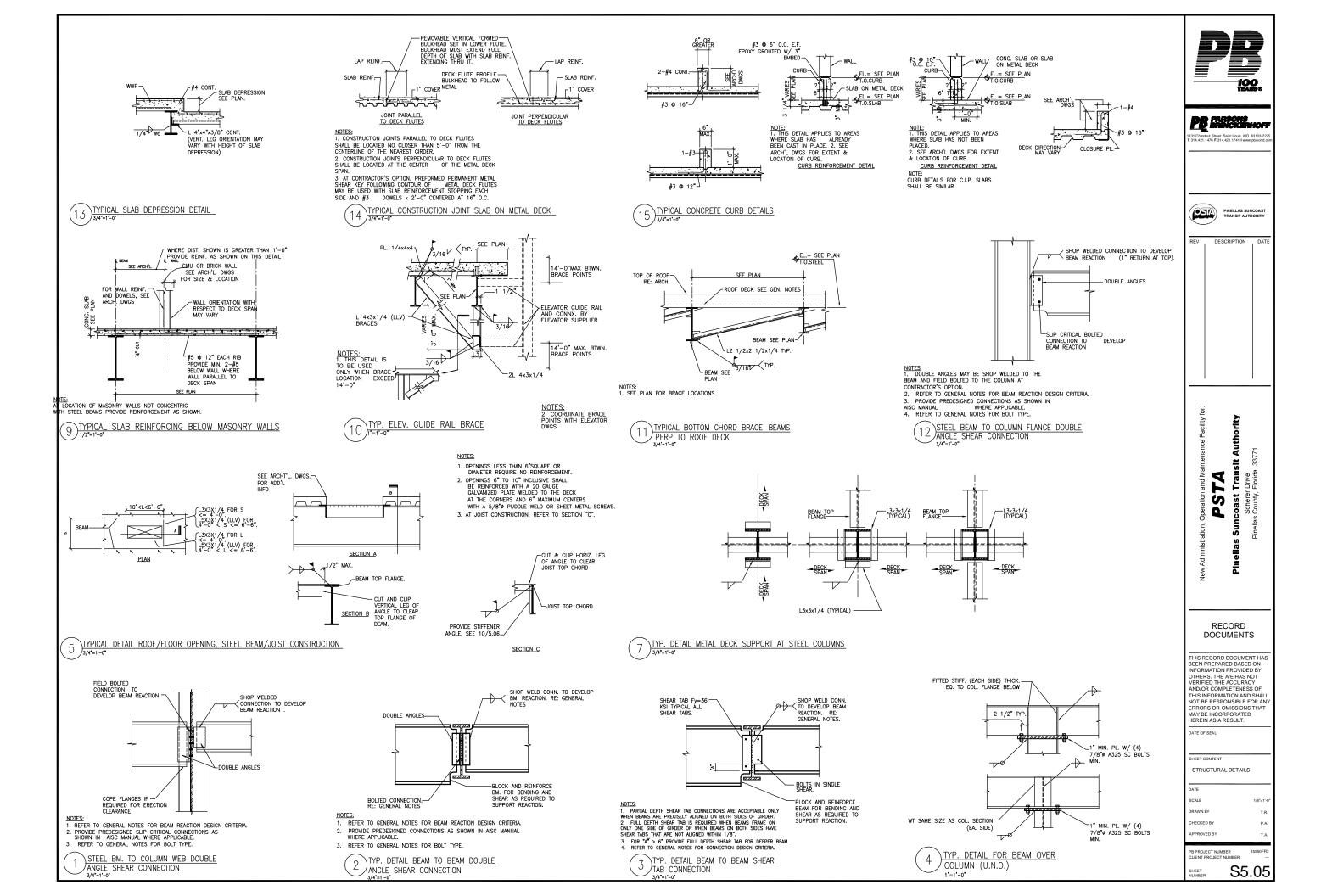
RECORD DOCUMENTS

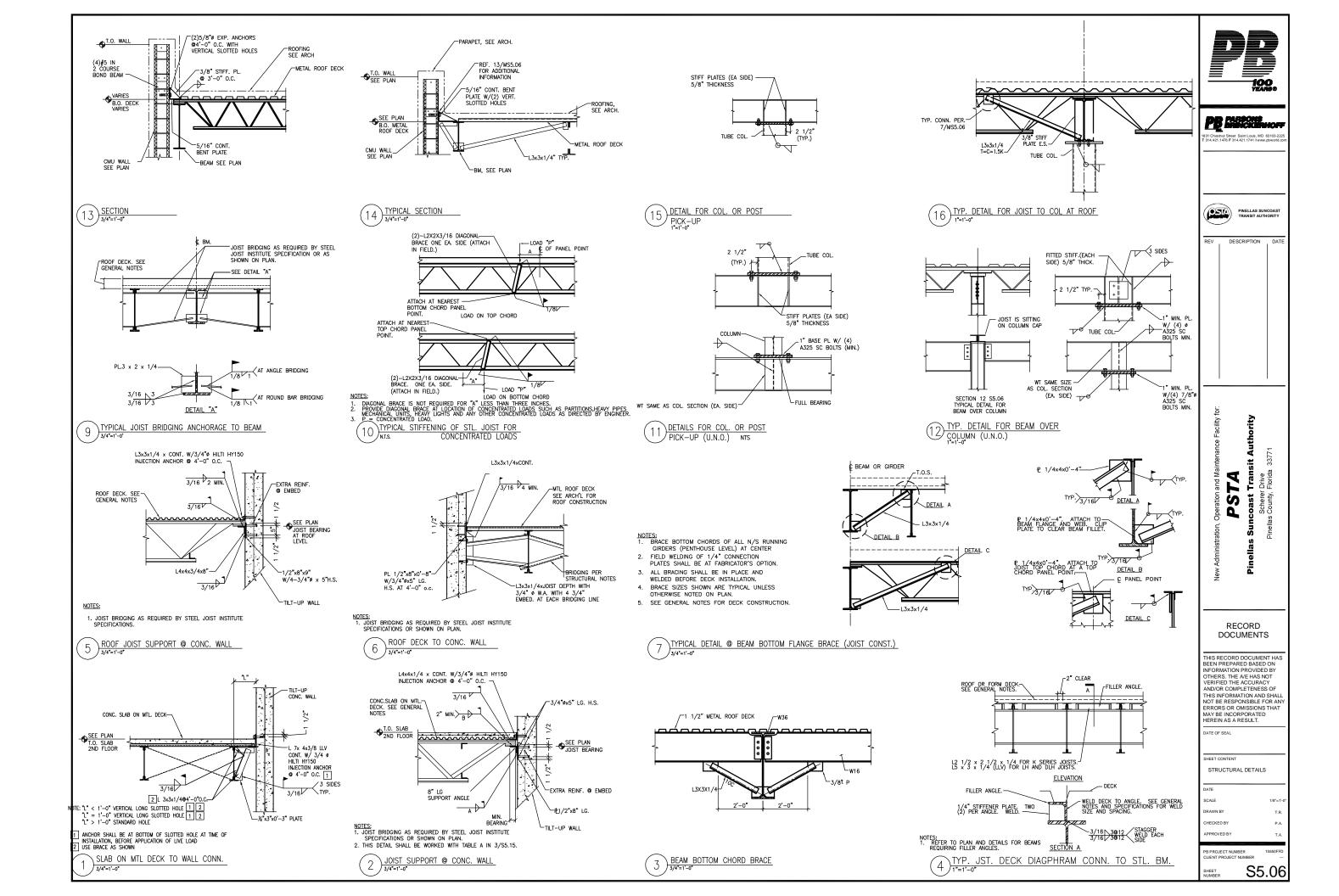
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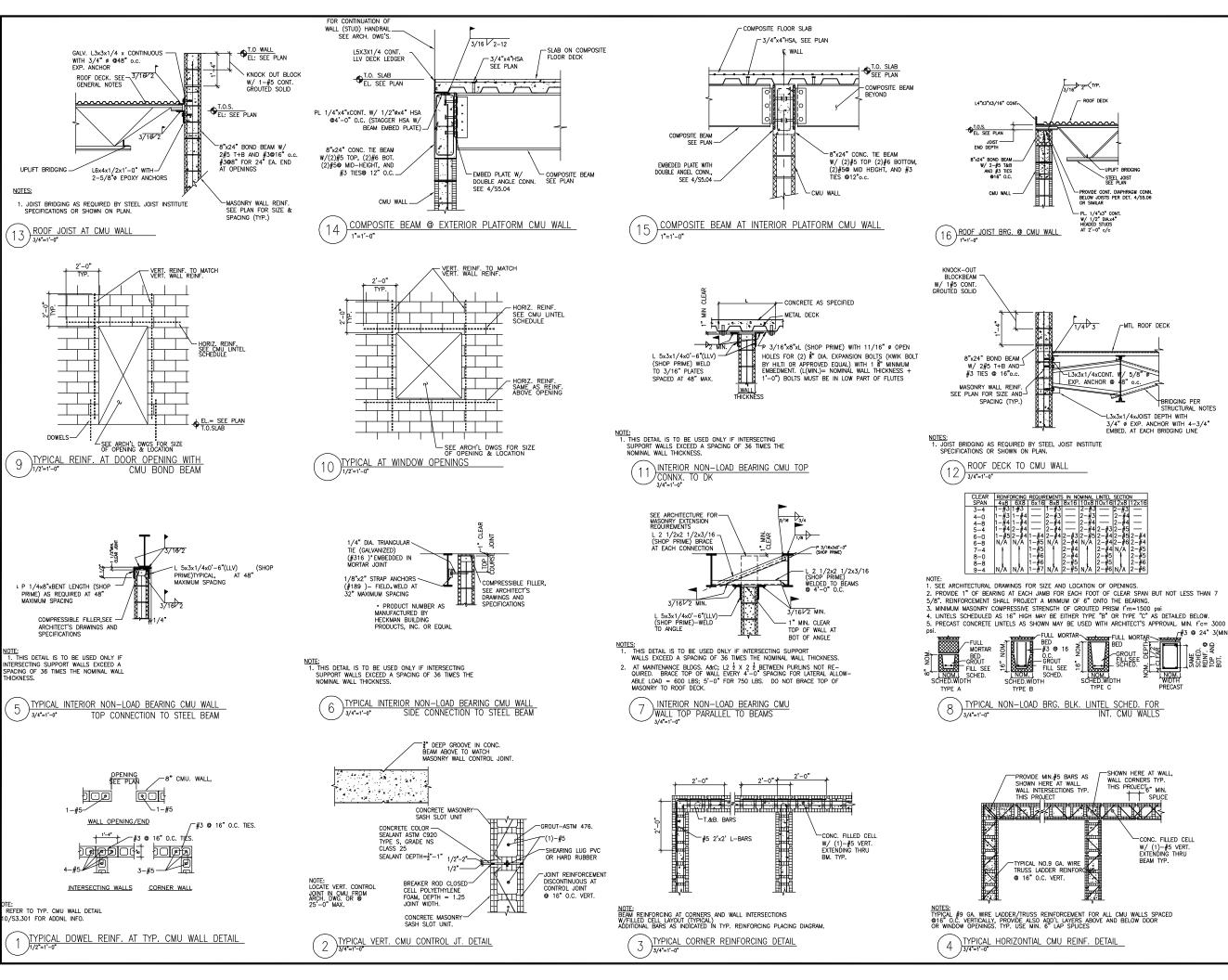
DATE OF SEAL STRUCTURAL DETAILS

T.R P.A.

PROJECT NUMBER







100

PB Bantons



PINELLAS SUNCOAST TRANSIT AUTHORITY

DESCRIPTION

STA

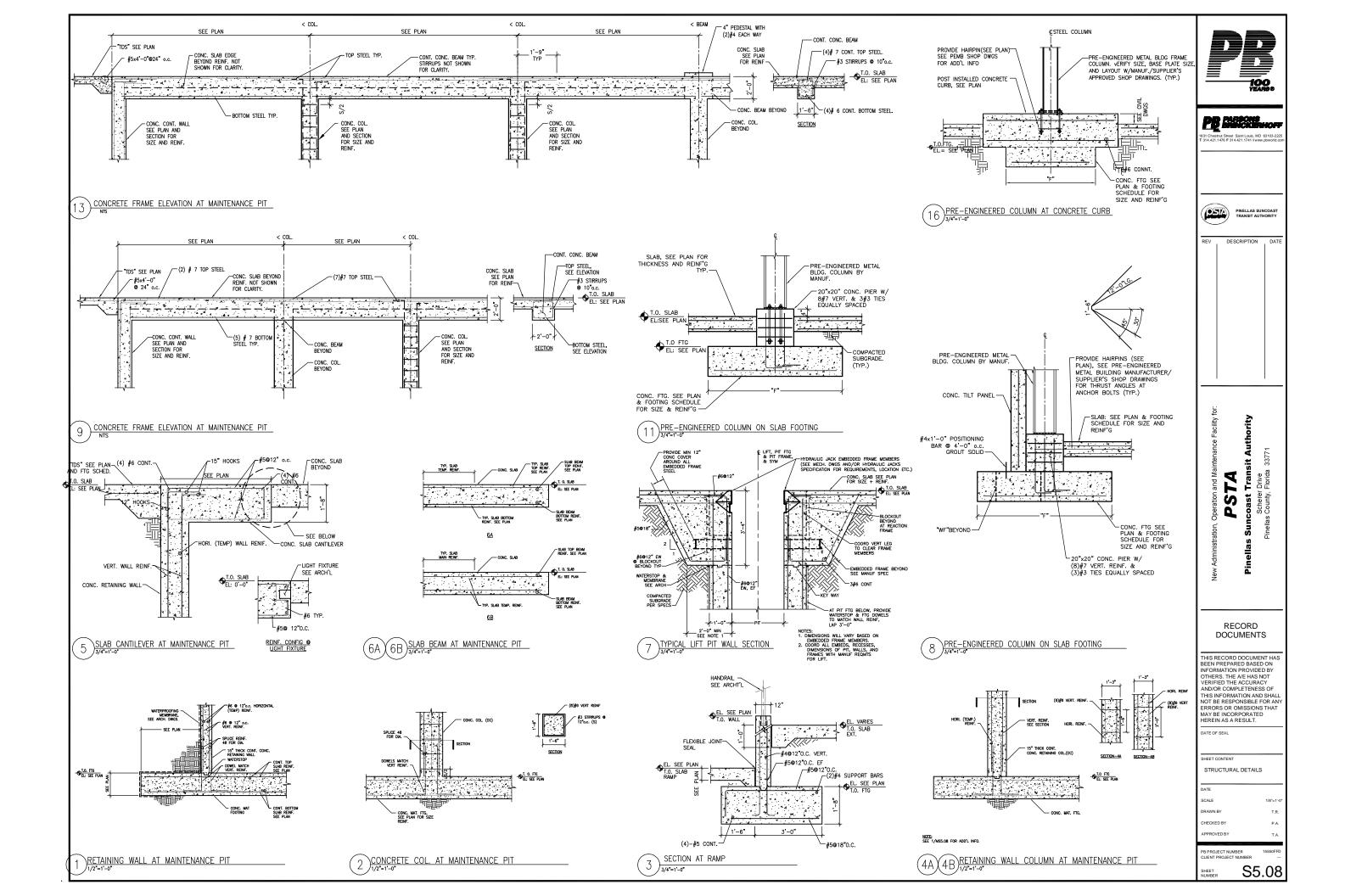
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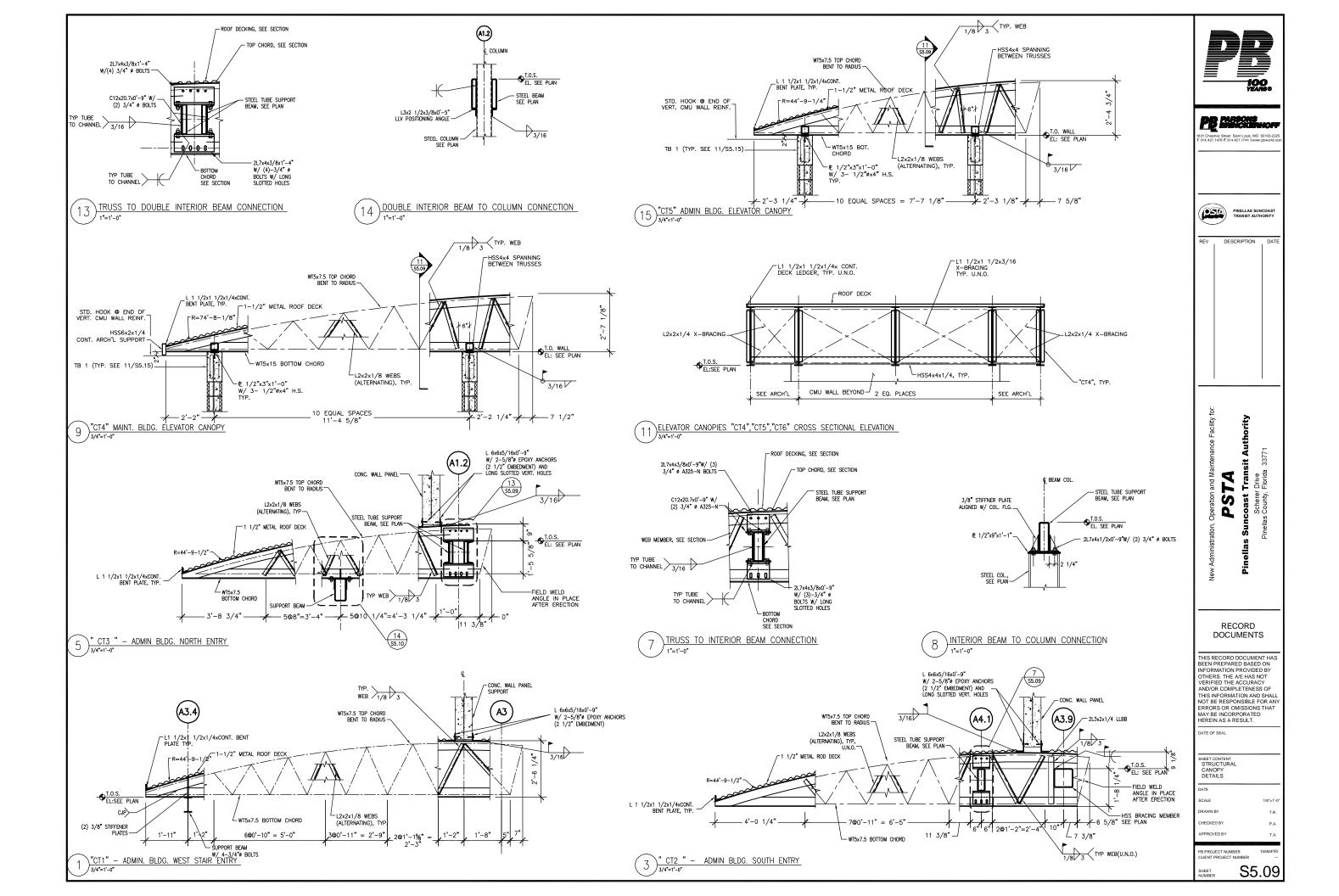
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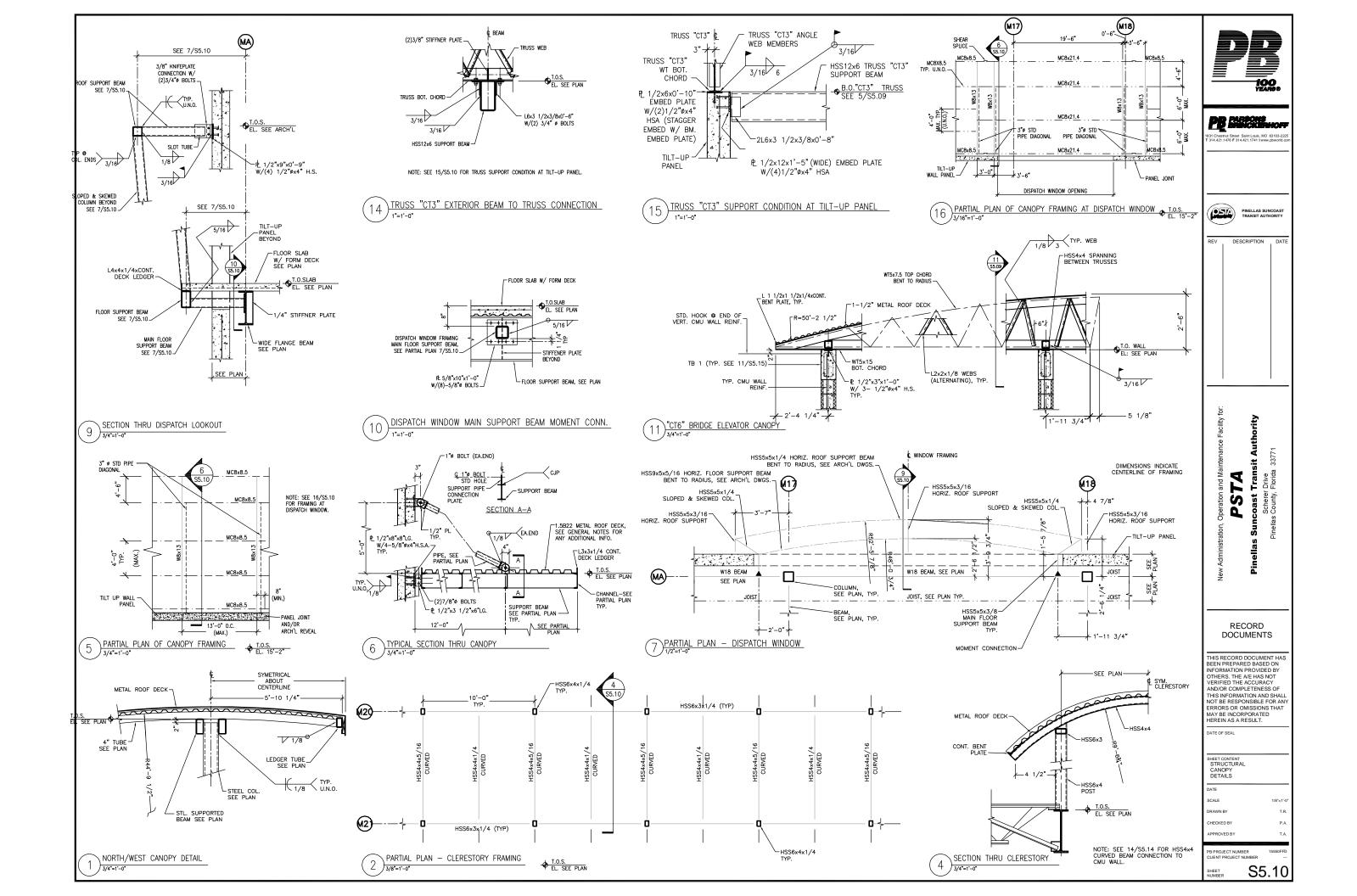
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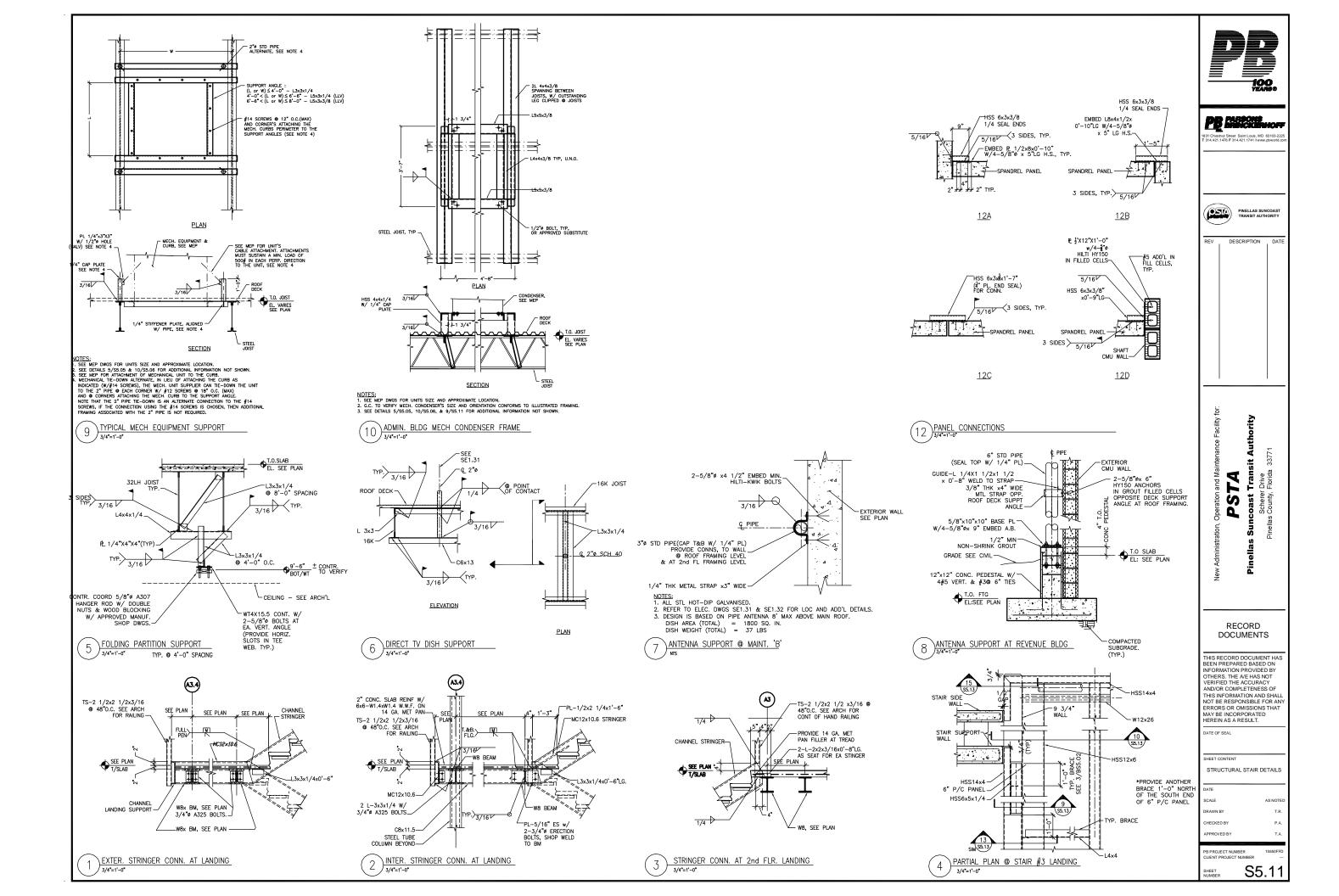
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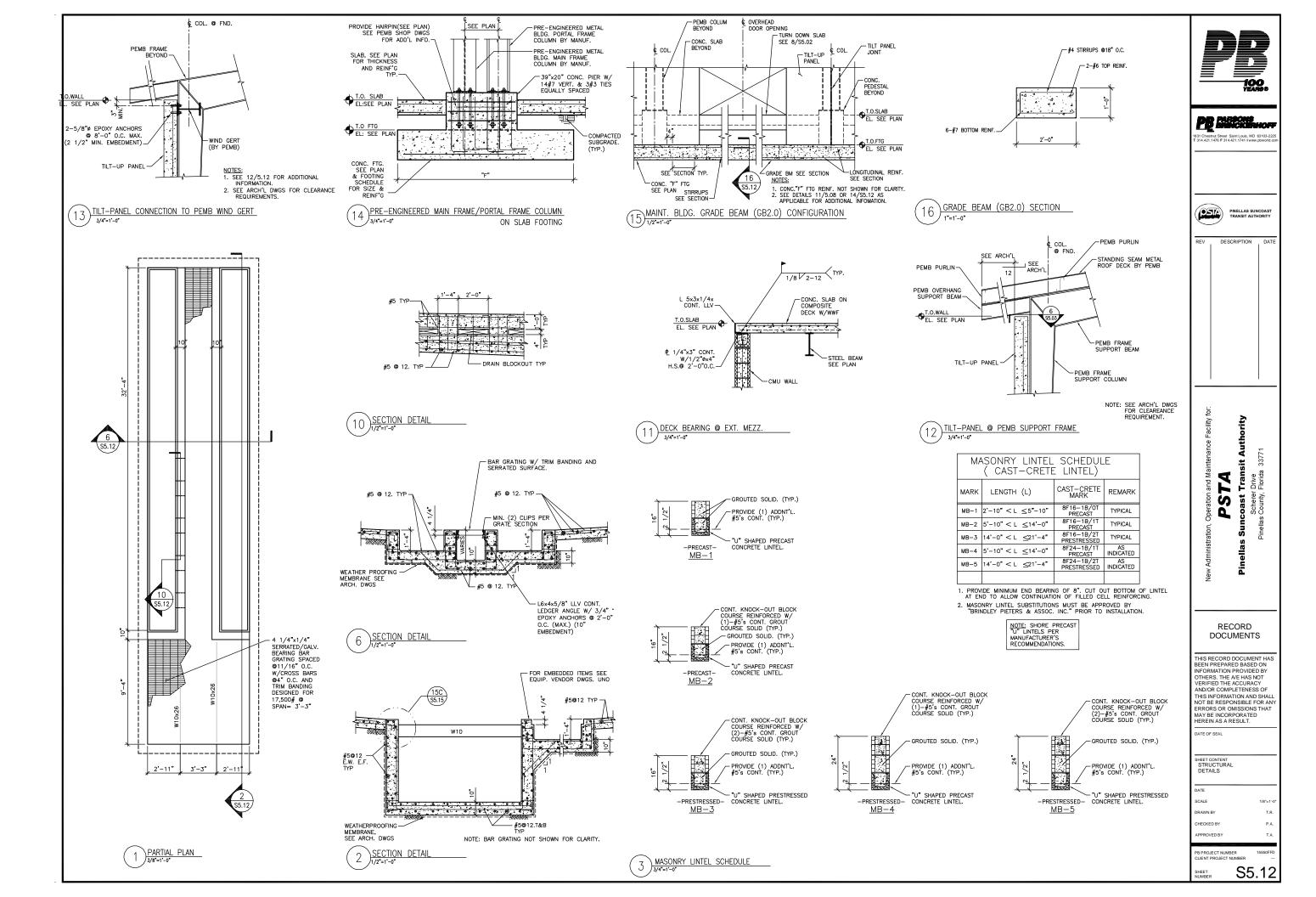
STRUCTURAL DETAILS

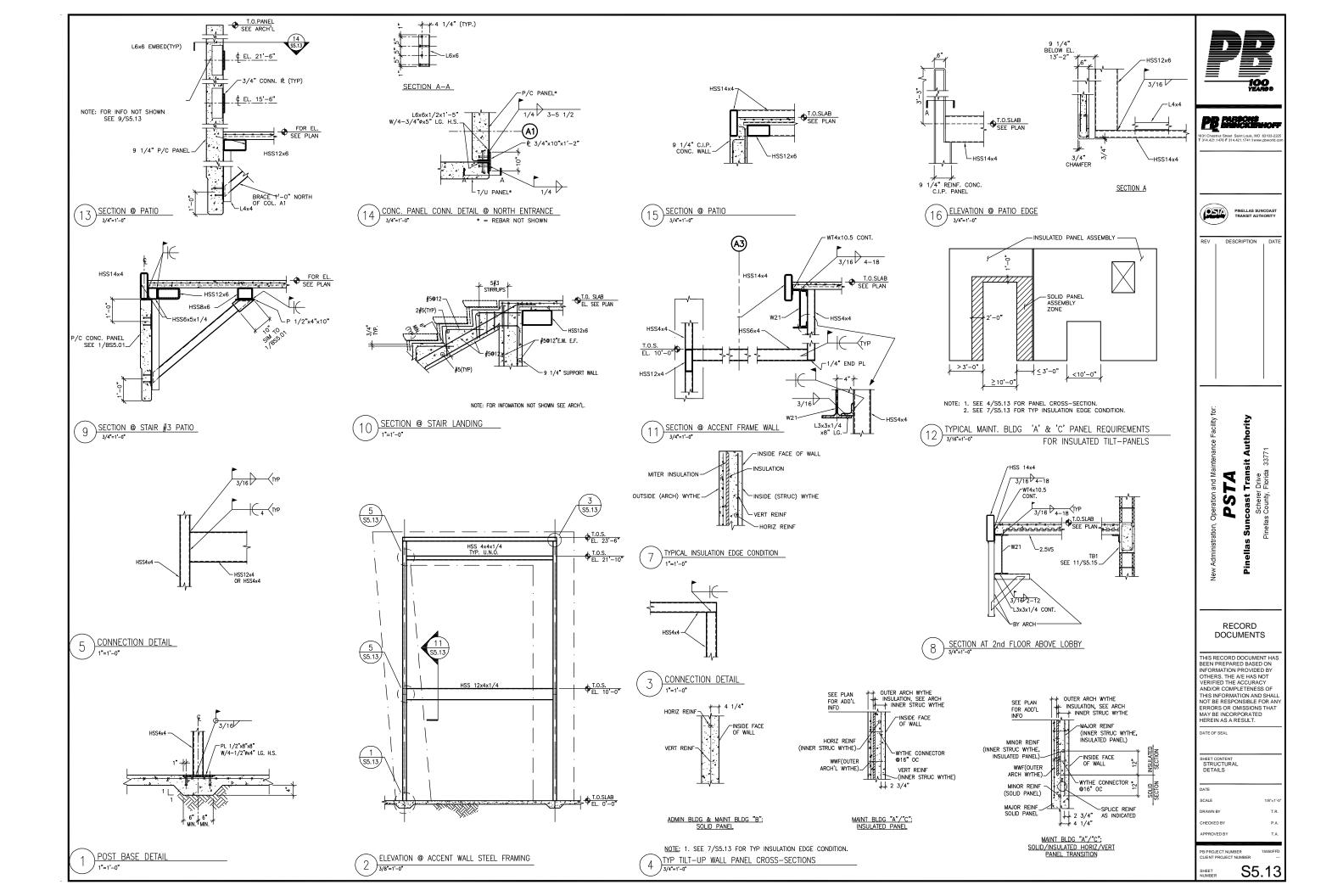


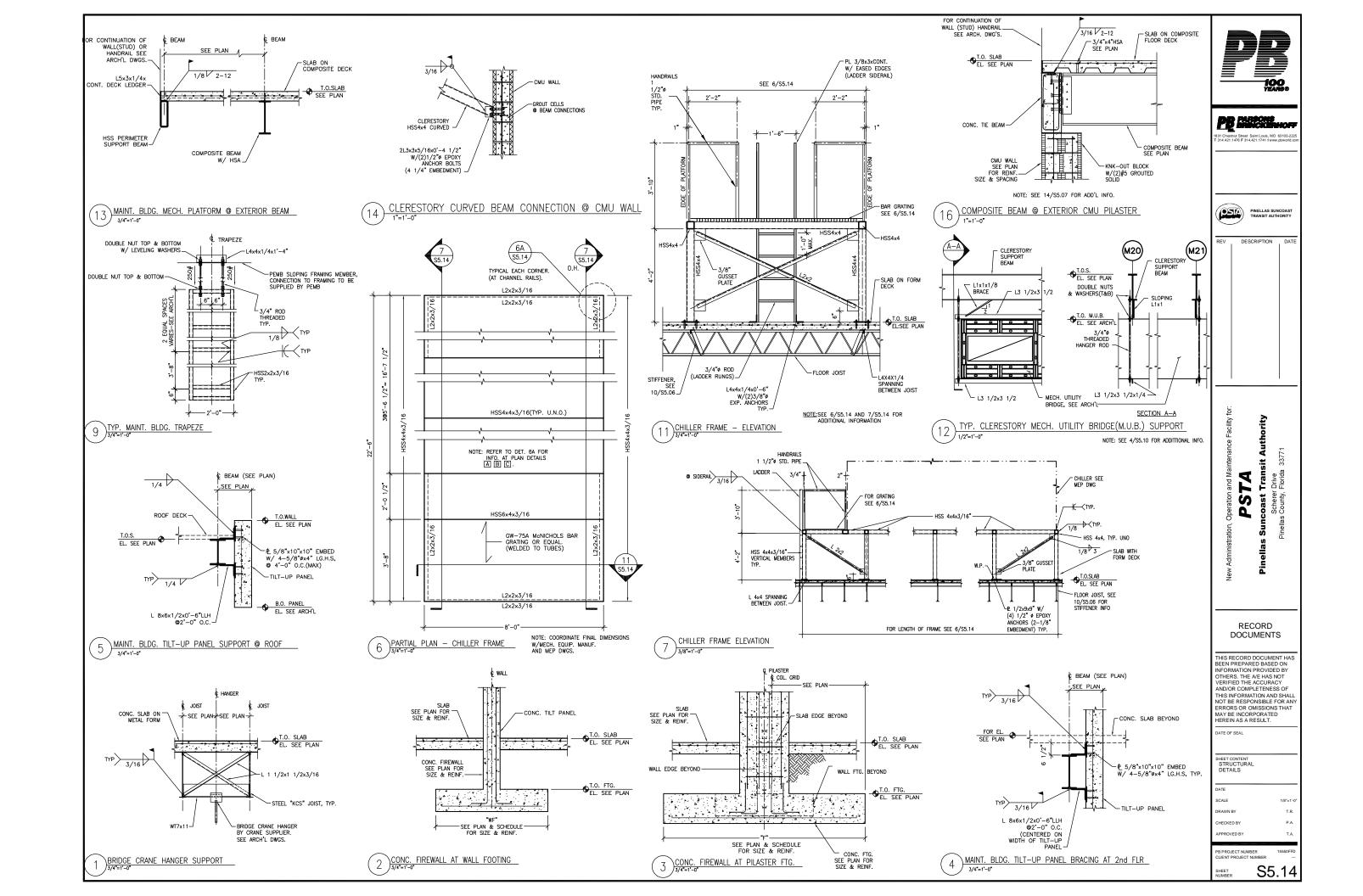


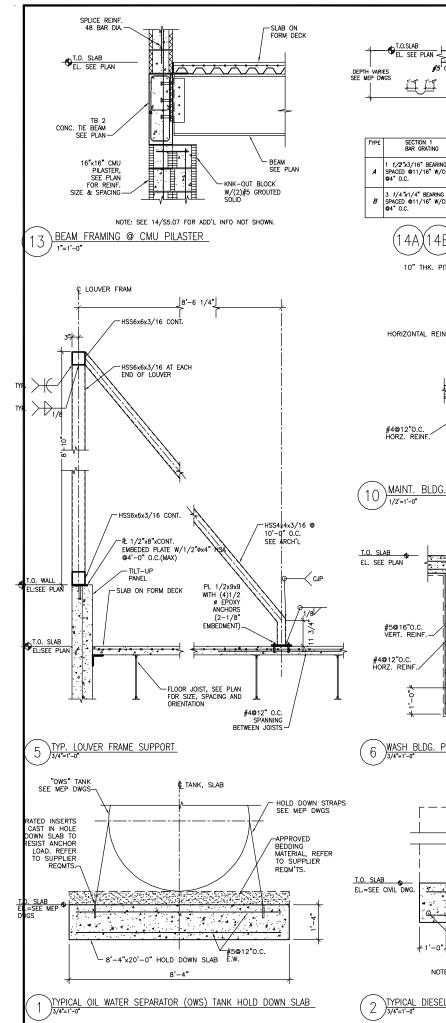






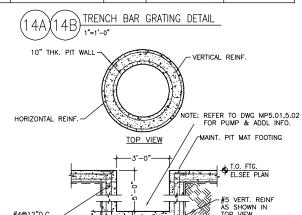






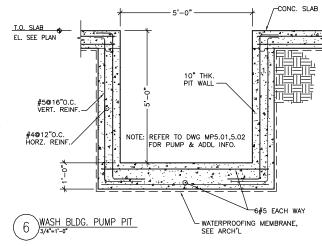


<i>T</i> YPE	SECTION 1 BAR GRATING	SECTION 2 RECESS DEPTH	SECTION 3 LEDGER ANGLE	SECTION 4 DESIGN LOAD
A	1 1/2"x3/16" BEARING BARS SPACED @11/16" W/CROSS BARS @4" O.C.	1 1/2"	L3x3x3/8 CONT. W/1/2 # EPOXY ANCHORS 2'-0" O.C.(MAX.), (4 1/4" EMBEDMENT)	W=230 PSF @SPAN= 5'-0"
В	3 1/4 1/4" BEARING BARS SPACED @11/16" W/CROSS BARS @4" O.C.	3 1/4"	L4x4x3/B CONT. W/1/2 of EPOXY ANCHORS 2'-0" O.C.(MAX.), (4 1/4" EMBEDMENT)	P=4250# @SPAN= 5'-0"



WATERPROOFING MEMBRANE,

SEE ARCH'L \MAINT. BLDG. PIT — PUMP PIT



	DIESEL TANK SEE MEP DWGS
T.O. SLAB  EL.=SEE CIVIL DWG.	#5@12"O.C.E.W.

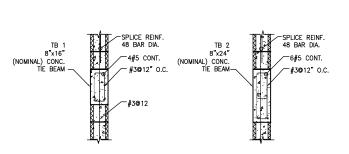
NOIE:	SEE	MEP	DWGS	FUR	SIZE	ANU	LOCATION	OF	>

TYPICAL DIESEL GENERATOR SLAB ON GRADE

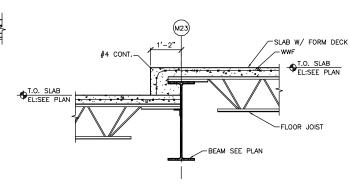
	SECTION 1	SECTION 2	SECTION 3	
TYPE	RECESS DEPTH	SHEAR TAB CONNECTION	EMBED PLATE	
Α	1 1/2"	PL 3/8"x3 1/2"x0'-5" W/	₽ 1/2"x7"x0'-7" W/	
		(2) 3/4" ø BOLTS	(4) 5/8" ø HSA CONFIGURATION 1	
В	3 1/4"	PL 3/8"x3 1/2"x0'-5" W/	Æ 1/2"x7"x0'-7" W/	
		(2) 3/4" ø BOLTS	(4) 5/8" ø HSA CONFIGURATION 1	
С	4 1/4"	PL 1/2"x3 1/2"x0'-9" W/	PL 3/4"x6"x1'-0" W/	
		(3) 3/4" Ø BOLTS	(6) 3/4" øx4" HSA CONFIGURATION 2	

NOTE: SEE 14A,14B/S5.15 FOR ADDITIONAL INFORMATION





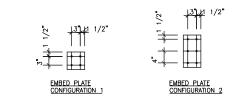
1 1 TIE BEAM DETAILS

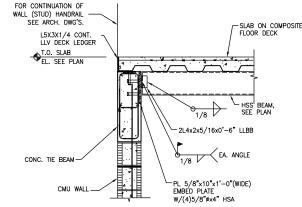


GIRDER FRAMING AT TERRACE

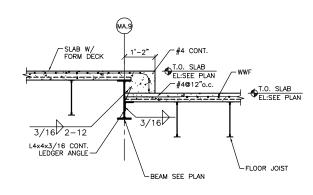
TABLE A									
JOIST	SPACING	PLATE LENGTH	#H.S.	MIN. BEARING	SUPPORT L	WELD A	WELD 6		
K SERIES	2'-0"	CONT.	2/FT.	2 1/2"	L4x4x3/4	1/4	3/16		
K SERIES	>2'-0"	9" LG.	4/PL	2 1/2"	L4x4x3/4	1/4	3/16		
LH SERIES	2'-0"	CONT.	2/FT.	4"	L6x6x1	5/16	1/4		

- STEEL BAR GRATING T.O. SLAB
EL: SEE PLAN - SHEAR TAB CONNECTION, SEE TABLE, SECTION 2 BEAM W/ BOTTOM FLANGE COPED, SEE PLAN EMBED PLATE -SEE TABLE SECTION 3





NOTE: SEE 14/S5.07 FOR ADD'L INFO. HSS BEAM @ EXTERIOR PLATFORM CMU WALL

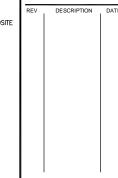


BEAM FRAMING AT TERRACE (8) 3/4"=1'-0"









**PSTA** 

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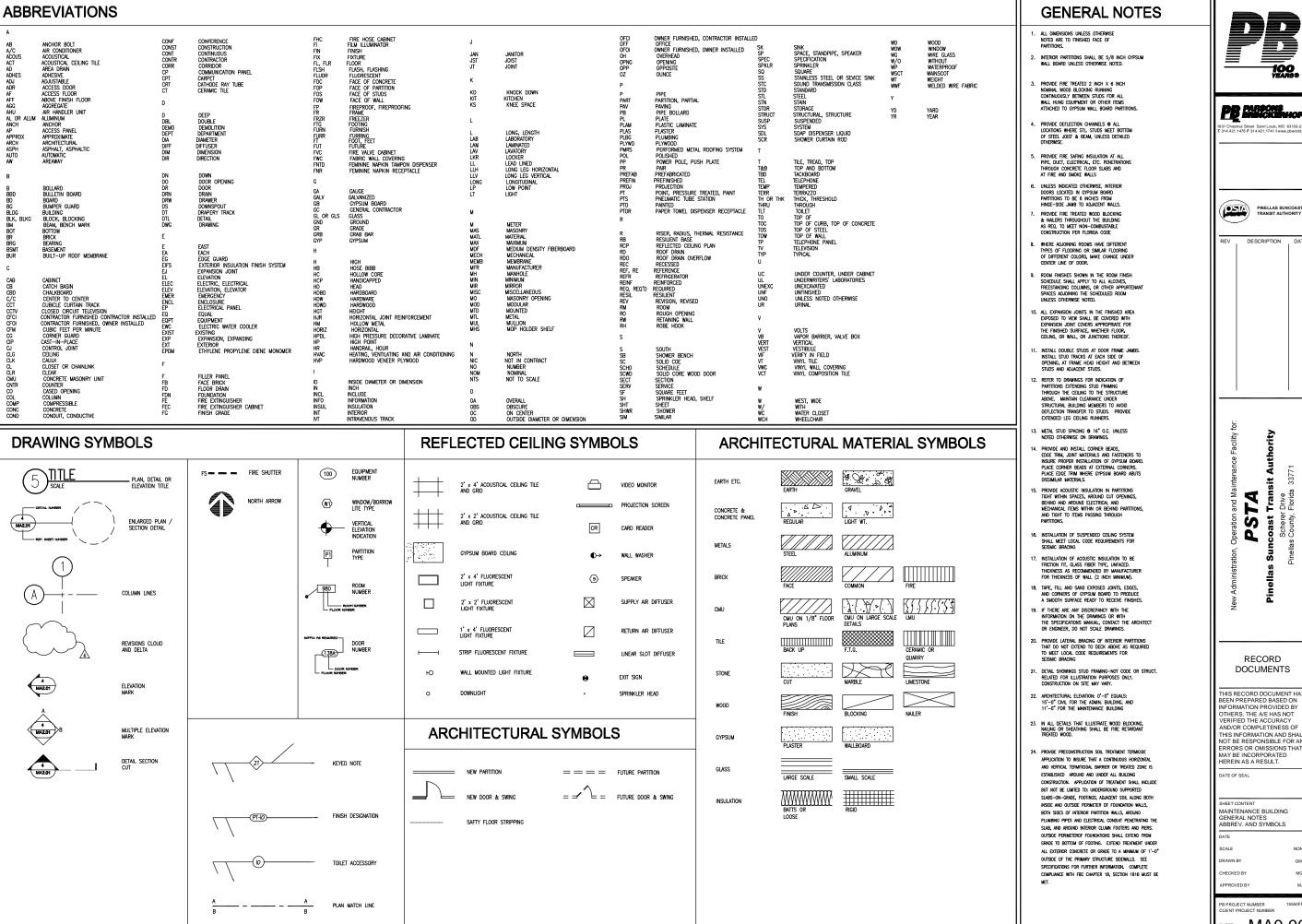
SHEET CONTENT STRUCTURAL DETAILS

DRAWN BY T.R. CHECKED BY

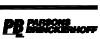
S5.15

P.A.

# Maintenance Architectural Drawing MA0.00- MA1.13

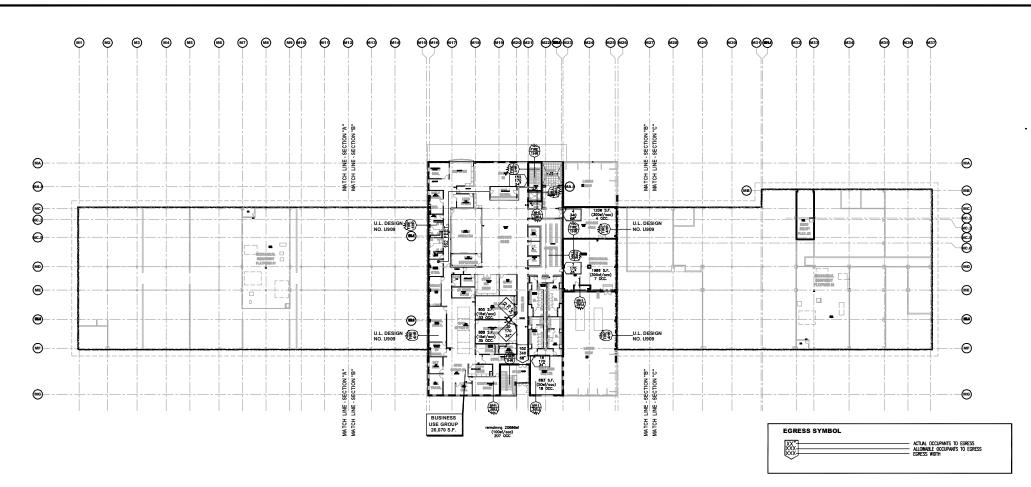






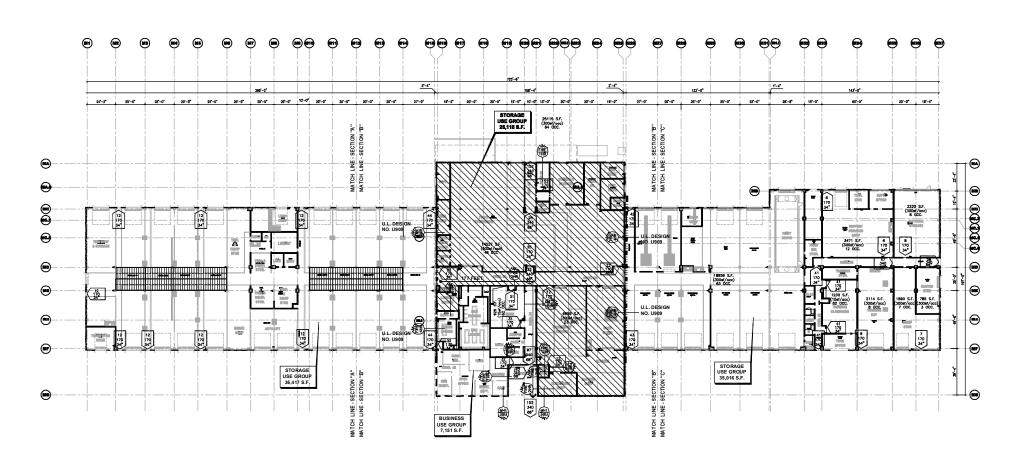
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MA0.00





SECOND FLOOR LIFE SAFETY PLAN - COMPOSITE



# **BUILDING CODE INFORMATION**

### FLORIDA CODE 2001

- 1. ALL EXTERIOR GLAZED OPENINGS AND SKYLIGHTS SHALL MEET THE MINIMUM REQUIREMENTS FOR WIND-BORNE DEBRIS IMPACT RESISTANCE STANDARDS FOR LARGE MISSLE TESTS MEETING THE REQUIREMENTS OF SSTD 12, ASTM E 1886 AND ASTM E 1996 OR MAMI-DADE PA 201, 202 AND 203. REFERENCE STRUCTURAL DRAWINGS FOR ADDITIONAL INFORMATION.
- 2. MODIFIED BITUMEN MEMBRANE ROOF SYSTEM SHALL MEET THE MINIMUM REQUIREMENTS FOR WIND RESISTANCE OF NON-BALLASTED LOW-SLOPE ROOFS AS TESTED IN ACCORDANCE WITH FM 4450, FM 4470, UL 580, OR UL 1897.
- 3. BUILDING CATEGORY II (ASCE 7-98 TABLE 1-1)

	•			
DESC	DESCRIPTION		ANALYSIS	
USE GROUP	PRIMARY	(S-2), REP	(S-2), REPAIR GARAGE	
	SECONDARY	(B), BUSINE	(B), BUSINESS	
CONSTRUCTION TYPE			IV (UNPROTECTED) SPRINKLER THROUGHOUT	
BUSINESS, (B)		ALLOWABLE	ACTUAL	
	BUILDING HEIGHT	55 FEET	35 FEET	TABLE 500
	MAXIMUM STORY	5 STORY	2 STORY	
	AREA MULTI-STORY	34,000 SF	SEE PLAN	
STORAGE, (S-	STORAGE, (S-2)		ACTUAL	
	BUILDING HEIGHT	55 FEET	31 FEET	TABLE 500
	MAXIMUM STORY	4 STORY	1 STORY	
	ONE STORY ONLY	48,000 SF	SEE PLAN	

# CONSTRUCTION FIRE RESISTANCE RATINGS (HOURS)

	· · · · · · · · · · · · · · · · · · ·	-
OCCUPANCY SEPARATION AUTOMOBILE REPAIR GARAGE - SPECIAL OCCUPANCY	2	TABLE 704.1
PARTY AND FIRE WALLS	4	TABLE 600
INTERIOR BEARING WALLS	NONCOMBUSTIBLE	
COLUMNS SUPPORTING OTHER COLUMNS OR MORE THAN ONE FLOOR	NONCOMBUSTIBLE	
COLUMNS SUPPORTING ONE FLOOR ONLY	NONCOMBUSTIBLE	
COLUMNS SUPPORTING ROOF ONLY	NONCOMBUSTIBLE	
BEAMS, GIRDERS, TRUSSES, ARCHES SUPPORTING COLUMNS OR MORE THAN ONE FLOOR	NONCOMBUSTIBLE	
SUPPORTING ONE FLOOR ONLY	NONCOMBUSTIBLE	
SUPPORTING ROOF ONLY	NONCOMBUSTIBLE	
FLOORS & FLOOR/CEILING ASSEMBLIES	NONCOMBUSTIBLE	
ROOFS & ROOF/CEILING ASSEMBLIES	NONCOMBUSTIBLE	
HORIZONTAL SEPARATION FROM PROPERTY LINE for		

## EXTERIOR BEARING WALLS, EXTERIOR NONBEARING WALLS, & GABLE ENDS OF ROOF

	O FEET TO 3 FEET	1 0% WALL OPENINGS	TABLE 600			
	OVER 3 FEET TO 10 FEET	1 10% WALL OPENINGS				
	OVER 10 FEET TO 20 FEET	NONCOMBUSTIBLE 20% WALL OPENINGS				
	OVER 20 FEET TO 30 FEET	NONCOMBUSTIBLE 40% WALL OPENINGS				
	OVER 30 FEET	NONCOMBUSTIBLE NO LIMIT WALL OPENINGS				

FIRE RESISTANCE RATING OF EXIT ACCESS CORRIDORS		0		TABLE	704.2.4
30 OR MORE OCCUPANTS					
BUSINESS, ST	ORAGE				
MIN. FIRE RESISTANCE OF PARTITIONS & OPENINGS		PARTITION	OPENING	TABLE	705.1.2
	SHAFT ENCLOSURES	1	1		
	(including stairs, elevators, exits)				
	LESS THAN 4 STORIES				
	OCCUPANCY SEPARATION	2	1-1/2		
	FIRE WALLS	4	3		
			- //		

BUSINESS -	(B)			TABLE 100	
	LENGTH OF EXIT ACCESS TRAVEL	300 FEET			
	MAX.IMUM DEAD END CORRIDOR	50 FEET 0.2 INCHES			
	EGRESS WIDTH PER PERSON CORRIDOR				
	EGRESS WIDTH PER PERSON STAIRS	0.3 INCHE	S		
	MINIMUM CORRIDOR WIDTH	44 INCHES	i	1	
	MINIMUM CLEAR OPENING OF EXIT DODRS	32 INCHES	i	1	
	MINIMUM STAIR WIDTH	44 NCHES		1	
STORAGE -	(S-2)			TABLE 100	
	LENGTH OF EXIT ACCESS TRAVEL	400 FEET		1	
	MAX.IMUM DEAD END CORRIDOR	100 FEET			
	EGRESS WIDTH PER PERSON CORRIDOR	0.2 INCHE	5		
	EGRESS WIDTH PER PERSON STAIRS	0.3 INCHE	S		
	MINIMUM CORRIDOR WIDTH	44 INCHES	;		
	MINIMUM CLEAR OPENING OF EXIT DOORS	32 INCHES			
	MINIMUM STAIR WIDTH	44 NCHES		1	
MINIMUM OCC	CUPANT LOAD			1	
	BUSINESS - (B)	100 GROSS SQ. FT.		TABLE 100	
	STORAGE - (S-2)	300 GROS	S SQ. FT.		
MEZZANINE S	INGLE EXIT CRITERIA			SECTION 1	
	OCCUPANT LOAD LESS THAN	30		1	
	MAXIMUM TRAVEL DISTANCE	100 FEET			

# **HOURLY PARTITION RATINGS**

GRAPHIC	DESCRIPTION		REFERENCE	
	1-HOUR	GYPSUM BOARD CMU		U419) U905)
	2-HOUR	GYPSUM BOARD CMU		U419) U905)
	4 - HOUR FIRE WALL	SEPARATION		
	2-HOUR FLOOR ASSEM			D780) 11/MA4.
FE F⊕- (FIRE EXTINGUISHER SURFACE MOUNT)  FEC F⊕ (FIRE EXTINGUISHER CABINET MOUNT - SEMI-RECESSED)	FIRE EXTINGUISHER (RECESS MOUNT IN MAINTENANCE ARE ENLARGED FLOOR PLANS FO	ED IN ALL OFFICE SPACE, SURFACE A AND UTILITY ROOMS — REFERENCE R LOCATIONS)		
• 177'	MOST REMOTE EGRESS	PATH		







DESCRIPTION

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RECORD

DOCUMENTS

BUSINESS -	- (B)			TABLE	1004
	LENGTH OF EXIT ACCESS TRAVEL	300 FEET			
	MAX.IMUM DEAD END CORRIDOR	50 FEET			
	EGRESS WIDTH PER PERSON CORRIDOR	0.2 INCHES	5		
	EGRESS WIDTH PER PERSON STAIRS	0.3 INCHES	3	1	
	MINIMUM CORRIDOR WIDTH	44 INCHES		1	
	MINIMUM CLEAR OPENING OF EXIT DODRS	32 INCHES		1	
	MINIMUM STAIR WIDTH	44 NCHES		1	
STORAGE -	(S-2)			TABLE	1004
	LENGTH OF EXIT ACCESS TRAVEL	400 FEET			
	MAX.IMUM DEAD END CORRIDOR	100 FEET		1	
	EGRESS WIDTH PER PERSON CORRIDOR	0.2 INCHES		1	
	EGRESS WIDTH PER PERSON STAIRS	0.3 INCHES		1	
	MINIMUM CORRIDOR WIDTH	44 INCHES		1	
	MINIMUM CLEAR OPENING OF EXIT DODRS	32 INCHES		i	
	MINIMUM STAIR WIDTH	44 NCHES		1	
MINIMUM OC	CUPANT LOAD			1	
	BUSINESS - (B)		100 GROSS SQ. FT.		1003.
	STORAGE - (S-2)	300 GROS	S SQ. FT.		
MEZZANINE	SINGLE EXIT CRITERIA			SECTIO	N 100
	OCCUPANT LOAD LESS THAN	30		1	
	MAXIMUM TRAVEL DISTANCE	100 FEET		1	

# THIS RECORD DOCUMENT HAS BEEN PREPARED BASED ON INFORMATION PROVIDED BY OTHERS. THE AVE HAS NOT VERIFIED THE ACCURACY

AND/OR COMPLETENESS OF AND/OR COMPLETENESS OF THIS INFORMATION AND SHALL NOT BE RESPONSIBLE FOR AN ERRORS OR OMISSIONS THAT MAY BE INCORPORATED HEREIN AS A RESULT. DATE OF SEAL SHEET CONTENT

1st FLR. LIFE SAFETY PLAN

2nd FLR. LIFE SAFETY PLAN
BUILDING CODE INFO.

B PROJECT NUMBER

SHEET MAO.10

FIRST FLOOR LIFE SAFETY PLAN - COMPOSITE
SOLLE 1022 = 1'-09"







PINELLAS SUNCOAST TRANSIT AUTHORITY

DESCRIPTION

sit Authority

**PSTA** 

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RECORD DOCUMENTS

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DATE OF SEAL



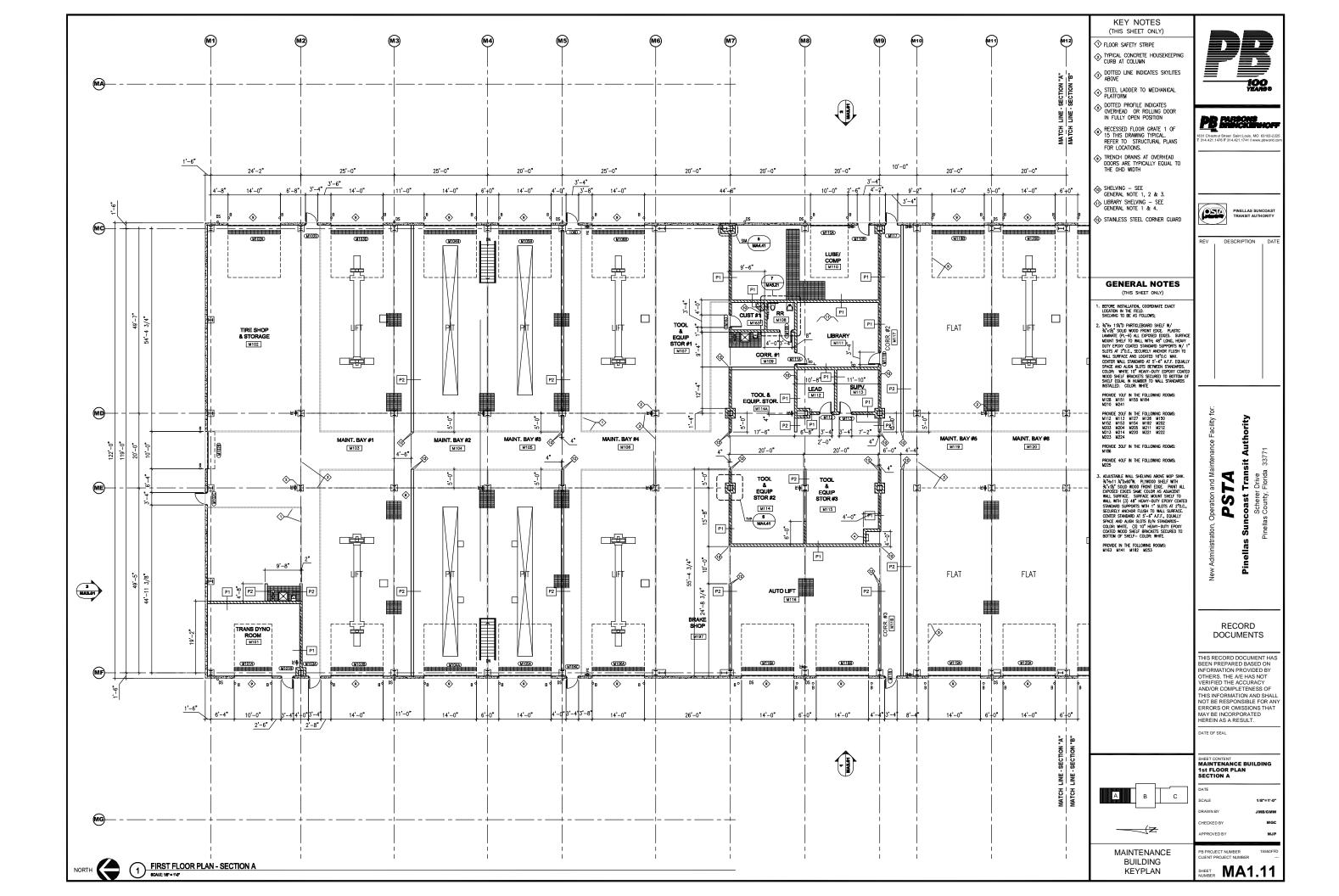
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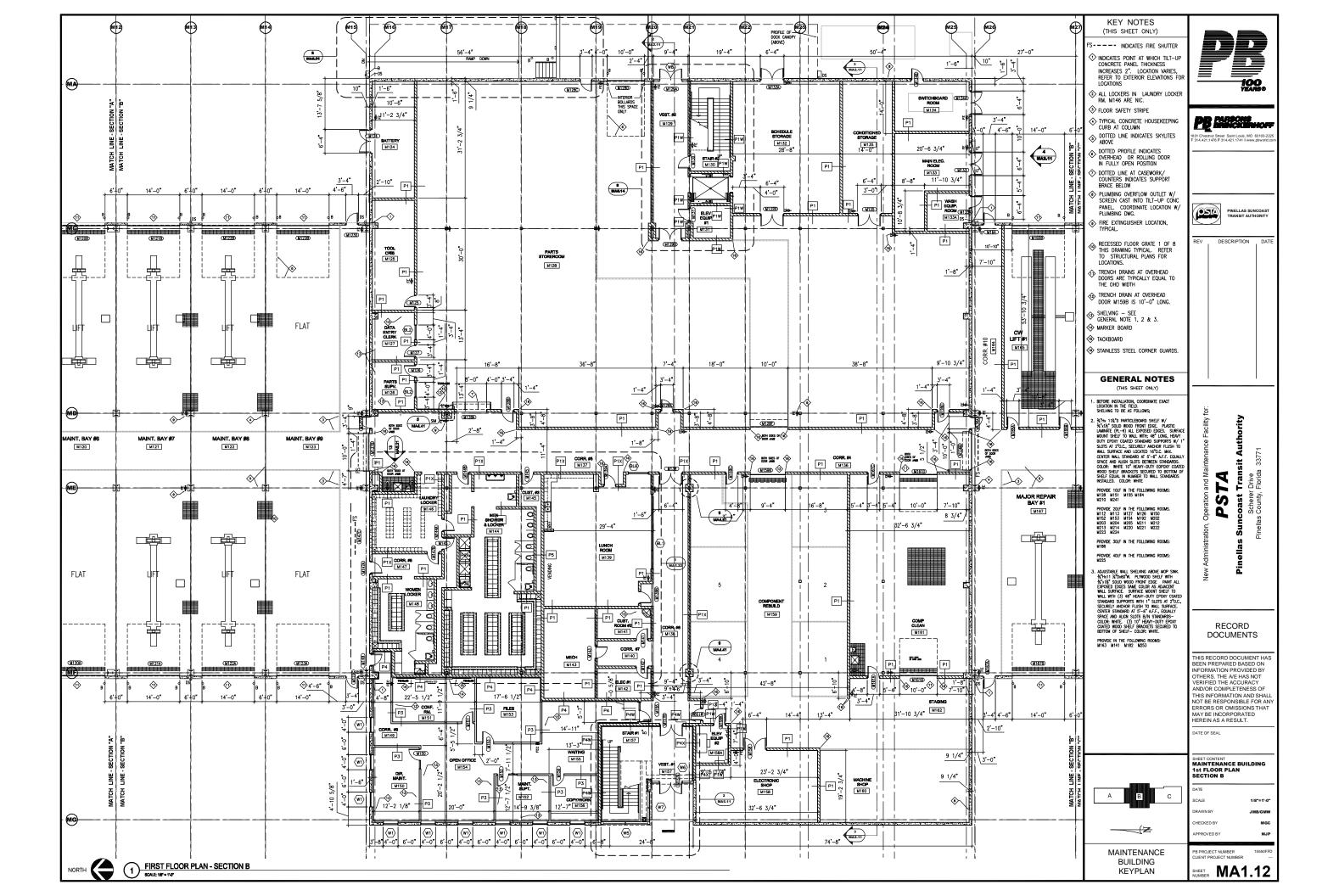
GMW

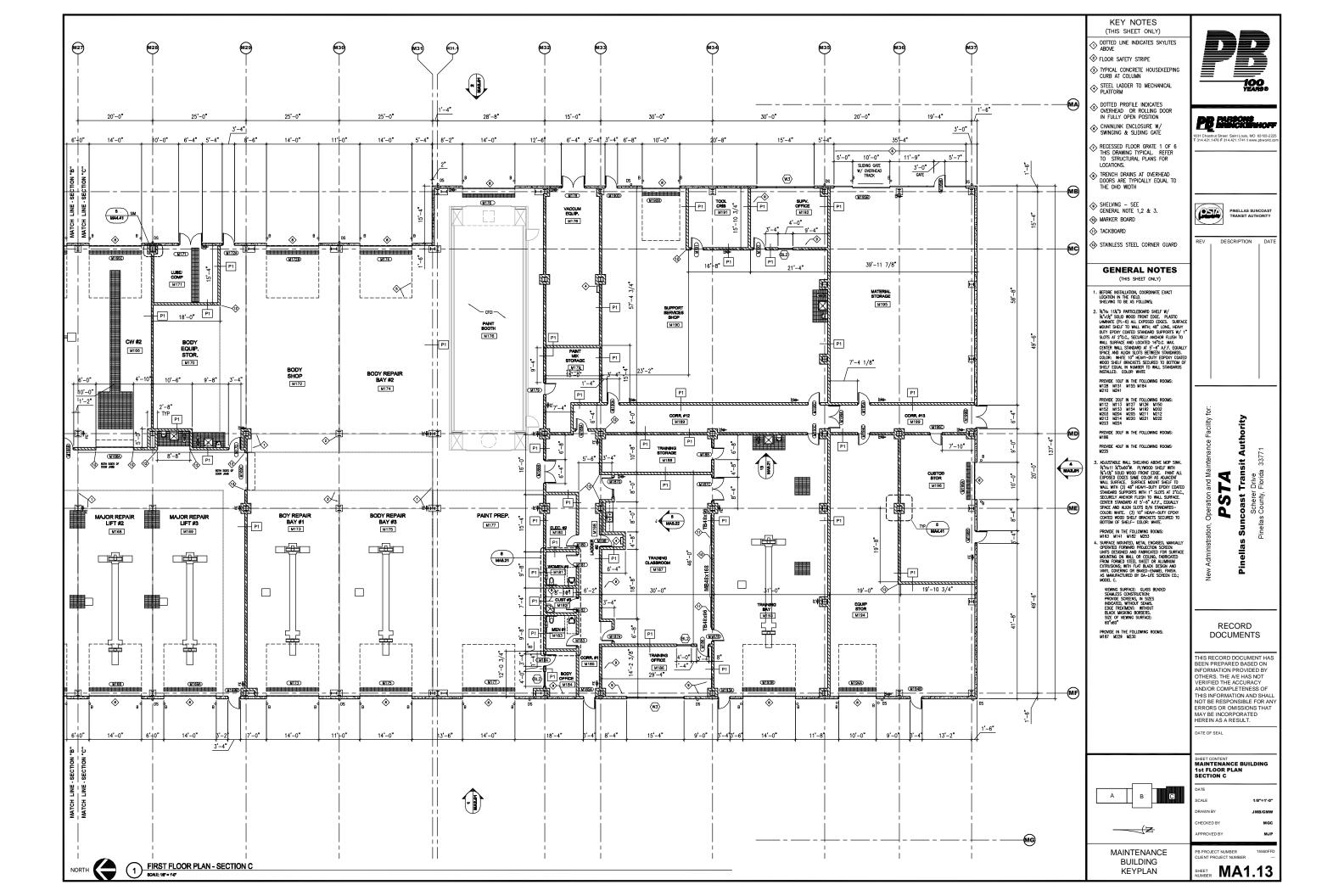
MAINTENANCE BUILDING

KEYPLAN

SHEET MA1.10







# Maintenance Architectural Drawing MA2.01C- MA3

