

## RFP # 17-058P DESIGN - BUILD FREESTANDING STEEL BRIDGE CRANE AND RELATED SERVICES ADDENDUM No. 1 May 23, 2017

## A. Clarifications

- 1. Section 1.3, Item G (Power Volts): Requirements for Power volts will now accept single phase. (Section 1.8 systems options is correct)
- 2. Please see the attached links to directories providing Certified DBEs.
  - a. <u>http://www3b.dot.state.fl.us/EqualOpportunityOfficeBusinessDirectory/CustomSearch.aspx</u>
  - b. <u>https://osd.dms.myflorida.com/directories</u>
  - c. <u>https://tampa.diversitysoftware.com/?TN=tampa</u>
- 3. Documents attached to this Addendum:
  - a. Dimension breakdown for Free Standing Bridge Crane (1 page)
  - b. Structural Drawings Maintenance (29 pages)
  - c. Maintenance Architectural Drawing MA0.00- MA1.13 (6 pages)
  - d. Maintenance Architectural Drawing MA2.01C- MA3.03 (7 pages)

## **B.** Questions and Answers

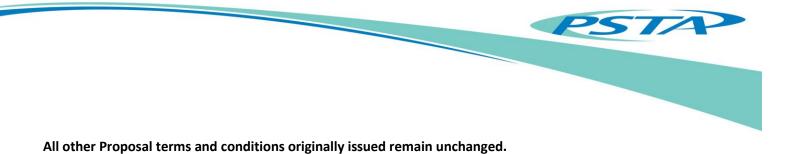
1. **Question:** Can 3 phase power of some sort be made available to the area? If so, what type? Ex. 230-3-60, 460-3-60, etc...

**Response:** Both 208 volt 3 phase and 480 volt 3 phase power can be provided. The panels serving this area have approximately 3 to 5 hp capacity. If a larger motor or feeder is required, a feeder from a panel further away can be provided.

2. **Question:** We would need to be released from the requirement on page twelve (12) item H. The Proposer must provide a 100% Performance Bond and a 50% Payment Bond.

**Response:** Please reference page 104 of the RFP document for Bonding requirements exceeding \$100,000.

Bonding for Construction Activities	5% bid guarantee bond. 100% performance bond. Payment bond equal to:	§ 15.o(1)
Exceeding \$100,000	– 50% for contracts < \$1M. – 40% for contracts >\$1M – < \$5M. – \$2.5M for contracts > \$5M.	5(-)

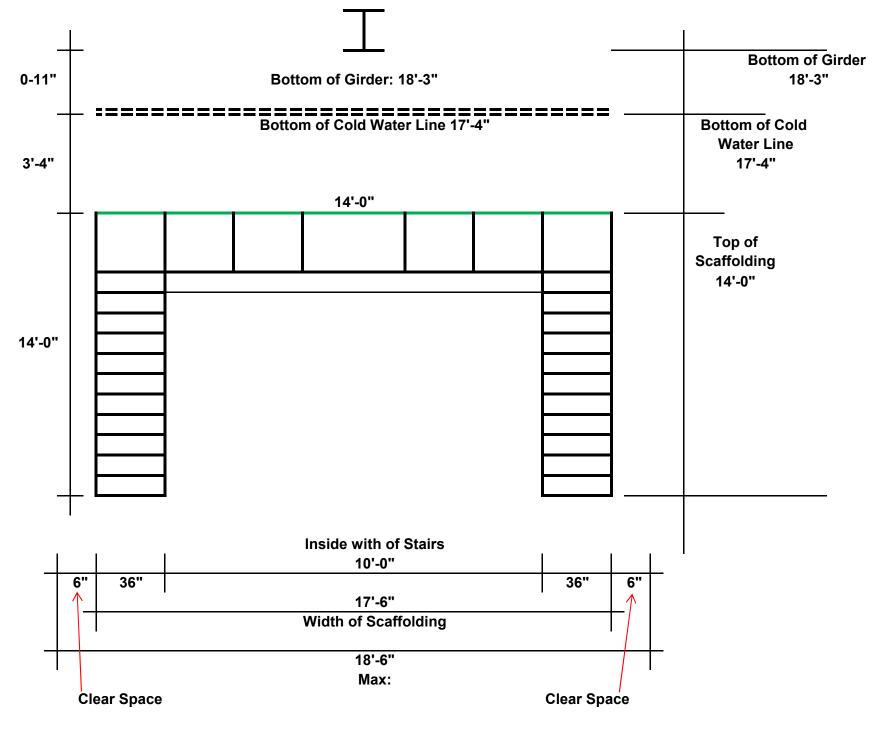


REMINDER: Make sure you mark "Addendum No. 1" on Attachment "1" Acknowledgement of Addendum and remember to sign and return Acknowledgement Addendum form with your submittal package. Failure to do so may result in the disqualification of your proposal submittal.

The RFP is revised to the extent specifically amended by this Addendum #1. Otherwise, all provisions of the RFP remain in effect.

Eric L. Haubner Purchasing Agent II Pinellas Suncoast Transit Authority EHaubner@psta.net

# Dimension breakdown for Free Standing Bridge Crane



Proposer's are responsible for verifying field dimensions.

## Structural Drawings Maintenance

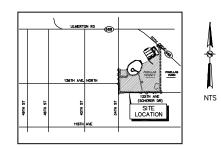


## **Pinellas Suncoast Transit Authority**

Administration Building: 3201 Scherer Drive Pinellas County, Florida 33771 Maintenance Building: 3101 Scherer Drive Pinellas County, Florida 33771



## LOCATION MAP:



SECTION 11 TOWNSHIP 30S RANGE 16E PINELLAS COUNTY, FLORIDA

## PROJECT TEAM:

Architecture Mechanical Electrical Plumbing Fire Protection Interior Design



11757 Katy Freeway, Suite 600 Houston, Texas 77079 T: 281.558.7273 F: 281.558.7282

1831 Chestnut Street Saint Louis, Missouri 63103 T: 314.421.1476 F: 314.421.5664

Structural BRINDLEY PIETERS & ASSOCIATES, INC.



401 CenterPointe Circle, Suite 1501 Altamonte Springs, Florida 32701 T: 407.830.8700 F: 407.830.8877

Civil



Volume 3



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NOTE: STRUCTURAL GENERAL NOTES & DETAILS ARE DUPLICATED IN VOLUMES 2, 3 AND 4. SOME DETAILS MAY NOT APPLY TO ALL VOLUMES.

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PB PROJECT NUMBER 15550FFD CLIENT PROJECT NUMBER

### ENERAL NOTES

THE GOVERNING CODE FOR THIS PROJECT IS THE FLORIDA BUILDING CODE, 2001. AND LATEST AMENDMENTS. THIS CODE PRESCRIBES WHICH EDITION OF EACH REFERENCED STANDARD APPLIES

- TO THIS PROJECT. TO THIS PROJECT. TO THE BEST OF OUR KNOWLEDGE, THE STRUCTURAL DRAWINGS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE REQUIREMENTS OF THE GOVERNING BUILDING CODE.
- CONSTRUCTION IS TO COMPLY WITH THE REQUIREMENTS OF THE GOVERNING BUILDING CODE AND ALL OTHER APPLICABLE FEDERAL, STATE, AND LOCAL CODES, STANDARDS, REGULATIONS AND LAWS.
- THE STRUCTURAL DOCUMENTS ARE TO BE USED IN CONJUNCTION WITH THE ARCHITECTURAL DOCUMENTS. USE THESE NOTES IN CONJUNCTION WITH THE PROJECT SPECIFICATIONS. IF A CONFLICT EXISTS, THE MORE STRINGENT GOVERNS.
- DETAILS LABELED "TYPICAL" APPLY TO ALL SITUATIONS THAT ARE THE SAME OR SIMILAR TO THOSE SPECIFICALLY REFERENCED, WHETHER OR NOT THEY ARE KEYED IN AT EACH LOCATION, QUESTIONS REGARDING THE APPLICABILITY OF TYPICAL DETAILS
- SHALL BE RESOLVED BY THE ARCHITECT. OPENINGS SHOWN ON STRUCTURAL DRAWINGS ARE ONLY PICTORIAL. SEE THE ARCHITECTURAL AND MECHANICAL DRAWINGS FOR THE SIZE AND LOCATION OF
- OPENINGS IN THE STRUCTURE. CONTRACTORS WHO DISCOVER DISCREPANCIES, OMISSIONS OR VARIATIONS IN THE CONTRACTORS WHO DISCOVER DISCREPANCIES, OMISSIONS OR VARIATIONS IN THE CONTRACT DOCUMENTS DURING BIDDING SHALL IMMEDIATELY NOTIFY THE ARCHITECT. THE ARCHITECT WILL RESOLVE THE CONDITION AND ISSUE A WRITTEN CLARIFICATION. THE CONTRACTOR SHALL COORDINATE ALL CONTRACT DOCUMENTS WITH THE CONTRACTOR SHALL COORDINATE ALL CONTRACT DOCUMENTS, WITH FIELD CONDITIONS AND DIMENSIONS AND PROJECT SHOP DRAWINGS PRIOR TO CONSTRUCTION. DO NOT SCALE DRAWINGS; USE ONLY PRINTED DIMENSIONS. ELECTRONIC DRAWINGS SHOULD NOT BE ASSUMED TO BE DRAWN TO SCALE. REPORT ANY DISCREPANCIES IN WRITING TO THE ARCHITECT PRIOR TO PROCEEDING WITH WORK. DO NOT CHANGE SIZE OR LOCATION OF STRUCTURAL MEMBERS WITHOUT WRITTEN INSTRUCTIONS FROM THE STRUCTURAL ENGINEER OF RECORD. THE STRUCTURE IS DESIGNED TO BE STRUCTURALLY SOUND WHEN COMPLETED. PRIOR TO COMPLETION, THE CONTRACTOR IS RESPONSIBLE FOR STABILITY AND TEMPORARY BRACING, INCLUDING, BUT NOT LIMITED TO, MASONRY WALLS, TILT CONCRETE WALLS. WHEREVER THE CONTRACTOR IS UNSURE OF THESE CONCRETE WALLS, WHEREVER THE CONTRACTOR IS UNSURE OF THESE
- REQUIREMENTS, THE CONSTRUCTION MANAGER SHALL RETAIN A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF FLORIDA TO DESIGN AND INSPECT THE TEMPORARY BRACING AND STABILITY OF THE STRUCTURE. DESIGN SUPERIMPOSED LOADS: LIVE LOAD DEAD LOAD
- (C) 5 PSF
- DESIGN SUPERIMPOSED DEAD LOADS LISTED ABOVE DO NOT INCLUDE MASONRY OR STUD WALLS OR OTHER CONCENTRATED LOADS. SEE ARCHITECTURAL DRAWINGS & MEP DWGS. FOR THESE LOADS.
- OFFICES INCLUDE 20 PSF ALLOWANCE FOR PARTITIONS
- DESIGN WIND LOADS GOVERNING CODE .....FLORIDA BUILDING CODE 2001....... BASIC WIND SPEED (3-SECOND GUST)..... .../ASCE 7-98 .....V = 123 MPH BUILDING CATEGORT BUILDING CATEGORY (ASCE 7-98 TABLE 1-1) ..... ..... = 1.0 ......B ......33 FEET ......Kd =0.85 ......Kzt=1.0 ......GCp: = 0.18 ....GCp: = 0.55

#### ADJACENT BUILDINGS AND PROPERTY

- THE CONTRACTOR SHALL PROTECT ADJACENT PROPERTY, HIS OWN WORK AND THE PUBLIC FROM HARM. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR CONSTRUCTION MEANS AND METHODS, AND JOBSITE SAFETY INCLUDING ALL OSHA REQUIREMENTS. THE CONTRACTOR SHALL ENSURE THAT ALL CONSTRUCTION METHODS USED WILL NOT CAUSE DAMAGE TO THE ADJACENT BUILDINGS AND PROPERTY. THIS SHALL INCLUDE ALL FOUNDATION INSTALLATION, THE CONTRACTOR IS ADVISED TO PERFORM ALL PHOTOGRAPHIC SURVEYS AND
- OTHER DOCUMENTATION OF THE ADJACENT BUILDINGS BEFORE THE START OF

#### REINFORCED CONCRETE (REF SECTION 03300)

	INCLUE CONCILIE (INCLUE) (INCLUE)				
1.	COMPLY WITH ACI 301 AND 318 AND SPECIFICATION SECT	TIONS			
2.	PROVIDE STRUCTURAL CONCRETE WITH A MINIMUM ULTIMA	TE COMPRESSIVE DESIGN			
	STRENGTH fc', IN 28 DAYS AS FOLLOWS:				
	ELEMENT	STRENGTH			
	FOOTINGS, PIT SLAB	3000 PSI			
	SLABS ON GRADE AT MAINTEN/OPERATIONS	4000 PSI			
	TYPICAL SLAB ON DECK	3000 PSI			
	ELEVATED SLAB	4000 PSI			
	TILT UP WALLS	5000 PSI			
	ELEVATED SLAB, OVER MAINTEN. PIT	4000 PSI			
		1000 001			

- C.I.P. COLUMNS, BEAMS, WALLS 4000 PSI USE NORMAL WEIGHT CONCRETE 150PCF FOR ALL STRUCTURAL
- MEMBERS WITH CARBONATE OR SILICEOUS ACGREGATE, U.N.O. PROVIDE ASTM A-615 GRADE 60 REINFORCING STELL. REINFORCING SHALL BE ACCURATELY PLACED, RIGIDLY SUPPORTED AND FIRMLY TIED IN PLACE, WITH APPROPRIATE BAR SUPPORTS AND SPACERS. LAP CONT. REINE, AS NOTED IN LAP SPLICE SCHEDULE, LAP BOTTOM STEEL OVER SUPPORTS AND TOP STEEL AT MIDSPAN (U.O.N.). HOOK DISCONTINUOUS ENDS OF ALL TOP BARS AND ALL BARS IN WALLS, U.O.N. PROVIDE COVER OVER REINFORCING AS FOLLOWS: TOP CIDEC

	DOTTON	101	3023	
FOOTINGS, FOUND WALLS	3"	2"	3"	
CONCRETE PIERS	-	-	3"	
SLABS ON GRADE	2"	1"	2"	
SLABS ABOVE GRADE	3/4"	3/4"	1"	
SLABS EXPOSED TO WEATHER	1 1/2"	1 1/2"	1 1/2"	
WALLS RETAINING FILL	-	-	2"	
WALLS; TILT WALLS ABOVE GRADE	-	-	1"	
DEALAS	1 1 /0"	1 1 /0"	1 1 /0"	

- BEAMS COLUMNS WHERE SPECIFIED, PROVIDE PLAIN, COLD-DRAWN ELECTRICALLY-WELDED STEEL WIRE
- FABRIC CONFORMING TO ASTM A-185. SUPPLY IN FLAT SHEETS ONLY, LAP SPLICE ONE CROSS WIRE SPACING PLUS TWO INCHES PROVIDE THE FOLLOWING TEMPERATURE REINF. FOR ONE-WAY SLABS, U.O.N.:
- SLAB THICKNESS REINFORCING 3 1/2" - 4 1/4"#3 @ 12" 0/0

	4 1/2" - 6 1/2"	#4 @ 18" O/C
	7"- 7 1/2"	#4 @ 15" 0/C
	8" - 9"	#4 @ 12" O/C
IN	ADDITION TO SPECIFIED REINFORCING	PROVIDE 5 TONS OF F

REINFORCING BARS TO BE DETAILED, FABRICATED, DELIVERED TO SITE AND PLACED AS DIRECTED BY THE ARCHITECT/ENGINEER TO ACCOUNT FOR UNFORESEEABLE CONDITIONS

#### REINFORCED CONCRETE (CONT)

- UTILITIES SHALL NOT PENETRATE BEAMS OR COLUMNS BUT MAY PASS THROUGH SLABS AND WALLS INDIVIDUALLY, U.O.N. FOR OPENINGS 24" LONG OR LESS, CUT REINE AND REPLACE ALONGSIDE OPENING WITH SPLICE BARS OF FOUNALENT AREA WITH 48 BAR. DIA. LAP. PREPARE AND SUBMIT SHOP DRAWINGS FOR OPENINGS LONGER THAN 24". FOR RECTANGULAR OPENINGS 12" LONG OR LONGER, ADD 1#5 x 5' MID DEPTH DIAGONAL AT ALL 4 CORNERS.
- WHERE REINFORCING STEEL CONGESTION PERMITS, CONDUIT AND PIPES UP TO 1 DIAMETER MAY BE EMBEDDED IN CONCRETE PER ACI 318, SECTION 6.3. SPACE AT 3 DIAMETERS O.C. PLACE BETWEEN OUTER LAYERS OF REINF. IF CONDUITS ARE SIGNIFICANTLY CONGESTED, ADDITIONAL REINFORCING PERPENDICULAR TO PIPING MAY BE REQUIRED, REQUESTS TO EMBED LARGER PIPES SHOULD BE ACCOMPANIED BY A DETAILED DESCRIPTION AND BE SUBMITTED TO THE ARCHITECT FOR EVALUATION. PROVIDE CONSTRUCTION JOINTS IN ACCORDANCE WITH ACI 318, SECTION 6.4.
- PROVIDE KEYWAYS AND ADEQUATE DOWELS. SUBMIT DRAWINGS SHOWING LOCATION OF CONSTRUCTION JOINTS AND DIRECTION OF POUR FOR REVIEW. PROVIDE 3/4" CHAMFER FOR ALL EXPOSED CORNERS.
- IN WALLS, GRADE AND TIE BEAMS PROVIDE CORNER BARS TO MATCH HORIZONTAL 12
- WALL BARS AT ALL CORRERS. PROVIDE REINFORCING STEEL PLACER WITH A SET OF STRUCTURAL DRAWINGS FOR FIELD REFERENCE. INSPECT REINFORCING STEEL PLACING FROM STRUCTURAL 13.
- DRAWINGS COORDINATE EXACT SIZES AND LOCATIONS OF HOUSEKEEPING PADS, SUMP PITS
- TRENCH DRAINS, SLEEVES AND EMBEDS WITH OTHER TRADES AND WITH ARCH/MEP DWGS.

#### EXPANSION ANCHORS

- USE WEDGE-TYPE EXPANSION ANCHORS SUCH AS THE HILTI KWIK BOLT II, ITW RAMSET RED HEAD TRUBOLT WEDGE, POWERS RAWL POWER-STUD, OR ACCEPTED EQUIVALENT. FOLLOW MANUFACTURER'S SPECIFICATIONS FOR USE AND INSTALLATION. 2
- CONFIRM THE ABSENCE OF REINFORCING STEEL BY DRILLING A 1/4" DIAMETER PILOT HOLE FOR EACH ANCHOR. DO NOT CUT REINFORCING STEEL WITHOUT APPROVAL OF THE STRUCTURAL ENGINEER.
- PROVIDE ANCHOR EMBEDMENT, SPACING AND EDGE DISTANCE AS SHOWN ON THE 3.

#### SHALLOW FOUNDATIONS

- GEOTECHNICAL INVESTIGATION REPORT, PROPOSED BUSINESS ADMINISTRATION, OPERATIONS, & MAINTENANCE FACILITY, ST. PETERSBURG, FLORIDA, PROJECT NO. T-02-2003 BY MC SQUARED, INC., DATED JULY 2002 AND ANY SUBSEQUENT AMENDMENTS. THE CONSTRUCTION MANAGER SHOULD OBTAIN A COPY OF THE REPORT AND IT'S AMENDMENTS AND FOLLOW ALL RECOMMENDATIONS WITHIN
- PARTICULARLY DURING FOUNDATION & SLAB ON GRADE CONSTRUCTION. FOOTING SIZES AND REINFORCING ARE BASED ON AN ASSUMED ALLOWABLE SOIL BEARING CAPACITY OF MIN. 2500 PSF. AT A BEARING ELEVATION OF 5' BELOW EXTERIOR GRADE ON COMPACTED SOIL AS SPECIFIED IN THE GEOTECH REPORT. CENTER ALL FOOTINGS UNDER THEIR RESPECTIVE COLUMNS OR WALLS, U.O.N. TOP OF ALL FOOTINGS IS A MINIMUM OF 18" BELOW THE GRD. FLOOR LEVEL, U.O.N.
- SIDES OF THE FOOTINGS SHALL BE FORMED. EXCAVATED SIDES TO FORM THE FOOTINGS IS NOT ACCEPTABLE.

#### EXCAVATION, BACKFILL AND DEWATERING

- THE CONTRACTOR IS SOLELY RESPONSIBLE FOR ALL EXCAVATION PROCEDURES INCLUDING LAGGING, SHORING, AND PROTECTION OF ADJACENT PROPERTY, STRUCTURES, STREETS AND UTILITIES IN ACCORDANCE WITH THE REQUIREMENTS OF THE LOCAL BUILDING DEPARTMENT AND OSHA REGULATIONS. DO NOT EXCAVATE WITHIN ONE FOOT OF THE ANGLE OF REPOSE OF ANY SOLL BEARING FOUNDATION UNLESS THE FOUNDATION IS PROPERLY PROTECTED AGAINST SETTLEMENT. 2. DO NOT BACKFILL AGAINST WALLS UNTIL 7 DAYS AFTER THE WALLS ARE BRACED BY
- DO NOT BOTALL AGMINIST WELES AND SOUTH OF SATISFY AND AND AND ADDRESS ADDRESS ADDRESS AND ADDRESS AND ADDRESS AND ADDRESS AND
- IN A MANNER THAT DOES NOT INCONVENIENCE OR DAMAGE THE WORK. GROUND WATER: THE CONTRACTOR'S ATTENTION IS DRAWN TO GROUND WATER
- ELEVATIONS DISCUSSED IN THE GEOTECHNICAL REPORT. TEMPORARY WELL POINTS WILL BE NEEDED SO THAT THE GROUND WATER LEVEL IS MAINTAINED AT LEAST TWO (2) FEET BELOW BOTTOM OF FOOTINGS DURING COMPACTION AND CONSTRUCTION AT DEEP EXCAVATIONS.

#### SLABS ON GRADE

- REFER TO GEOTECHNICAL REPORT FOR SUBGRADE PREPARATION MORE THAN 12" BELOW BOTTOM OF SLAB. 2. ABOVE SUBGRADE, USE FILL CONTAINING NOT MORE THAN 10% PASSING #200 SIEVE
- AND MAXIMUM 1 INCH DIAMETER. COMPACT TO 95% OF MAXIMUM DRY DENSITY AS DETERMINED BY MODIFIED PROCTOR ASTM D-1557. EACH LAYER OF FILL SHALL NOT EXCEED 6" LOOSE THICKNESS. COMPACT PRIOR TO PLACEMENT OF THE NEXT LAYER. FILL PLACEMENT AND COMPACTION SHALL BE MONITORED AND ACCEPTED BY THE
- 3. TESTING AGENCY. TAKE A MIN. OF ONE FIELD DENSITY TEST (ASTM D-1556 OR D-2922) FOR EACH 2,500 SQUARE FEET OF EACH LAYER. THE TESTING AGENCY SHALL RANDOMLY SELECT TEST LOCATIONS.
- ANDUMEL SELECT TEST DUCTIONS.
   FOR INTERIOR SLABS PLACE 8 MIL POLYETHYLENE SHEETING BETWEEN SOIL AND BOTTOM OF SLAB. DO NOT USE ANY SHEETING BELOW EXTERIOR CONCRETE SLABS.
   SLAB THICKNESS AND REINFORCING ARE; U.N.O. ON PLANS, AS FOLLOWS:
- AREA THICKNESS REINFORCING 6X6 W2.9XW2.9 TOP AND BOT. PARKING AREAS 6" 6" ELECTRICAL AND MECHANICAL ROOMS EXEMD OXMD O TOP AND DO

ELECTRICAL AND MECHANICAL ROOMS	0	ONDWZ.SAWZ.S IOF AND BUT.
STORAGE ROOMS, SERVICE CORRIDORS	6"	6X6 W2.9XW2.9 TOP AND BOT.
ELEVATOR LOBBY	5"	6X6 W2.9XW2.9 TOP
TYPICAL & OFFICE	4"	6X6 W2.0XW2.0 TOP
OPERATIONS BLDG "B"	6"	6X6 W2.9XW2.9 TOP AND BOT.
MAINTENANCE BLDG "A&C"	8"	
FUEL\WASH BAYS	8"	#4@12" TOP AND BOT.
PLACE CONCRETE IN LONG-STRIP CONSTRU	CTION I	METHOD. PROVIDE CRACK CONTROL
JOINTS AT 21'-4" FEET MAX. TO LIMIT ARE	AS BET	WEEN JOINTS TO 400 SQ. FT. IN
ALL FLOATING SLABS ON GRADE. LOCATE 1	O CON	FORM TO BAY SPACING WHENEVER

POSSIBLE, ADD CRACK CONTROL JOINTS AT RE-ENTRANT CORNERS WHICH TEND TO INVITE CRACKS.

- IN SIDEWALKS AND WALKWAYS, LOCATE ISOLATION JOINTS AT 20 FT. O.C. MAX. SCORE AND TOOL BETWEEN ISOLATION JOINTS IN EQUAL BAYS OF 5 FT. OR LESS.
- SEE THE ARCHITECTURAL/MEP DRAWINGS FOR SLAB ON GRADE, SIZE & LOCATION OF DEPRESSIONS, SLOPE, TRENCH DRAINS, SUMPS, HOUSEKEEPING PADS AND OTHER REQUIREMENTS.

#### CONCRETE MASONRY

CONSTRUCT MASONRY IN ACCORDANCE WITH SPECIFICATION SECTIONS/04810 AND 04230; ACI 530/ASCE 5, "BUILDING CODE REQUIREMENTS FOR CONCRETE MASONRY STRUCTURES"; AND ACI 530.1/ASCE 6, "SPECIFICATIONS FOR THE DESIGN AND CONSTRUCTION OF LOAD-BEARING CONCRETE MASONRY".

WITHOUT REVIEW.

GOVERN

MANAGER/FABRICATOR

SPAN CONDITIONS

ASTM A525 G90 CLASS

CONDITIONS

.3

- THE STRUCTURE CONSISTS OF A SKELETON FRAME. FRECT MASONRY AFTER STRUCTURAL FRAMING SUPPORTING THE LEVEL ABOVE IS IN PLACE. SECURE MASONRY TO COLUMNS WITH GALVANIZED DOVETAIL ANCHORS (OR APPROVED EQUAL) IN EVERY SECOND COUNTS WITH ORCHAILED DOVELAIL ANOTOR'S (OR AFFROVED GOAL) IN EVERY SECOND COURSE. DO NOT POUR TIE COLUMNS UNTIL ALL SHORING AND RE-SHORING IN THAT STORY HAS BEEN REMOVED. USE 50% SOLID, NOMINAL 8X8X16, CONCRETE MASONRY UNITS CONFORMING TO ASTM
- C90. LAY UP UNITS IN RUNNING BOND. SAWCUT UNITS WHICH ARE NOT IN MULTIPLES OF 8". UNITS SHALL BE AT LEAST 8" LONG. BOND CORNERS BY LAPPING ENDS 8" IN SUCCESSIVE VERTICAL COURSES. DESIGN OF WALLS IS BASED ON A f m' OF 1500 PSI. USE TYPE S MORTAR IN ACCORDANCE WITH ASTM C270 EXCEPT USE TYPE M
- USE THE S MORTAR IN ACCURDANCE WITH ASIM C2/U EACEPT USE THE MORTAR BELOW GRADE. HEAD AND BED JOINTS SHALL BE 3/8" FOR THE THICKNESS OF THE FACE SHELL. WEBS ARE TO BE FULLY MORTARED IN ALL COURSES OF PIERS, COLUMNS AND PILASTERS; IN THE STARTING COURSE; AND WHERE AN ADJACENT CELL IS TO BE GROUTED. REMOVE MORTAR PROTRUSIONS EXTENDING 1/2" OR MORE INTO CELLS TO BE GROUTED.
- USE STANDARD (9 GAUGE) HORIZONTAL JOINT REINFORCING CONFORMING TO ASTM A-82 IN EVERY OTHER COURSE. OVERLAP DISCONTINUOUS ENDS 6". USE
- PREFABILICATED CONFIGN AND TEES. USE TRUSS TYPE, EXCEPT USE LADDER TYPE FOR ALL SINGLE WYTHE IN WALLS WITH VERTICAL REINFORCING. USE FINE GROUT CONFORMING TO ASTM C-476, WITH A MINIMUM COMPRESSIVE STRENGTH OF 2500 PSI IN 28 DAYS. AGGREGATE TO CONFORM TO ASTM C404 FOR FINE GROUT, WITH SLUMP OF 8" TO 10". GROUT ALL MASONRY CONTAINING REINFORCING, ALL CELLS OF 4 HOUR RATED WALLS, AND WHERE INDICATED ON THE REINFORCING, ALL CELLS OF 4 HOOR CALED WALLS, AND WHERE INDICATED ON DRAWINGS. ALLOW MORTAR TO CURE 24 HOURS PRIOR TO GROUTING, PROVIDE CLEANOUT OPENINGS AT THE BASE OF CELLS CONTAINING REINFORCING STEEL TO CLEAN THE CELL AND TO TIE THE VERTICAL BAR TO THE DOWEL. IN HIGH-LIFT
- GROUTING, USE 5'-0" (MAX.) LIFTS, WITH 1/2 HOUR TO 1 HOUR BETWEEN LIFTS. VIBRATE EACH LIFT AND RECONSOLIDATE THE PREVIOUS LIFT. USE ASTM A-615 GRADE 60 REINFORCING STEEL. REINFORCE WALLS WHERE INDICATED ON THE DRAWINGS AND 1#5 VERTICAL AT ALL INTERSECTIONS, EACH SIDE OF OPENINGS AND AT THE ENDS OF WALLS. USE BAR SPACERS @ 10 FT. O.C. WHERE GROUT POUR HEIGHT EXCEEDS 10'. AT BOND BEAM CORNERS AND INTERSECTIONS, PLACE 1 #5 x 5' T & B CORNER
- BAR, WITH 30" LEGS EACH WAY, AT THE EXTERIOR FACE. REINFORCED MASONRY WALL CONSTRUCTION SHALL BE INSPECTED BY AN ENGINEER
- OR ARCHITECT IN ACCORDANCE WITH ACI 530.1/ASCE6. WHERE ANCHOR BOLTS, WEDGE ANCHORS OR ANCHORS SET IN EPOXY ARE SET IN A MASONRY WALL, FILL CELLS WITH GROUT FOR BOLTED COURSE, ONE COURSE 10 ABOVE AND TWO COURSES BELOW.
- PROVIDE LINTELS OR HEADERS WITH MIN. 8" BEARING OVER ALL MASONRY OPENINGS. USE PRESSURE-TREATED WOOD FOR WOOD IN CONTACT WITH MASONRY. 13
- CONTROL JOINTS SHALL BE PROVIDE IN ALL CONCRETE MASONRY CONSTRUCTION AT LOCATIONS INDICATED ON THE ARCHITECTURAL DRAWINGS AND SPECIFICATIONS. THE SPACING OF CONTROL JOINTS SHALL NOT EXCEED 20'-0".
- HORIZONTAL WALL REINFORCING SHALL BE STOPPED EACH SIDE OF CONTROL JOINTS. SEE ARCHITECTURAL DRAWINGS FOR SEALANT REQUIREMENTS AT CONTROL JOINTS. 16. REINFORCE MASONRY WALLS WITH #5@48" MAX
- VERTICAL (U.N.O.). PROVIDE CONTINUOUS HORIZONTAL BOND BEAM COURSE W/2 #5 HORIZONTAL EVERY 10'-O" OF HEIGHT AND AT TOP OF WALL. ALL LAPS 48 BAR DIAMETERS. PROVIDE DOWELS TO MATCH WALL REINFORCEMENT.

- STEEL ROOF DECK (REF. SECTION 05300) 1. ROOF DECK SHALL BE AS FOLLOWS: A. ALL METAL ROOF DECK EXCEPT WHERE SHOWN ON PLAN SHALL BE 1 1/2" DEEP

- ALL METAL ROOF DECK EXCEPT WHERE SHOWN ON PLAN SHALL BE 1 1/2" DEEP 22 GAGE WIDE RIB, MIN. PROPERTIES: Sp- 0.186IN Sn=.192 IN , I=0.169IN, GALVANIZED G90.
   ROOF DECK SHALL BE PLACED IN AT LEAST TWO SPAN SEGMENTS. NO SINGLE SPAN CONDITIONS SHALL BE USED.
   C. STEEL DECK SHALL CONFORM TO ASTM A446 GRADES A, B, C, D, E OR F FOR GALVANIZED DECK, MINIMUM YIELD STRENGTH OF 33,000 PSI.
   D. STEEL DECK SHALL BE GALVANIZED WITH A PROTECTIVE ZINC COATING CONFORMING TO ASTM A525 G90 CLASS. E. ATTACHMENT:

1. WELDING:

- a) ROOF DECK UNITS SHALL BE WELDED TO EACH STRUCTURAL SUPPORT ROUF DELK UNITS SHALL BE WELDED TO EACH STRUCTORAL SUPPORT MEMBER USING 5/8" DIAMETER PUDDLE WELDS AT ALL RIBS (36/7 FASTENER LAYOUT). WELD METAL SHALL PENETRATE ALL LAYERS OF DECK MATERIAL AT END LAPS AND SIDE JOINTS AND SHALL BE COMPLETELY FUSED TO THE SUPPORTING MEMBERS.
- COMPLETELY FUSED TO THE SUPPORTING MEMBERS.
   D) SIDE LAPS OF ADJACENT UNITS SHALL BE FASTENED BY WELDING (ON 18 GAUGE OR HEAVIER DECK ONLY) OR SHEET METAL SCREWS, SO THAT SPACING BETWEEN FASTENERS AND BETWEEN THE FIRST FASTENER AND SUPPORT DOES NOT EXCEED 18 INCHES.
   C) AT ALL ROOF OPENINGS AND PERIMETER ROOF EDGE CONDITIONS, ROOF
- c) AT ALL ROOF OPENINGS AND PERIMETER ROOF EDGE CONDITIONS, RC DECK UNITS SHALL BE WELDED TO EDGE STEEL USING 5/8"DIAMETER PUDDLE WELDS AT 6" O.C.
  2. PROVIDE A MINIMUM END BEARING OF 2" OVER SUPPORTS.
  3. END LAPS OF SHEETS SHALL BE A MINIMUM OF TWO INCHES AND SHALL OCCUR OVER SUPPORTS. ROOFS SHALL BE REECTED BEGINNING AT THE LOW SIDE TO INSURE THAT END LAPS ARE SHINGLE FASHION.
  4. ATTACH DECK TO EDGE SUPPORT WITH 5/8" DIA PUDDLE WELD @ 6" O.C.

## METAL FORM DECK (REF. SECTION 05600) . FORM DECK SHALL BE AS FOLLOWS:

- FORM DECK SHALL BE AS FOLLOWS:
  A. ALL METAL FORM DECK EXCEPT WHERE SHOWN ON PLAN SHALL BE 1 1/2" DEEP 24 GAGE WIDE RIB, GALVANIZED G90.
  B. FORM DECK SHALL BE PLACED IN AT LEAST TWO SPAN SEGMENTS. NO SINGLE SPAN CONDITIONS SHALL BE USED.
  C. STEEL DECK SHALL CONFORM TO ASTM A446 GRADES A, B, C, D, E OR F FOR GALVANIZED DECK, MINIMUM YIELD STRENGTH OF 33,000 PSI.
  D. STEEL DECK SHALL BE GALVANIZED WITH A PROTECTIVE ZINC COATING CONFORMING TO ASTM A525 G90 CLASS.
  E. ATTACHMENT:

#### E. ATTACHMENT: 1. WELDING:

- a) FORM DECK UNITS SHALL BE WELDED TO EACH STRUCTURAL SUPPORT FORM DECK UNIS SHALL BE WELDED TO EACH STRUCTURAL SUPPORT MEMBER USING 5/8" DIAMETER PUDDLE WELDS AT 12" SPACING MAX. WELD METAL SHALL PENETRATE ALL LAYERS OF DECK MATERIAL AT END LAPS AND SIDE JOINTS AND SHALL BE COMPLETELY FUSED TO THE SUPPORTING MEMBERS SUPPORTING MEMBERS.
- b) SIDE LAPS OF ADJACENT UNITS SHALL BE FASTENED BY WELDING (ON 18 b) Side LAPS OF ALGACENT UNITS SPACIAL BE FASIENED BI WELDING (ON TO GAUGE OR HEAVIER DECK ONLY) OR SHEET METAL SCREWS, SO THAT SPACING BETWEEN FASTENERS AND BETWEEN THE FIRST FASTENER AND SUPPORT DOES NOT EXCEED 18 INCHES.
   c) AT ALL ROOF OPENINGS AND PERIMETER ROOF EDGE CONDITIONS, ROOF
- c) AI ALL ROOF OPENINGS AND PERIMETER ROOF EDGE CONDITIONS, ROUDLE WINTS SHALL BE WELDED TO EDGE STEEL USING 5/8" DIAMETER PUDDLE WELDS AT 12" O.C.
   2. PROVIDE A MINIMUM END BEARING OF 2" OVER SUPPORTS.
   3. END LAPS OF SHEETS SHALL BE A MINIMUM OF TWO INCHES AND SHALL OCCUR OVER SUPPORTS. ROOFS SHALL BE ERECTED BEGINNING AT THE LOW SIDE TO INSURE THAT END LAPS ARE SHINCLE FASHION.

4. ATTACH DECK TO EDGE SUPPORT WITH 5/8" DIA PUDDLE WELD @ 12" O.C.

SHOP DRAWINGS AND OTHER SUBMITTALS (REF. SECT. 01330) REFER TO DIVISION 1 OF THE SPECIFICATIONS FOR SUBMITTAL PROCEDURES AND REQUIREMENTS. REFER TO THE APPLICABLE SPECIFICATION SECTIONS FOR TECHNICAL CONTENT REQUIREMENTS. INCOMPLETE SUBMITTALS WILL BE RETURNED WILHOUT REVIEW. SUBMIT SPECIFIC COMPONENTS, SUCH AS COLUMNS, FOOTINGS, ETC., IN A SINGLE PACKAGE. SUBMIT SIMILAR FLOORS TOGETHER. DO NOT USE OR REPRODUCE DESIGN DRAWINGS AS PART OF SHOP DRAWINGS WITHOUT THE WRITTEN PERMISSION OF THE ARCHITECT. WHEN CAD FILES OR COPIES OF THE DESIGN DRAWINGS ARE MADE AVAILABLE, IT IS UNDER THE FOLLOWING ALL INFORMATION CONTAINED IN THE CAD FILES OR COPIES OF THE DESIGN DRAWINGS ARE INSTRUMENTS OF SERVICE OF THE ARCHITECT AND SHALL NOT BE USED FOR OTHER PROJECTS, ADDITIONS TO THE PROJECT OR THE COMPLETION OF THE PROJECT BY OTHERS. CAD FILES AND COPIES OF THE PB Balloks DESIGN DRAWINGS REMAIN THE PROPERTY OF THE ARCHITECT AND IN NO CASE SHALL THEIR TRANSFER BE CONSIDERED A SALE; B. CAD FILES OR COPIES OF THE DESIGN DRAWINGS ARE NOT CONTRACT t Saint Louis, MO 63 DOCUMENTS. IN THE EVENT OF A CONFLICT, THE DESIGN DRAWINGS SHALL THE USE OF CAD FILES OR COPIES OF THE DESIGN DRAWINGS SHALL NOT N ANY WAY RELIEVE THE CONTRACTOR RESPONSIBILITY FOR PROPER CHECKING AND COORDINATION OF DIMENSIONS, DETAILS, SIZES AND QUANTITIES OF MATERIALS AS REQUIRED FOR THE PREPARATION OF SHOP DRAWINGS THAT ARE COMPLETE AND ACCURATE; AND, DI THE CONTRACTOR SHALL REMOVE INFORMATION THAT IS NOT REQUIRED FOR THEIR WORK FROM THE CAD FILES OR COPIES OF THE DESIGN DRAWINGS, INCLUDING THE TITLE BLOCK. CLEARLY FLAG AND CLOUD ALL CHANGES AND ADDITIONS MADE ON RESUBMITTALS. (AIZO) PINELLAS SUNCOAST TRANSIT AUTHORITY ONLY THOSE AND CLOUDED WILL BE REVIEWED. SUBMITTALS FOR CUSTOM DESIGNED, LOAD-CARRYING ITEMS THAT ARE REQUIRED BY CODES OR STANDARDS TO RESIST FORCES MUST BE PREPARED BY, OR UNDER THE DESCRIPTION DIRECT SUPERVISION OF A DELEGATED ENGINEER. EXAMPLES INCLUDE PRECAST CONCRETE, STRUCTURAL LIGHT GAGE STEEL FRAMING, EXTERIOR ENCLOSURE SYSTEMS, AND SHORING AND RESHORING. A DELEGATED ENGINEER IS DEFINED AS A REGISTERED PROFESSIONAL ENGINEER WHO A DELEGATED ENGINEER IS DEFINED AS A REDISTERED PROFESSIONAL ENGINEER SPECIALZES IN AND UNDERTAKES THE DESIGN OF STRUCTURAL COMPONENTS OR STRUCTURAL SYSTEMS INCLUDED IN A SPECIFIC SUBMITTAL PREPARED FOR THIS PROJECT AND IS AN EMPLOYEE OR OFFICER OF, OR CONSULTANT TO, THE CONSTRUCTION MANAGER OF FABRICATOR RESPONSIBLE FOR THE SUBMITTAL. THE DELEGATED ENGINEER SHALL SIGN, SEAL AND DATE THE SUBMITTAL, INCLUDING CALCULATIONS AND DRAWINGS. SEE SPECIFICATIONS FOR MORE SPECIFIC CRITERIA. THE CONTRACTOR SHALL REVIEW AND APPROVE SUBMITTALS AND SHALL SIGN AND DATE EACH DRAWING PRIOR TO SUBMITTING TO THE ARCHITECT. THIS APPROVAL IS TO CONFIRM THAT THE SUBMITTAL IS COMPLETE, COMPLIES WITH THE SUBMITTAL REQUIREMENTS AND IS COORDINATED WITH FIELD DIMENSIONS, OTHER TRADES, ERECTIO SEQUENCING AND CONSTRUCTABILITY. THE STRUCTURAL ENGINEER REVIEWS SUBMITTALS TO CONFIRM THAT THE SUBMITTAL IS IN GENERAL CONFORMANCE WITH THE DESIGN CONCEPT PRESENTED IN THE CONTRACT DOCUMENTS. QUANTITIES AND DIMENSIONS ARE NOT CHECKED. CONSTRUCTABILITY IS THE SOLE RESPONSIBILITY OF THE CONSTRUCTION THE STRUCTURAL ENGINEER'S REVIEW OF DELEGATED ENGINEER SUBMITTALS IS LIMITED TO VERIFYING THAT THE SPECIFIED STRUCTURAL SUBMITTAL HAS BEEN FURNISHED AND SEALED BY THE DELEGATED ENGINEER AND THAT THE DELEGATED ENGINEE AND SEALED BY THE DELEGATED ENGINEER AND THAT THE DELEGATED ENGINEER HAS UNDERSTOOD THE DESIGN INTENT AND USED THE SPECIFIED STRUCTURAL CRITERIA. NO DETAILED CHECK OF CALCULATIONS WILL BE MADE. ALL COMMENTS BY THE STRUCTURAL ENGINEER WILL BE MADE ON THE SHOP DRAWINGS. ority ₽ CALCULATIONS ARE FOR ARCHITECT'S AND ENGINEER'S RECORDS AND ARE NOT Auth APPROVED NOR RETURNED. 10. ALL SHOP DRAWINGS MUST BE REVIEWED AND STAMPED BY THE CONTRACTOR PRIOR TO TO SUBMITTAL TO THE ARCHITECT/ENGINEER. THE CONTRACTOR SHALL SUBMIT FOR THE ARCHITECTS/ENGINEER'S 12 REVIEW SHOP DRAWINGS FOR THE FOLLOWING ITEMS REINFORCING STEEL TA PRECAST CONCRETE SHOP DWGS.(\*) STRUCTURAL PRECAST CONCRETE ENGINEERING CALCULATION (#) FORWORK SHORING PLANS(\*)(#) CONCRETE MIX DESIGNS CONSTRUCTION JOINT LOCATIONS IN STRUCTURAL FLOORS 5 0 PRE-ENGINEERED METAL BLDGS.(\*)(#) TILT UP CONCRETE WALL SHOP DWGS WITH REINF. & EMBED PLACEMENTS. Su ERECTION & SHORING PLANS (\*)(#) TILT WALL BRACING DESIGN FOR TEMPORARY & CONSTRUCTION LOADS (\*)(#) STRUCTURAL STEEL SHOP DWGS (\*) STRUCTURAL STEEL CONNECTIONS ENGINEERING CALCULATIONS (\*) ITEMS MARKED (\*) SHALL HAVE SHOP DRAWINGS SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF FLORIDA, ITEMS MARKED (#) SHALL BE SUBMITTED FOR ENGINEERS RECORD ONLY. COMPOSITE METAL DECK & SLAB (REF. SECTION 05600) THE METAL DECK SHOWN ON THE DRAWINGS IS THE MINIMUM REQUIRED FOR UNSHORED CONSTRUCTION FOR THE TYPICAL CONDITIONS OF TWO CONTINUOUS SPANS OR MORE. THE DECK SUPPLIER SHALL INCREASE THE GAGE THICKNESS, IF NECESSARY, FOR SINGLE RECORD THE DECK GAGE AND DEPTH HAVE BEEN SELECTED BASED ON THE WET WEIGHT OF DOCUMENTS CONCRETE AND THE FINAL DESIGN LOADS ONLY. CONSTRUCTION MATERIALS MAY NOT BE PLACED ON THE BARE METAL DECK. 3. THE FINAL SLAB THICKNESS SHALL BE NO LESS THAN CALLED FOR ON THE PLANS. 4. CONTRACTOR IS TO PROVIDE ADDITIONAL CONCRETE REQUIRED DUE TO THE DEFLECTION THIS RECORD DOCUMENT HAS OF UNSHORED BEAMS AND DECK. BEEN PREPARED BASED ON NFORMATION PROVIDED BY 5. STEEL DECK SHALL CONFORM TO ASTM A446 GRADES A, B, C, D, E OR F FOR CALVANIZED DECK, MINIMUM YELD STRENGTH OF 33,000 PSI. 6. STEEL DECK SHALL BE GALVANIZED WITH A PROTECTIVE ZINC COATING CONFORMING TO OTHERS. THE A/E HAS NOT /ERIFIED THE ACCURACY AND/OR COMPLETENESS OF AND/OR COMPLETENESS OF THIS INFORMATION AND SHALL NOT BE RESPONSIBLE FOR AN' ERRORS OR OMISSIONS THAT MAY BE INCORPORATED HEREIN AS A RESULT. DATE OF SEAL SHEET CONTENT GENERAL NOTES ABBREVIATIONS SYMBOLS SCALE DRAWN BY CHECKED BY PROVED B PROJECT NUMBER S0.01

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DAT

Drive Florida

1/8" = 1'-0

T.F

P.A

#### STRUCTURAL STEEL (REEF SECTION 05120)

- STEEL SHALL CONFORM TO THE FOLLOWING ASST. SPECIFICATION:
- A. W-SHAPES (O.N.O..), A572(GR50)
- B. STRUCTURAL STEEL PIPE (FY-35)
- C. STRUCTURAL STEEL TUBE (FY-46) D. BASE PLATES AND CONNECTION PLATES (FY=36)
- E. ALL OTHER STEEL (FY=36) 2. BOLTED CONNECTIONS:
- A. ALL TRUSS CONNECTIONS, A325-SC, SLIP CRITICAL, SIZES AS SHOWN IN THE DETAILS B. LOAD WASHERS MUST BE USED FOR FIELD CONNECTIONS. C. ALL BEAM TO COLUMN CONNECTIONS, BRACE CONNECTIONS AND MOMENT
- CONNECTED MEMBERS SHALL HAVE A325-N
- D. ALL OTHER CONNECTIONS (UNLESS NOTED OTHERWISE) 3/4" DIAMETER, A325 N E. OVERSIZED AND LONG-SLEEVED HOLES ARE PERMITTED ONLY WHEN DESIGN BY CONNECTION DELEGATED ENGINEER IS SUBMITTED AND APPROVE PRIOR TO SUBMISSION OF STEEL CONNECTION SHOP DWGS. STRUCTURAL DRAWINGS.
- F. THE SHOP DRAWINGS SHALL CLEARLY INDICATE THE TYPE OF BOLT USED IN EACH CONNECTION AND THE ALLOWABLE VALUES USED FOR THE VARIOUS BOLT TYPES.
- G. THE FOLLOWING MINIMUM STANDARDS APPLY: (U.N.O.)
- a) MINIMUM PLATE THICKNESS" 3/8"
- b) MINIMUM BOLT DIAMETER" 3/4"
- c) MINIMUM WELD" THICK THROAT 3/16"
- d) MINIMUM DESIGN LOAD ON ANY CONNECTION 10 kips
- 5. WELDED CONNECTIONS: A. ALL SHOP AND FIELD WELDING SHALL CONFORM TO WAS STRUCTURAL WELDING CODE, ANSI D1.1
- B. MINIMUM WELD: 3/16" THICK THROAT. C. ALL STEEL TO STEEL CONNECTIONS NOT SHOWN BOLTED SHALL BE WELDED TO DEVELOP FULL SHEAR CAPACITY OF CONNECTING MEMBERS AS PER AISC SPECIFICATIONS. MINIMUM SIZE OF FILLET WELD (UNLESS NOTED OTHERWISE ON DRAWINGS):
- ATERIAL THICKNESS OF MINIMUM SIZE OF THICKER PART JOINED FILLET WELD: TO 1/4" INCLUSIVE 1/8" ALL AROUND
- TO 1/4" TO 1/2" 3/16" ALL AROUND
- OVER 1/2" TO 3/4" 1/4" ALL AROUND
- OVER 1/2 10 5/4 1/2 1/2" ALL AROUND OVER 3/4" TO 1 1/2" 5/16" ALL AROUND 4. SPLICING OF STRUCTURAL STEEL WHERE NOT DETAILED IS PROHIBITED WITHOUT PRIOR WRITTEN APPROVAL OF THE ARCHITECT.
- BEAM CONNECTIONS: A. DESIGN CONNECTIONS FOR BEAMS TO SUPPORT 0.55 OF THE UNIFORM CAPACITY SHOWN IN AISC "TABLES FOR ALLOWABLE LOADS ON BEAMS" FOR THE GIVEN SECTION AND SPAN UNLESS OTHERWISE SHOWN.
- 6. GALVANIZING:
- GALVANIZING: A. HOT-DIP GALVANIZE AFTER FABRICATION ALL STRUCTURAL STEEL ITEMS AND THEIR CONNECTIONS PERMANENTLY EXPOSED TO THE OUTSIDE. B. EXAMINE DRAWINGS FOR OTHER ITEMS TO BE GALVANIZED.
- ANCHOR BOLTS EXCEPT FOR STEEL TRUSSES:
- A. ALL ANCHOR BOLTS SHALL BE MADE FROM THREADED ROUND STOCK, ASTM A36. ALL ANCHOR BOLTS, NUTS AND WASHERS USED WITH GALVANIZED BASE PLATES SHALL BE HOT DIP GALVANIZED AFTER FABRICATION.
- B. NUTS SHALL BE HEX HEAD, ASTM A563.
   C. WASHERS FOR ALL BASE PLATES SHALL BE 1/4" THICK PLATES EXTENDING MINIMUM That is to all back of the ba
- TEMPLATES SHALL BE DETAILED ON THE SHOP DRAWINGS. NON-SHRINK GROUT FOR BASE PLATES AND BEARING PLATES A. NON-METALLIC, SHRINKAGE RESISTANT, PREMIXED, NON-CORROSIVE, NON-STAINING PRODUCT CONTAINING PORTLAND CEMENT SILICA SANDS SHRINKAGE COMPENSATING AGENTS, AND FLUIDITY IMPROVING COMPOUNDS, AND SHALL CONFORM TO CORPS OF ENGINEERS SPECIFICATION FOR NON-SHRINK GROUT, CRD-C621-83.
- B. TWENTY-EIGHT DAY COMPRESSIVE STRENGTH SHALL BE A MINIMUM OF 6,000 PSI. SUBMITTALS:
- A. SEE INDIVIDUAL SPECIFICATION SECTIONS FOR DESIGN RESPONSIBILITIES. AND THE STRUCTURAL ENGINEER OF RECORD'S REVIEW RESPONSIBILITIES WITH REGARD TO SUBMITTAL REVIEW.
- B. SEE STRUCTURAL SPECIFICATIONS FOR ADDITIONAL SUBMITTAL REQUIREMENTS AND PROCEDURES. C. REPRODUCTION OF STRUCTURAL DRAWINGS FOR SHOP DRAWINGS WILL NOT BE PERMITTED.
- D. REVIEW OF SUBMITTALS OR SHOP DRAWINGS BY THE STRUCTURAL ENGINEER DOES NOT RELIEVE THE CONTRACTOR OF THE SOLE RESPONSIBILITY TO REVIEW AND CHECK ALL SUBMITTALS AND SHOP DRAWINGS BEFORE SUBMITTING TO THE STRUCTURAL ENGINEER. CONTRACTOR REMAINS SOLELY RESPONSIBLE FOR ERRORS AND OMISSIONS ASSOCIATED WITH THE PREPARATION OF SHOP DRAWINGS AS THEY PERTAIN TO MEMBER SIZES, DETAILS, AND DIMENSIONS SPECIFIED IN THE CONTRACT DOCUMENTS

### ENGINEERED LIGHT-GAGE METAL FRAMING (REF SECTION 05400)

- DESIGN OF ENGINEERED LIGHT-GAGE METAL FRAMING IS THE SOLE RESPONSIBILITY OF THE METAL SUPPLIER. SUBMIT SHOP DRAWINGS SEALED BY AN ENGINEER (DELEGATED ENGINEER) LICENSED IN THE STATE OF FLORIDA EVIEW OF SHOP DRAWINGS SHALL BE FOR CONFORMANCE WITH THE CONTRACT DOCUMENTS REGARDING ARRANGEMENT AND SIZES OF MEMBERS AND THE CONTRACTOR'S INTERPRETATION OF THE DESIGNS LOADS AND CONTRACT DOCUMENT DETAILS. SUCH REVIEW DOES NOT RELIEVE THE CONTRACTOR OF FULL RESPONSIBILITY FOR THE DESIGN OF THE ENGINEERED LIGHT-GAGE METAL FRAMING AND THEIR CONNECTIONS.
- LIGHT-GAGE METAL FRAMING SHALL BE DESIGNED IN ACCORDANCE WITH THE DESIGN/ CODE INFORMATION LISTED AT THE BEGINNING OF THESE STRUCTURAL GENERAL NOTES.
- DESIGN OF LIGHT GACE METAL FRAMING SHALL CONFORM TO THE LATEST EDITION OF SPECIFICATION FOR THE DESIGN OF COLD-FORMED STRUCTURAL STEEL MEMBERS, AISI

- OPEN WEB STEEL JOISTS (REF. SECTION 05210)
- . ALL STEEL JOISTS SHALL CONFORM TO THE STANDARDS OF THE STEEL JOIST INSTITUTE
- 2. THE JOIST MANUFACTURER SHALL REVIEW THE DRAWINGS AND PROVIDE JOISTS
- CAPABLE OF CARRYING THE FOLLOWING LOADS:
- A. ROOF DEAD LOAD......10 PSF B. SUPERIMPOSED DEAD LOAD......10
- REFER TO GROSS UPLIFT DIAGRAMS, DWG S0.03 AND SUPERIMPOSED DEAD LOADS TABULATED FOR NET UPLIFT DESIGN OF JOISTS BY FABRICATOR, BUT NOT LESS THAN 20 PSF
- NOT USED
- 4. THE JOISTS SHOWN ON THE PLANS ARE THE MINIMUM SIZE REQUIRED.
- DEPTHS SHOWN MAY NOT BE EXCEEDED. 5. JOISTS SHOULD BE CAMBERED IN ACCORDANCE WITH S.J.I. STANDARD CAMBERS. 5. JOISTS SHALL BE WELDED TO ALL SUPPORTING BEAMS WITH A MINIMUM OF
- TWO 3/16 INCH BY 2 INCH LONG FILLET WELDS, OR FOR THE UPLIFT FORCE WHICHEVER IS GREATER. 7. PROVIDE A MINIMUM END BEARING ON STEEL SUPPORTS AS REQUIRED BY SJI.
- PROVIDE A MINIMOM END BEARING ON SIELE SUPPORTS AS REQUIRED BY SUL STAGGER THE ENDS OF JOISTS IN RECESSARY. PROVIDE HORIZONTAL OR DIAGONAL TYPE BRIDGING FOR ALL JOISTS AS REQUIRED BY SUL SPECIFICATION. THE ENDS OF ALL BRIDGING LINES TERMINATING AT WALLS OR BEAMS SHALL BE ANCHORED THERETO AT TOP AND BOTTOM CHORDS. PROVIDE ALL REQUIRED BRIDGING ANCHORS. PROVIDE ALL JOISTS AND ACCESSORIES WITH ONE SHOPCOAT OF PAINT AS SPECIFICIENT ON SUL STANDARDS DEPEADE SULFACE AS REGUIRED BY DAINT
- SPECIFIED IN SJI STANDARDS. PREPARE SURFACE AS REQUIRED BY PAINT MANUFACTURERS RECOMMENDATIONS AND PROVIDE A CONTINUOUS DRY PAINT THICKNESS OF NOT LESS THAN 1 MIL. STEEL JOISTS THAT ARE TO RECEIVE SPRAYED FIREPROOFING. PAINT ADHESION SHALL CONFORM TO THE SPECIFICATIONS OF THE UNDERWRITERS LABORATORIES.
- 10. REFER TO THE SPECIFICATIONS FOR ALL OTHER REQUIREMENTS. 11. EXAMINE ARCHITECTURAL, STRUCTURAL AND MECHANICAL DRAWINGS FOR ANY
- ACCESSORIES OR ATTACHMENTS REQUIRED TO BE PROVIDED FOR STEEL JOISTS.

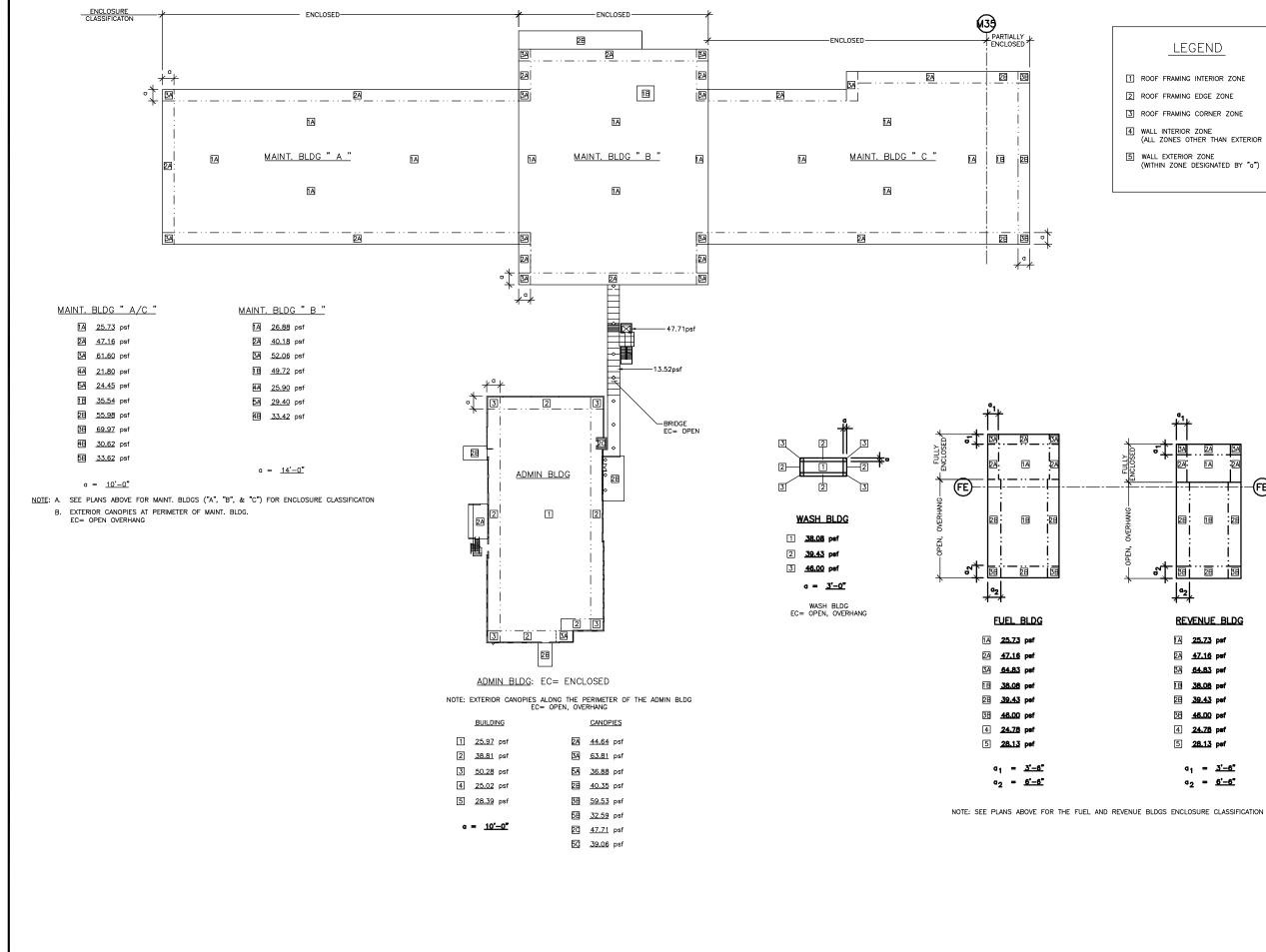
PRE-ENGINEERED METAL BUILDINGS (REF. SECTION 13125)

- DESIGNS OF PRE ENGINEERED SYSTEMS SPECIFIED IN THE CONTRACT DOCUMENTS WHICH ARE DESIGNED/ ENGINEERED BY OTHERS IS THE SOLE RESPONSIBILITY OF THE PEMB SUPPLIER. SUBMITTALS OF SUCH SYSTEMS SHALL BE SEALED BY AN ENCINEE LICENSED IN THE PROJECT STATE. REVIEW OF SUBMITTALS BY THE STRUCTURAL ENGINEER SHALL BE FOR CONFORMANCE WITH THE CONTRACT DOCUMENTS WITH REGARD TO THE ARRANGEMENT AND SIZES OF MEMBERS SHOWN ON THE STRUCTURAL CONTRACT DOCUMENTS AND THE CONTRACTOR'S INTERPRETATION OF THE DESIGN INFORMATION INCLUDED IN THE CONTRACT DOCUMENTS. SUCH REVIEW BY THE STRUCTURAL ENGINEER SHALL NOT IMPLY ANY RESPONSIBILITY FOR THE ACTUAL DESIGNS OF SUCH SYSTEMS. CONTRACTOR HAS FULL RESPONSIBILITY FOR DIMENSIONAL ACCURACY AND CONFORMANCE WITH THE INFORMATION CONTAINED IN THE CONTRACT
- DOCUMENTS. 2. SEE SPECIFIC SECTIONS OF STRUCTURAL GENERAL NOTES BELOW AND SPECIFICATIONS FOR THE APPROPRIATE DESIGN RESPONSIBILITIES OF THE CONTRACTOR
- S A CERTIFIC TESTING AGENCY SHALL BE ENGAGED BY CONTRACTOR TO PERFORM INDUSTRY STANDARD INSPECTIONS TO ENSURE CONFORMANCE WITH PLANS AND SPECIFICATIONS (IF PROVIDED). SUBMIT REPORTS TO ARCHITECT AND
- ENGINEER. 4. ALL STRUCTURAL STEEL SHALL BE MINIMUM ASTM A36 UNLESS NOTED OTHERWISE.
- STRUCTURAL SUBMITALS ARE REQUIRED FOR REVIEW AS AN INDICATION THAT INTENT HAS BEEN UNDERSTOOD AND THAT SPECIFIED CRITERIA HAVE BEEN USED.
   STRUCTURAL SUBMITALS SHALL IDENTIFY THE PROJECT AND LIST LOADING AND OTHER DESIGN CRITERIA. FABRICATION AND ERECTION DRAWINGS SHALL
- INDICATE IN DETAIL THE CONSTRUCTION OF THE STANDARD STRUCTURE USED OR AS MODIFIED TO COMPLY WITH THE REQUIREMENTS OF THE PARTICULAR PROJECT. THEY SHALL INDICATE ALL CONNECTION DETAILS, OPENINGS AND OTHER SPECIAL DETAILS. THEY SHALL INDUCATE ALL CONNECTION DETAILS, OPENINGS AND OTHER SPECIAL DETAILS. THEY SHALL SHOW THE MAGNITUDE AND LOCATION OF BUILDING REACTIONS ON THE FOUNDATION UNDER ALL DESIGN CONDITIONS. CALCULATIONS SUPPORTING THE DESIGN SHALL BE SUBMITTED NOT ONLY FOR THE STANDARD STRUCTURE BUT FOR MODIFICATIONS AND FOR RELATED
- THE STANDARD STRUCTURE BUT FOR MODIFICATIONS AND FOR RELATED COMPONENTS REQUIRING STRUCTURAL DESIGN. COLUMN REACTIONS AND ANCHOR BOLT GROUP DESIGN SHALL BE DETERMINED BY METAL BUILDING MANUFACTURER. TO TRANSMIT LOADS TO THE FOUNDATIONS. THESE DESIGNS SHALL BE SUBMITTED TO THE ENGINEER FOR FINAL COORDINATION WITH FOUNDATION. NO FABRICATION OF PEMB SHALL BEGIN PRIOR TO APPROVED ANCHOR BOLT DESIGNS. MOMENT FRAME BRACING LOCATIONS AS LATERAL RESISTING ELEMENTS ARE SHOWN ON THE DWGS.
- SUBMIT LOCATION OF ALL LATERAL RESISTING ELEMENTS FOR APPROVAL. NO X-BRACING IS PERMITTED UNLESS LOCATIONS ARE APPROVED BY ARCHITECT. 9. PROVIDE MIDHEIGHT HORIZONTAL STRUCTURAL MEMBER TO BRACE ALL EXTERIOR CMU AND TILT WALLS
- EXCEEDING 19ET HEIGHT

TILT-UP CONCRETE PANELS (REF. SECTION 03470)

- REINFORCING STEEL SHOWN ON PLANS IS FOR THE DESIGN OF SLABS INPLACE. ANY ADDITIONAL REINFORCING STEEL REQUIRED FOR LIFTING, TOGETHER WITH THE NECESSARY LIFTING INSERTS AND FASTENERS. IS TO
- BE PROVIDED BY THE TILT-SLAB CONTRACTOR-ENGINEER. 2. THE PANELS SHALL BE SUPPORTED TEMPORARILY IN PLACE AND BRACED TO WITHSTAND ALL LOADS UNTIL THEY ARE SUPPORTED BY THE STRUCTURAL STEEL MEMBERS AND ROOF DECKING.
- SIELL MEMBERS AND ROUP DECKING. 3. ALL PANELS SHALL BE ALIGNED AND CONNECTED TOGETHER. 4. GROUTING UNDER PANELS SHALL BE CONTINUOUS AND ACROSS FULL WIDTH OF PANELS, AFTER PANELS ARE PLUMB AND SET FOR THEIR FINAL POSITION. 5. FOR SIZE AND LOCATION ON EMBEDS FOR BEAM CONNECTIONS, SEE FLOOR AND ROOF PLANS AND CONNECTION DETAILS.
- CONCRETE FOR ALL PANELS I'C AT 28 DAYS = 5000 PSI, fy = 60 KSI. LOCATE ALL EMBEDS CORRECTLY IN THE CAST PANELS. 7. PANEL DIMENSIONS TO BE CHECKED FOR ACCURACY BY CONTRACTOR.
- 8. VERIFY WITH MECHANICAL, ELECTRICAL, AND PLUMBING FOR OPENINGS IN
- CONCRETE TILT-UPS. 9. ALL PANELS SHALL BE 9 1/4" THICK UNLESS NOTED OTHERWISE. 10. ALL PANELS SHALL BE REINFORCED WITH #4's @12"o.c. HORIZONTAL AND VERTICAL, U.N.O. 2-#5 CONTINUOUS AT BOTTOM
- OF PANEL, 2-#5 CONTINUOUS AT EACH SIDE AND TOP. 11. SEE STRUCTURAL DETAILS FOR DOWELS TO SLAB AND INSERTS AND PLATES FOR SUPPORT OF JOISTS AND BEAMS AND PANEL CONNECTIONS
- 12. PANELS SHALL NOT BE LIFTED UNTIL CONCRETE HAS ATTAINED A MINIMUM COMPRESSIVE STRENGTH I'C OF 2500 PSI.
- DELEGATED ENGINEER SHALL PROVIDE SHORING PLANS SIGNED AND SEALED BY FLORIDA PE FOR THE ERECTION OF THE PANELS.
   CONTRACTOR TO PROVIDE PANEL SHOP DRAWINGS WITH ALL EMBEDS.
- ALL PANEL DIMENSIONS SHALL BE PROVIDED ON SHOP DRAWINGS. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO LOCATE ALL
  - EMBEDS CORRECTLY IN THE CAST PANELS.



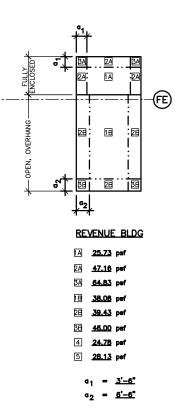


## WIND UPLIFT ROOF PRESSURES (GROSS)

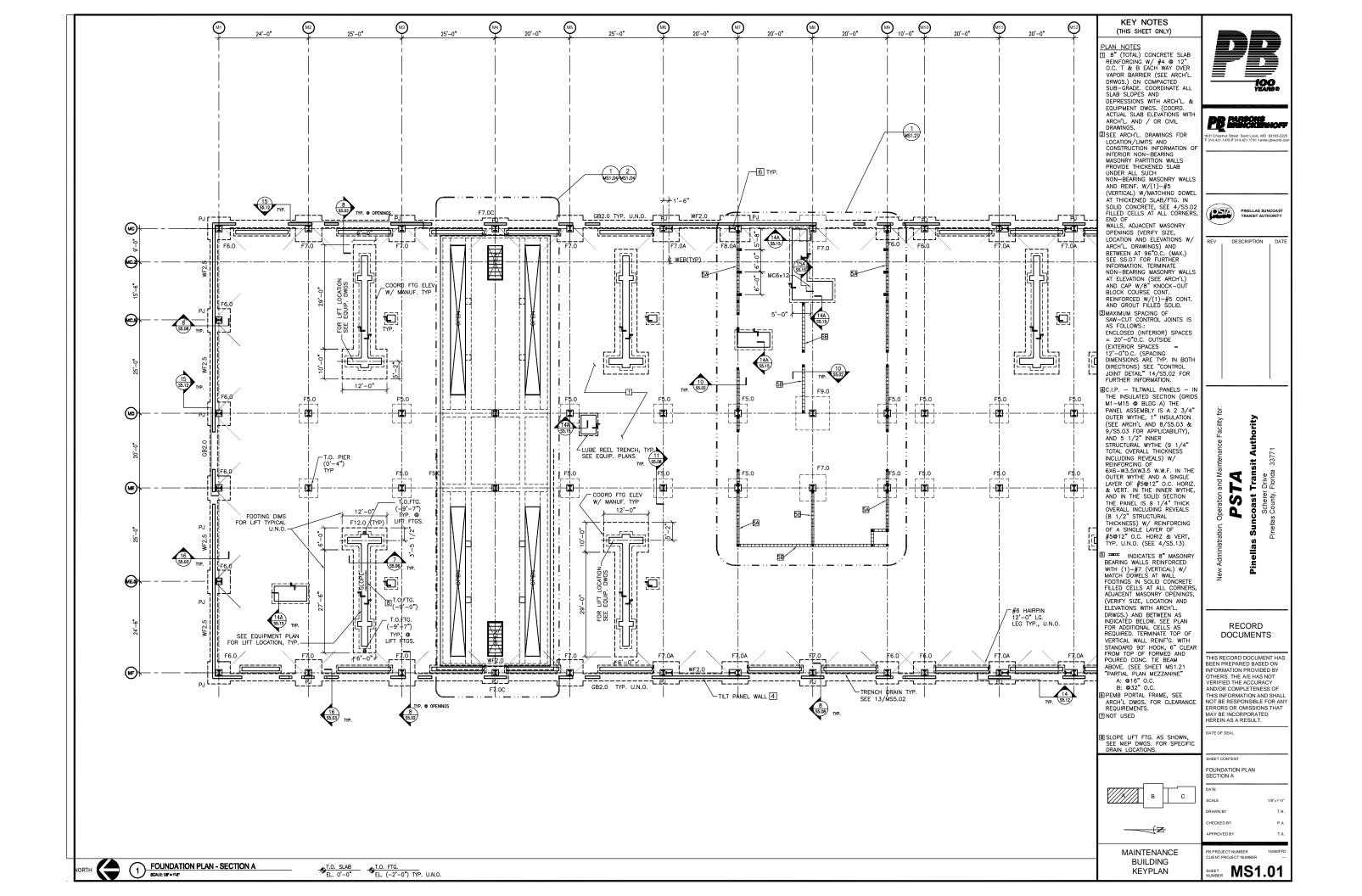
## LEGEND

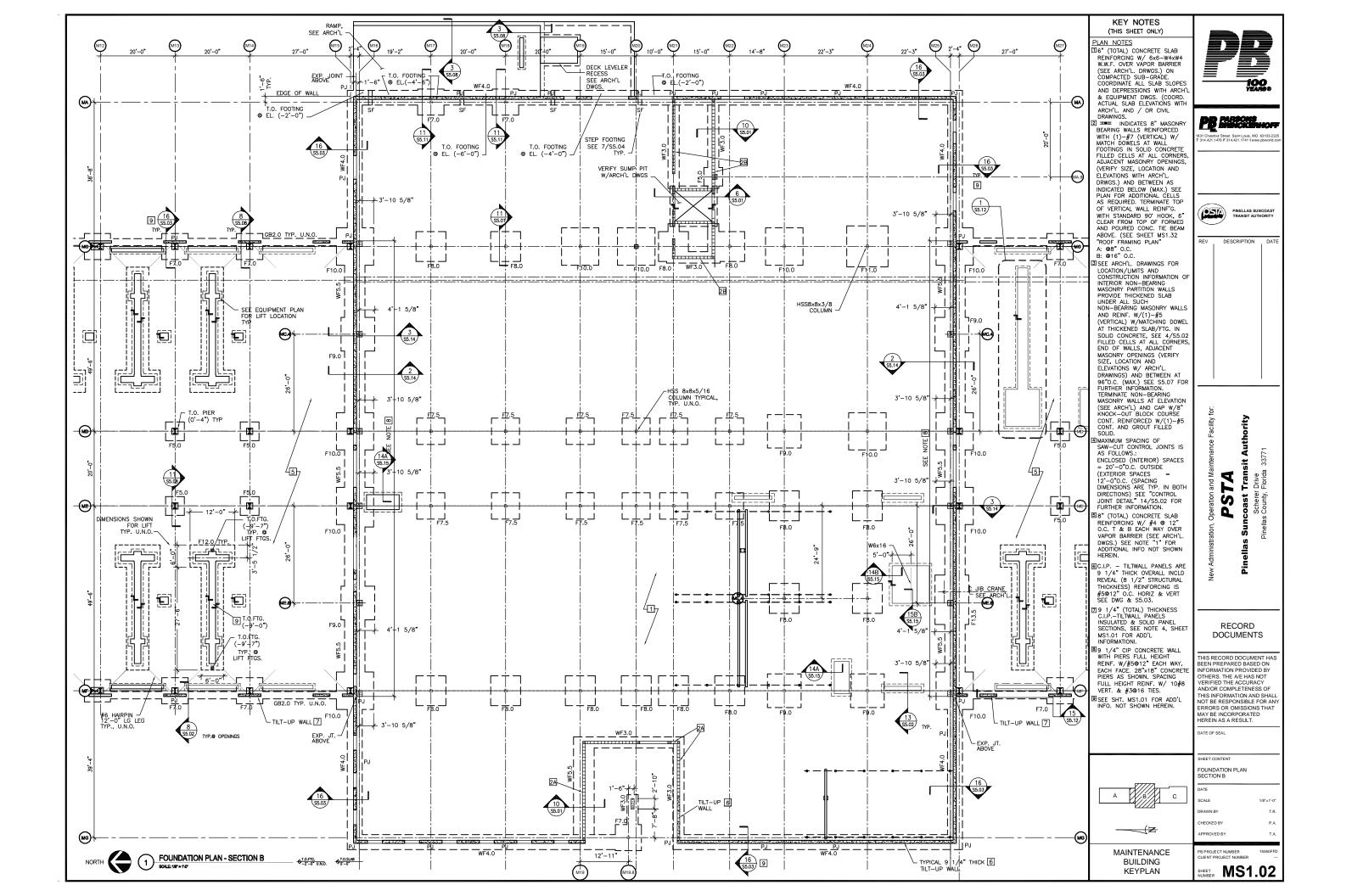
1	ROOF	FRAMING	INTERIOR	ZONE
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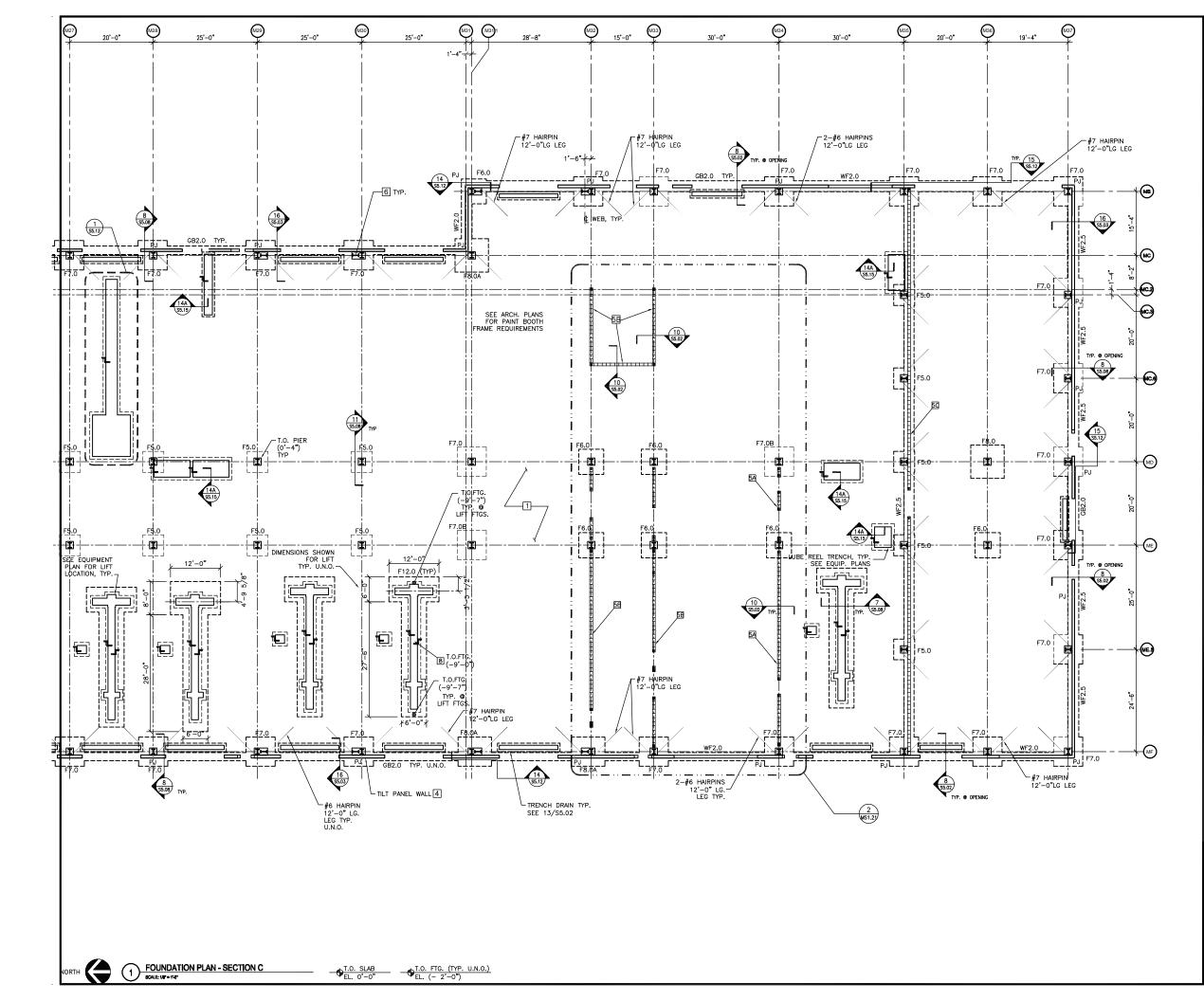
- 2 ROOF FRAMING EDGE ZONE
- 3 ROOF FRAMING CORNER ZONE
- 4 WALL INTERIOR ZONE (ALL ZONES OTHER THAN EXTERIOR ZONE)
- WALL EXTERIOR ZONE (WITHIN ZONE DESIGNATED BY "a")

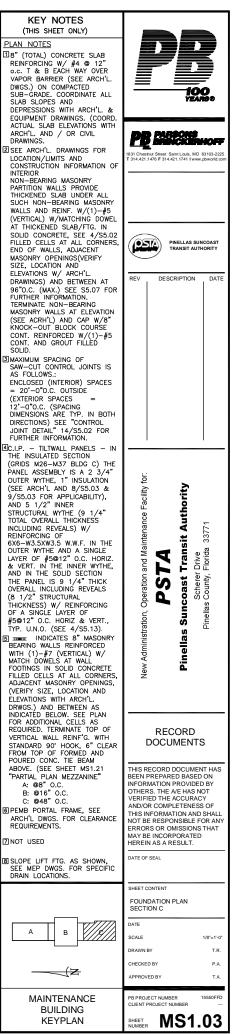


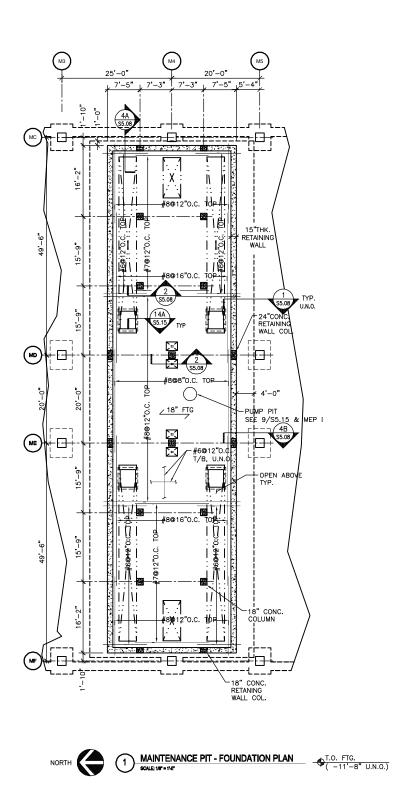


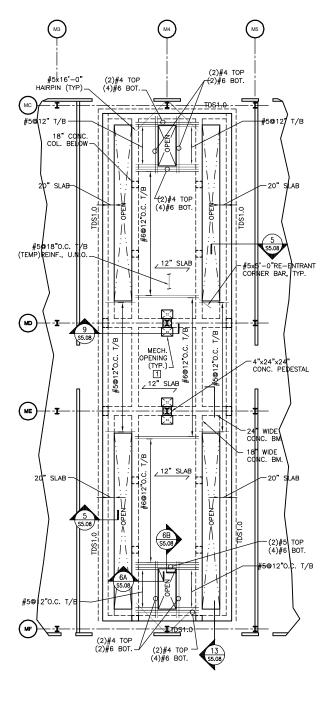






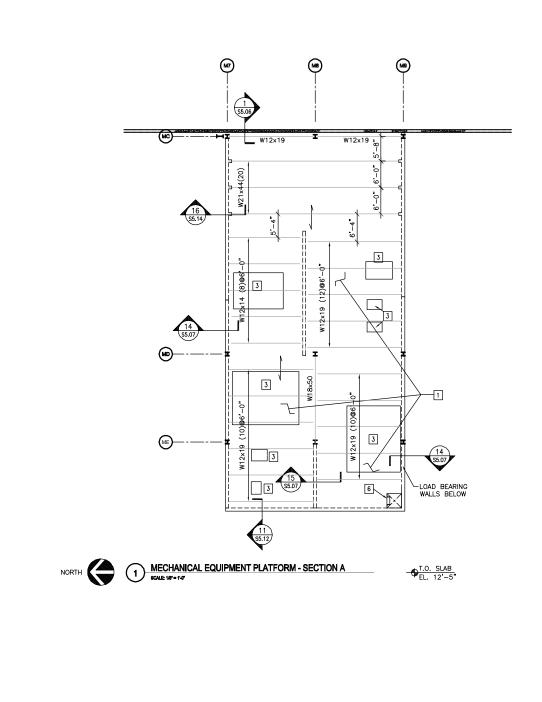


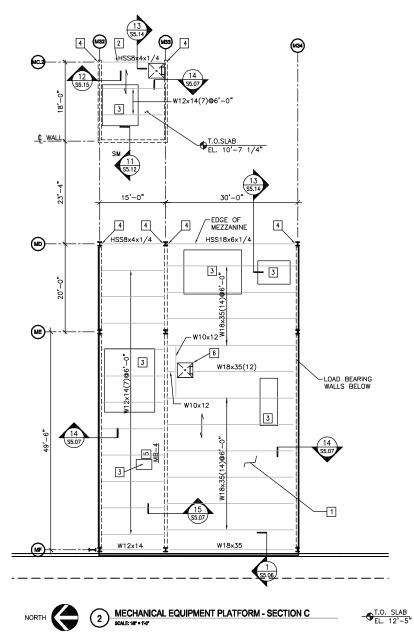




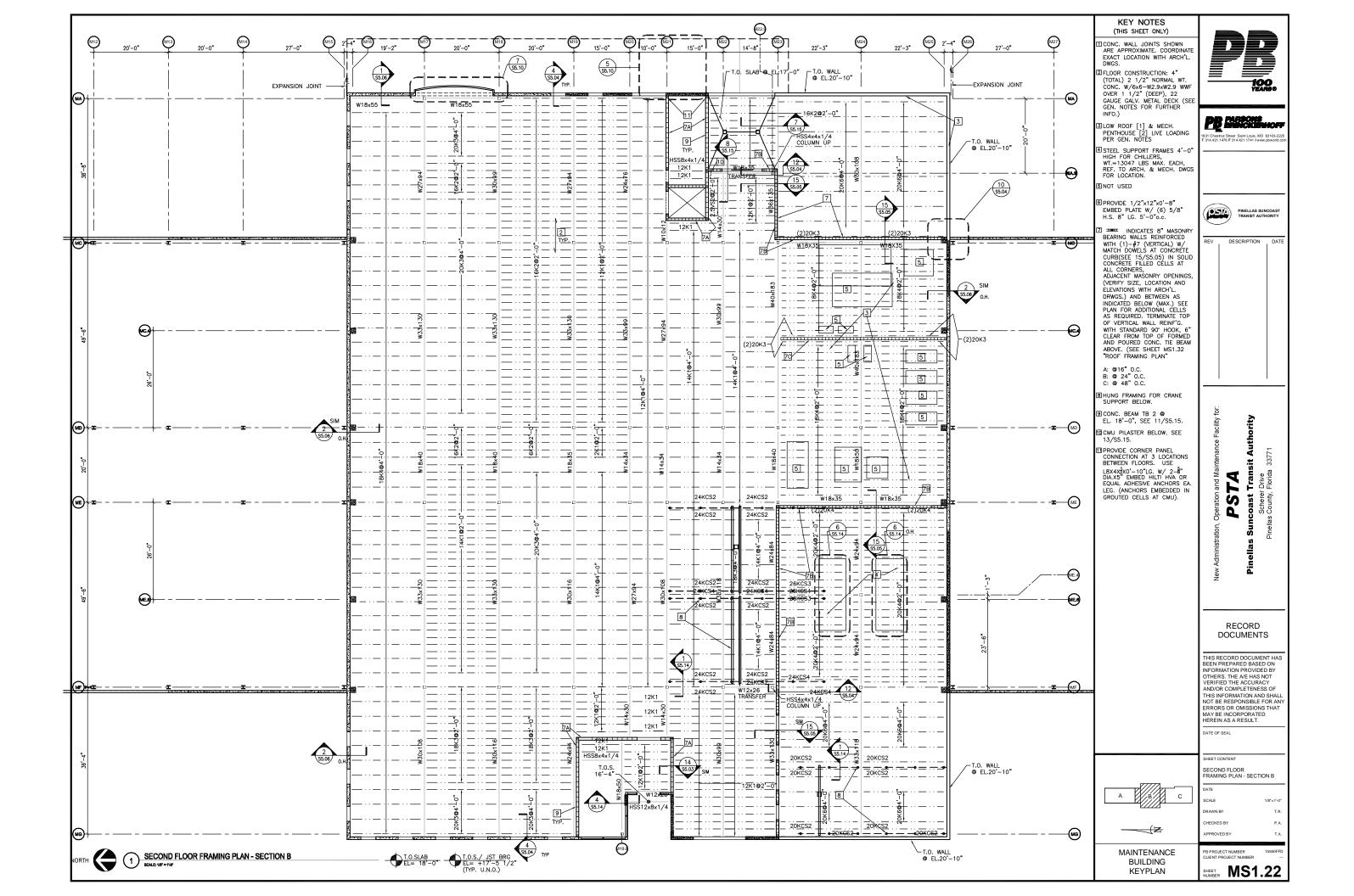
NORTH 2 MAINTENANCE PIT - SLAB PLAN SCALE: 18" - 15" 

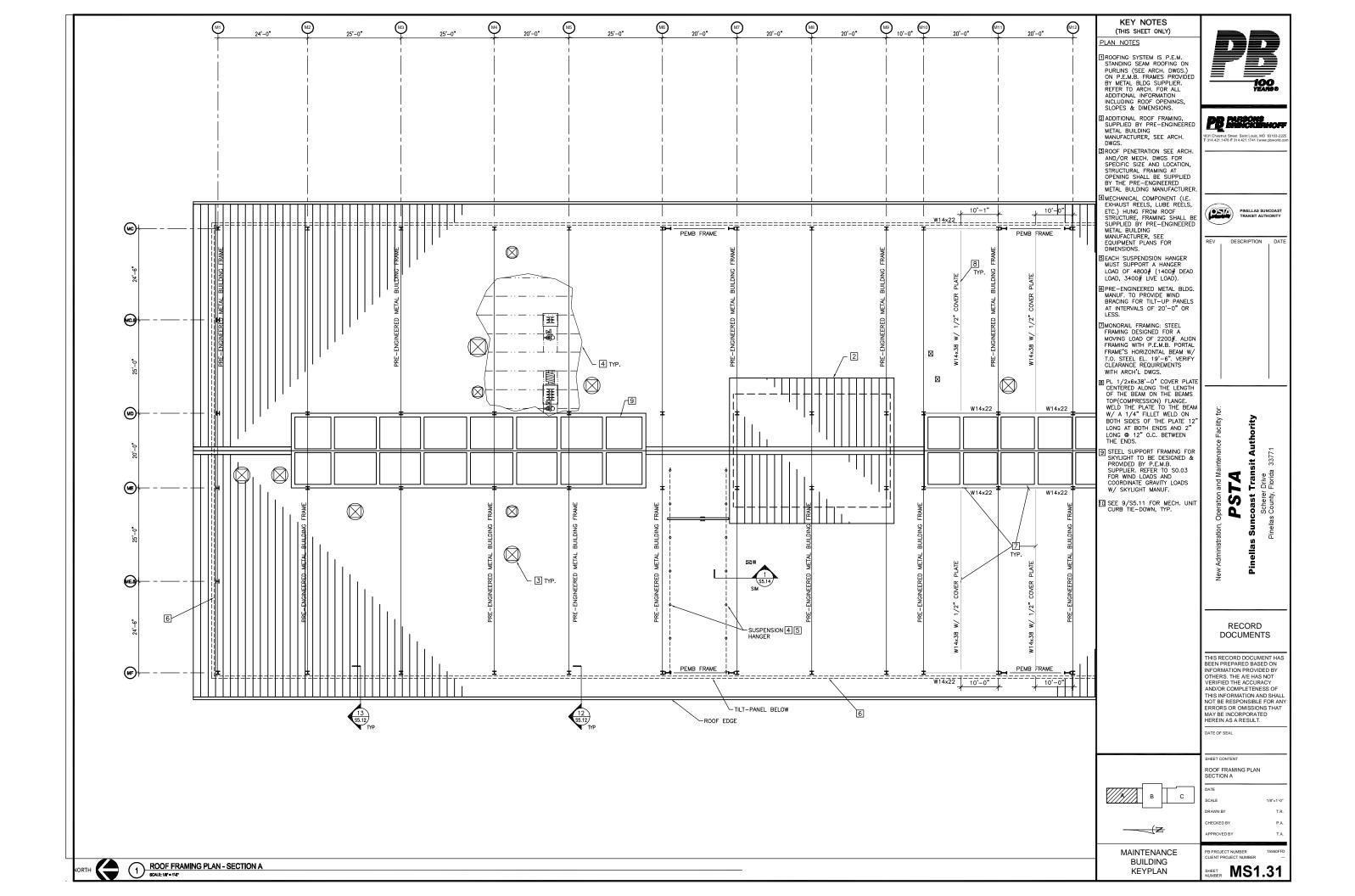
KEY NOTES	1
(THIS SHEET ONLY)	
1 SEE MECHANICAL DWGS. FOR SIZE & LOCATION OF	
MECHANICAL OPENINGS.	
	DR PARPONS
	1831 Cheshut Street Saint Louis, MO 63103-2225 T 314.421.1476 F 314.421.1741 I www.pbworld.com
	1 314.421.1470 P 314.421.1741 T www.powork.com
	PINELLAS SUNCOAST TRANSIT AUTHORITY
	<u> </u>
	REV DESCRIPTION DATE
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	New Administration, Operation and Maintenance Facility for. <b>PSTA</b> Pinellas Suncoast Transit Authority Scherer Drive Pinellas County, Florida 33771
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	THIS INFORMATION AND SHALL NOT BE RESPONSIBLE FOR ANY ERRORS OR OMISSIONS THAT
	MAY BE INCORPORATED HEREIN AS A RESULT.
	DATE OF SEAL
	SHEET CONTENT
	MAINTENANCE PIT
	FOUNDATION & SLAB PLANS
	SCALE 1/8"=1'-0"
	DRAWN BY T.R. CHECKED BY P.A.
	CHECKED BY P.A. APPROVED BY T.A.
	PB PROJECT NUMBER 15550FFD
	CLIENT PROJECT NUMBER
	SHEET MS1.04

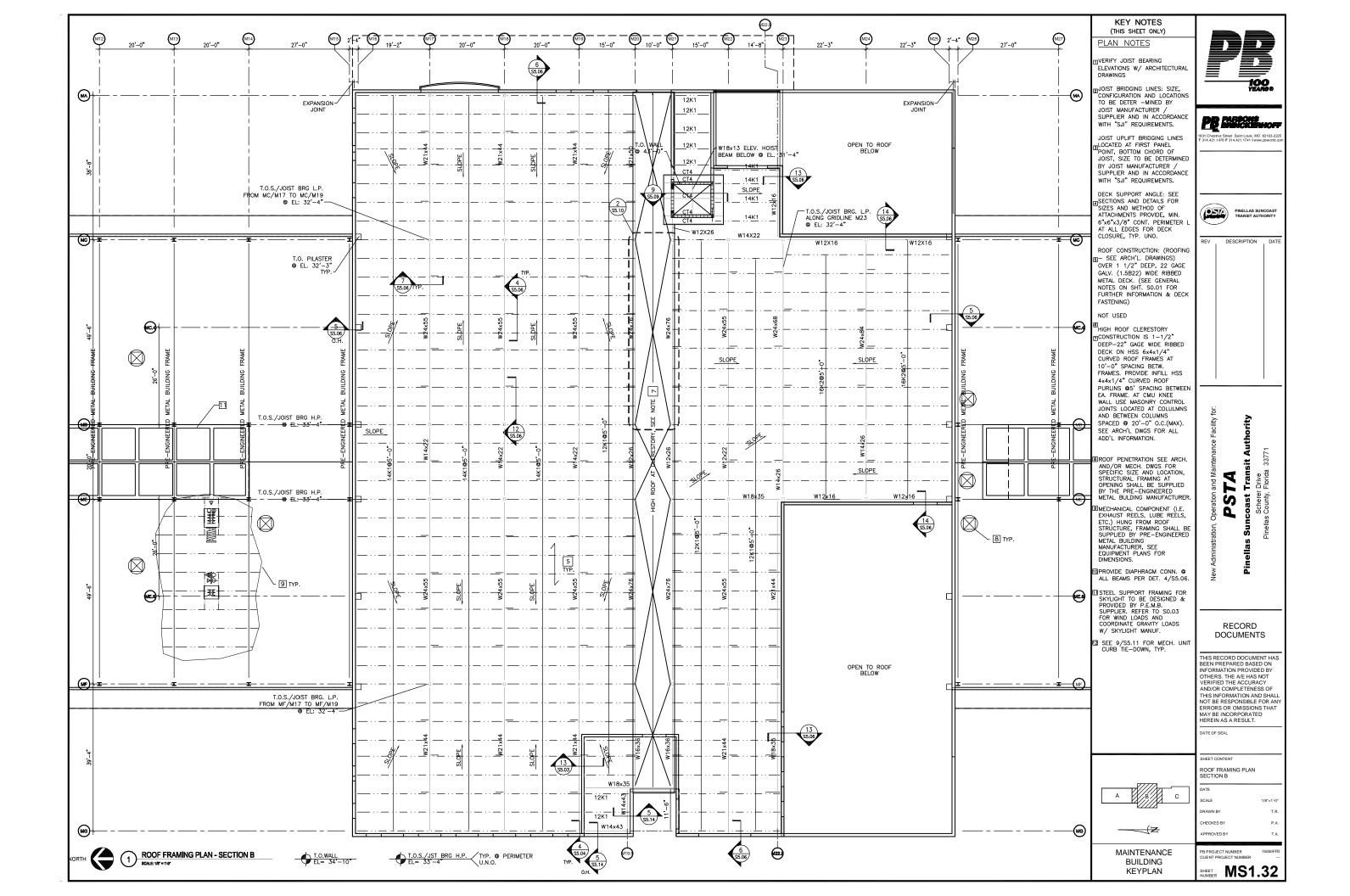


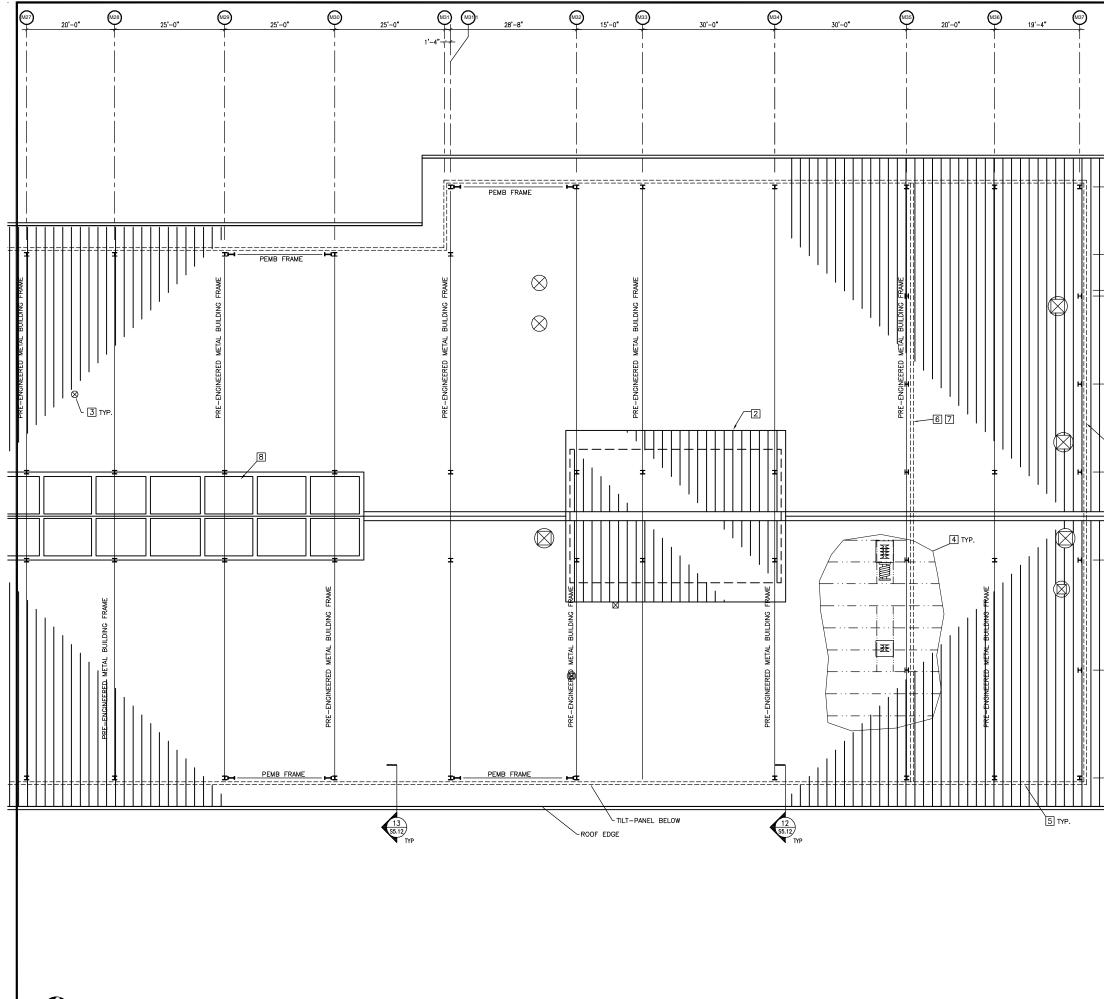


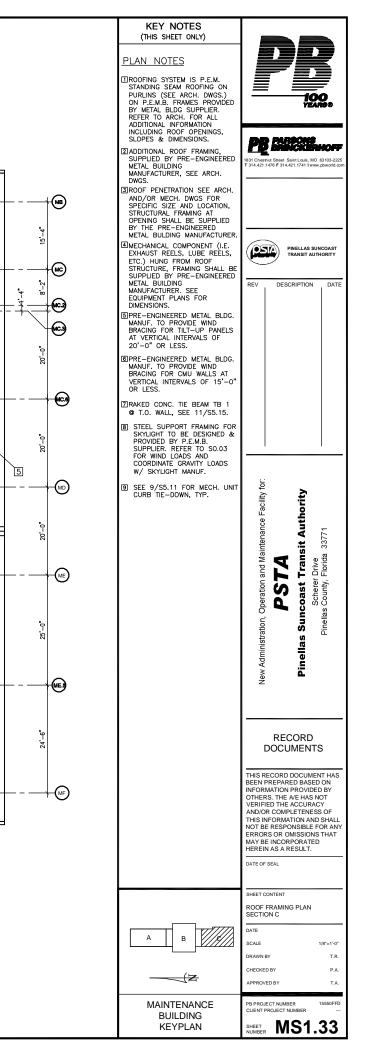
KEY NOTES (THIS SHEET ONLY)	
MEZZANINE SLAB CONSTRUCTION: 5" (TOTAL) NORMAL WEIGHT CONCRETE REINFORCED W/ Gx6-W1.4xW1.4 W.W.F. OVER ZVLIZO (2'DEEP, 20 GAGE) COMPOSITE METAL DECK (SEE GENERAL NOTES ON SHEET SO.01 FOR FURTHER INFORMATION) ATTACH COMPOSITE DECK WITH 36/4 PATERN OF WELDING WASHERS. PERMETER WELDS AT 12" O.C. MAX. #10 TEK. SCREWS OR EQUIV. SIDE LAP BETWEEN EA. SUPPORT	In the second se
2 STEEL FRAMING SUPPLIED BY PRE-ENGINEERED METAL BLDG. MANUFACTURER, TYP.     3 PROVIDE 6" HOUSEKEEPING	
PAD, SEE 9/S5.02 AND MEP DWGS FOR SIZE AND LOCATION.	PINELLAS SUNCOAST TRANSIT AUTHORITY
SPECIAL BEAM REACTION DUE TO HANDRAIL: VERTICAL = 12 KIPS, HORIZONTAL = 1 KIPS, TORSION = 3 KIP-FT.	REV DESCRIPTION DATE
SEE MASONRY LINTEL SCHEDULE (3/S5.12). TYPICAL LINTELS SPECIFIED BY LENGTH. SPECIAL LINTELS ARE NOTED ON PLANS.	
6 LADDERS, SEE ARCH'L.	
	· · ·
	New Administration, Operation and Maintenance Facility for: <b>PSTA</b> <b>Pinellas Suncoast Transit Authority</b> Scherer Drive Pinellas County, Florida 33771
	RECORD DOCUMENTS
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	DATE SCALE 1/8"=1'-0" DRAWN BY T.R.
	CHECKED BY P.A. APPROVED BY T.A.
	PB PROJECT NUMBER 15550FFD CLIENT PROJECT NUMBER
	SHEET MS1.21

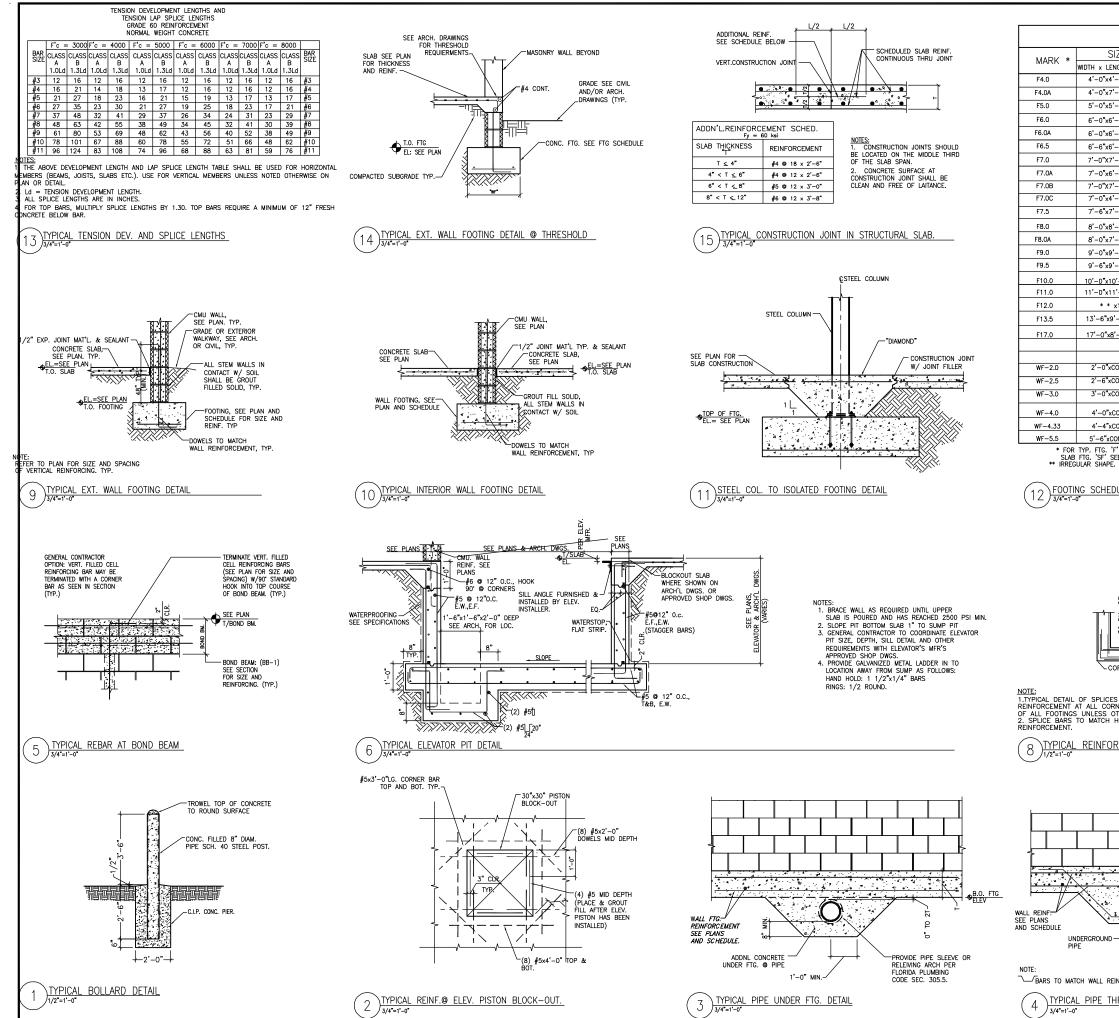






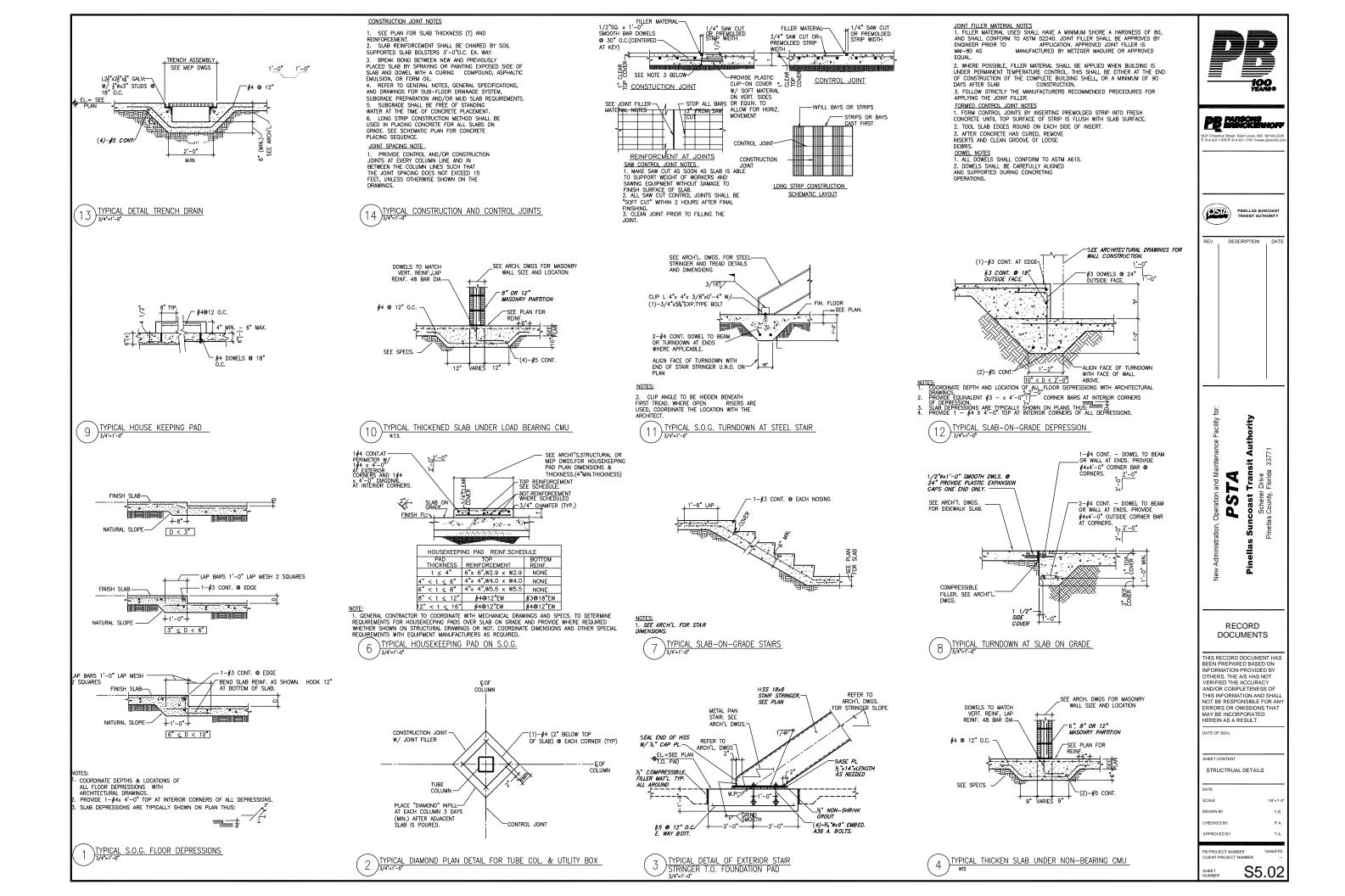


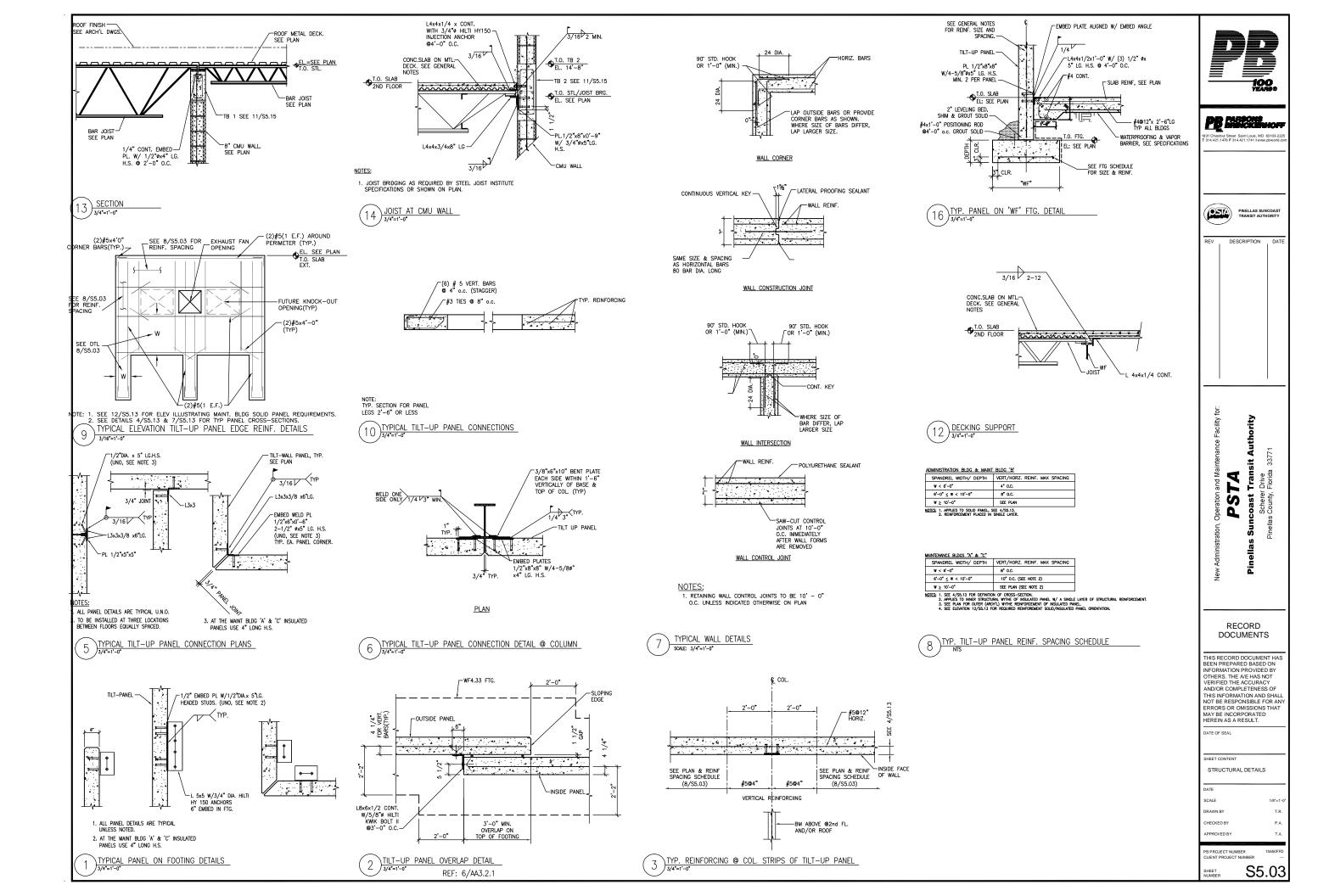


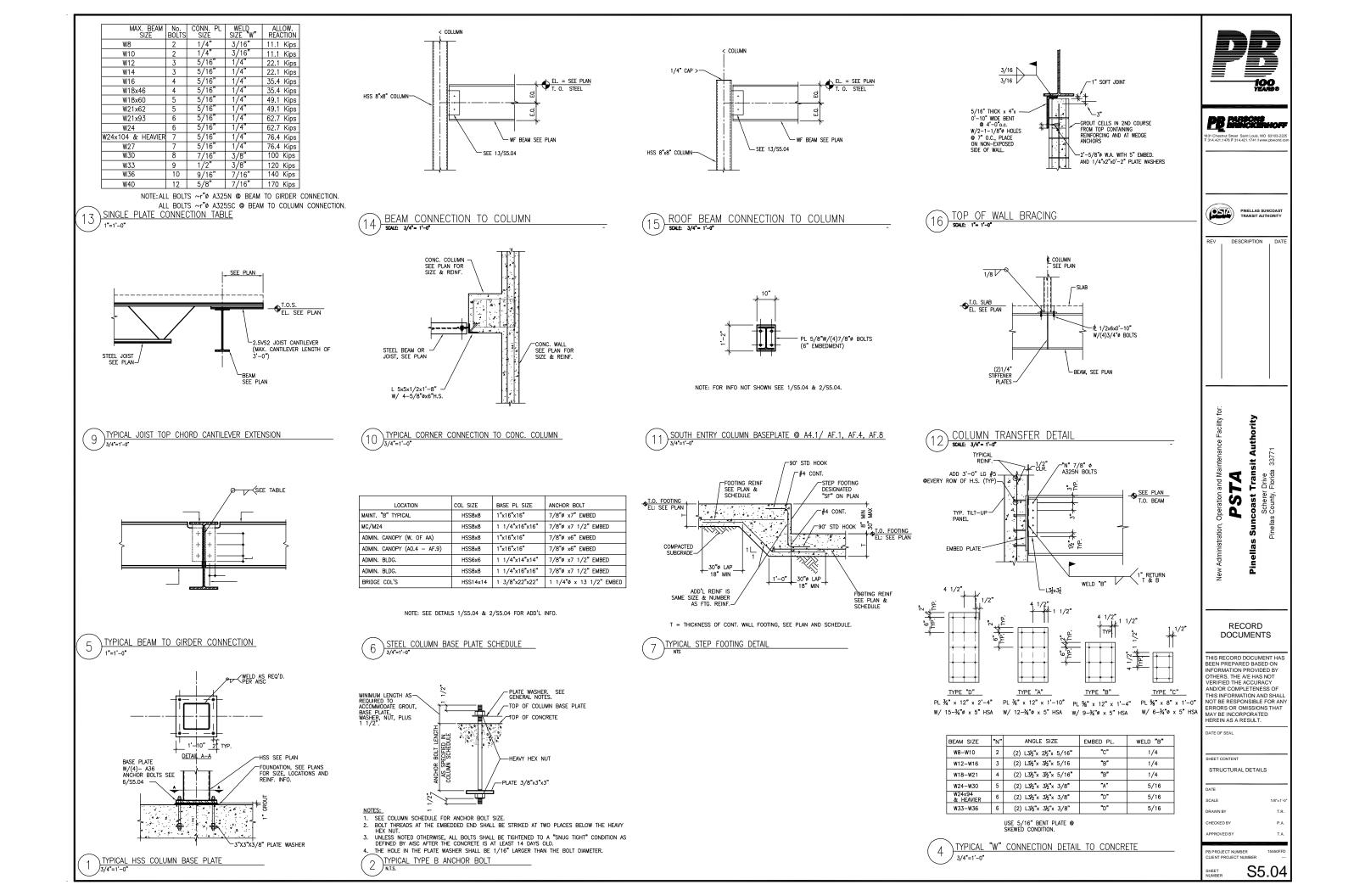


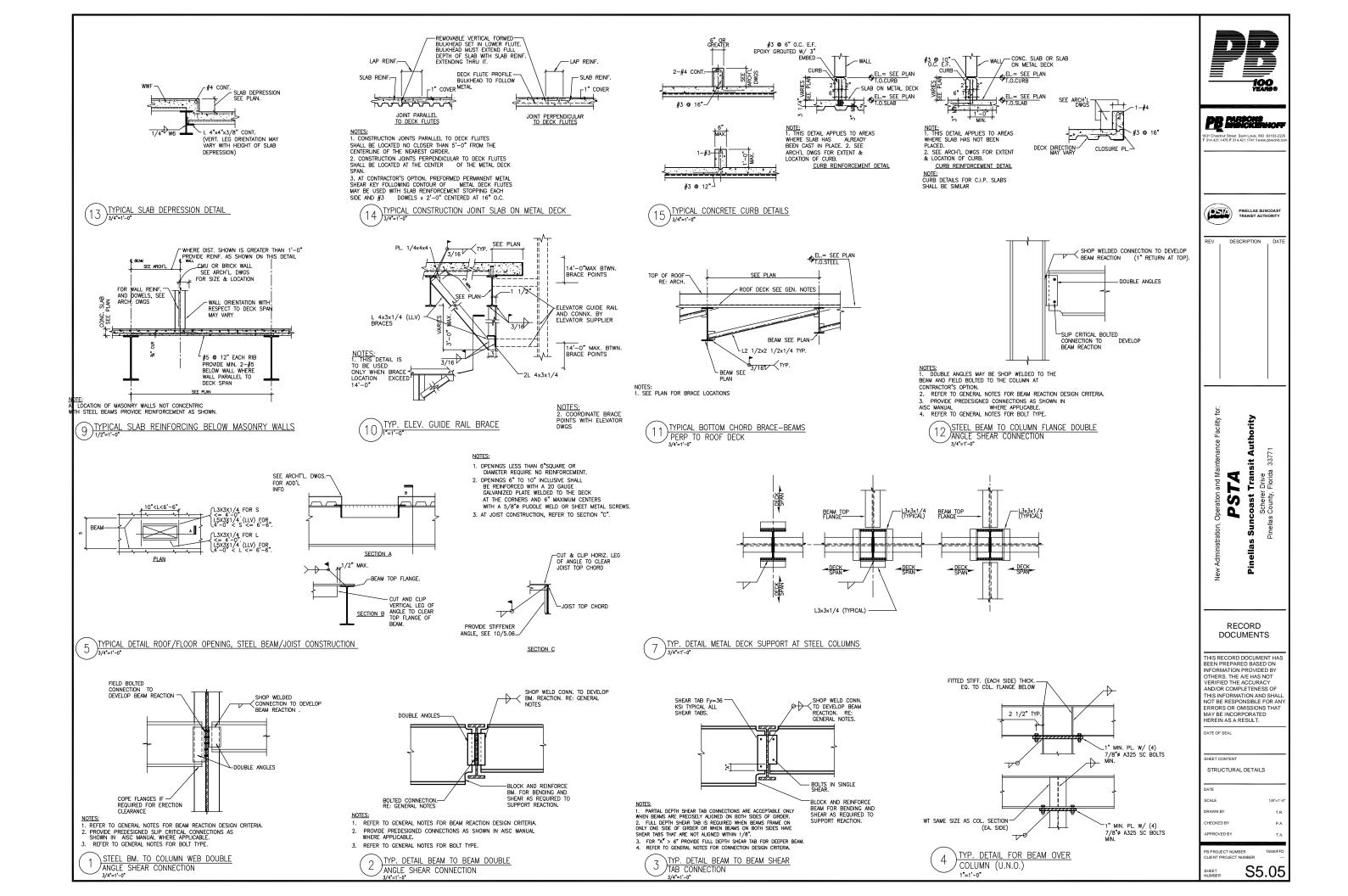
FOOTIN	G SCHEDULE		
SIZE			
	REINFORCEMENT		
LENGTH × DEPTH "x4'-0"x1'-0"	BOTTOM	TOP	
	5#5 EW		100 YEARS®
"x7'-0"x1'-2"	7-#5 L.W. 5-#5 S.W.		TEANSU
"x5'-0"x1'-0"	6#5 EW		
"x6'-0"x1'-0"	5#6 EW		
"x6'-0"x1'-3"	6#6 EW		PE BRICKENOF
"x6'-6"x1'-0"	7#5 EW		1831 Chestnut Street Saint Louis, MO 63103-2225
"X7'-0"X1'-0"	6-#6 E.W.		T 314.421.1476 F 314.421.1741 I www.pbworld.com
"x6'-0"x1'-0"	5-#6 L.W. 4-#6 S.W.		
"X7'-0"X1'-6"	7-#6 E.W.		
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"x7'-6"x1'-6"	8-#6 E.W.	8-#6 E.W.	
"x8'-0"x1'-6"	8-#7 E.W.		
"x7'-0"x1'-4"	7-#6 L.W. 6-#6 S.W.		PINELLAS SUNCOAST TRANSIT AUTHORITY
x9'-0"x1'-4"	6-#6_S.W. 9#6_EW		
"x9'-6"x1'-6"	10#6 EW	10#6 EW	REV DESCRIPTION DATE
"x10'-0"x1'-6"	10#8 EW		
"x11'-0"x1'-8"	10#8 EW		
* x1'-4"	#6@12'-0"o.c. EW	#6@12'-0"o.c. EW	
x9'-0"x1'-6"	8-#7 L.W. 6-#7 S.W.		
		#4@10"	
)"x8'-6"x1'-0"	#4@12" o.c. EW	#4@12" o.c. EW	
D"xCONT.x12"	(2)-#5 CONT		
	(2)-#5 CONT. #3@ 24"o.c. TRANSVERSE (3)-#5 CONT		
5"xCONT.x12"	(3)-#5 CONT. #3@ 24"o.c. TRANSVERSE		
D"xCONT.x12"	40 12"o.c. TRANSVERSE		
0"xCONT.x12"	(5)-#5 CONT.		
4"xCONT.x12"	(5)-#5CONT. #4@ 12"o.c. TRANSVERSE		
"xCONT. X12"	#40 12 o.c. TRANSVERSE (5)-#5CONT. #40 12"o.c. TRANSVERSE (5)-#5 CONT. #40 18"o.c. TRANSVERSE		
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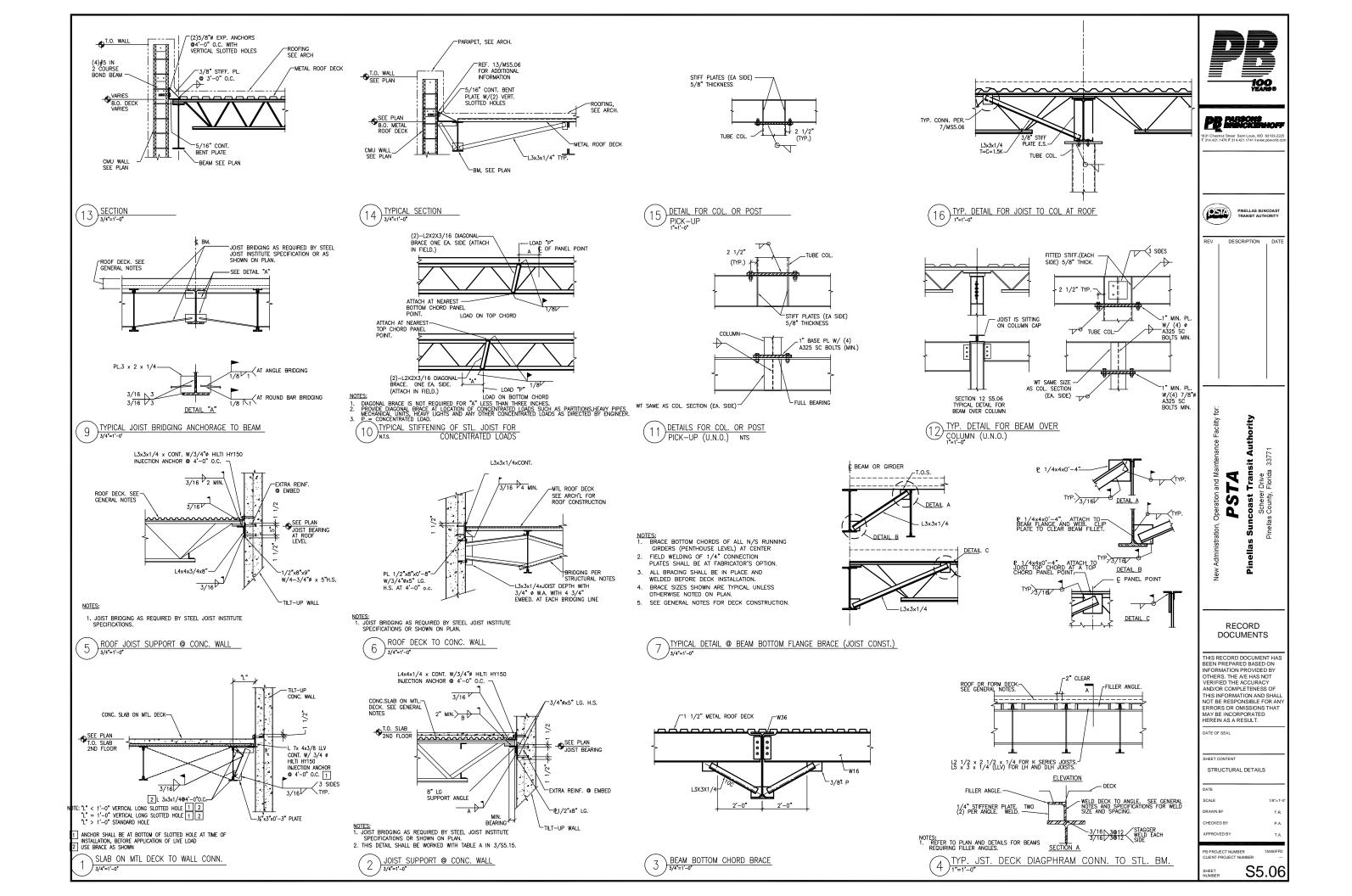
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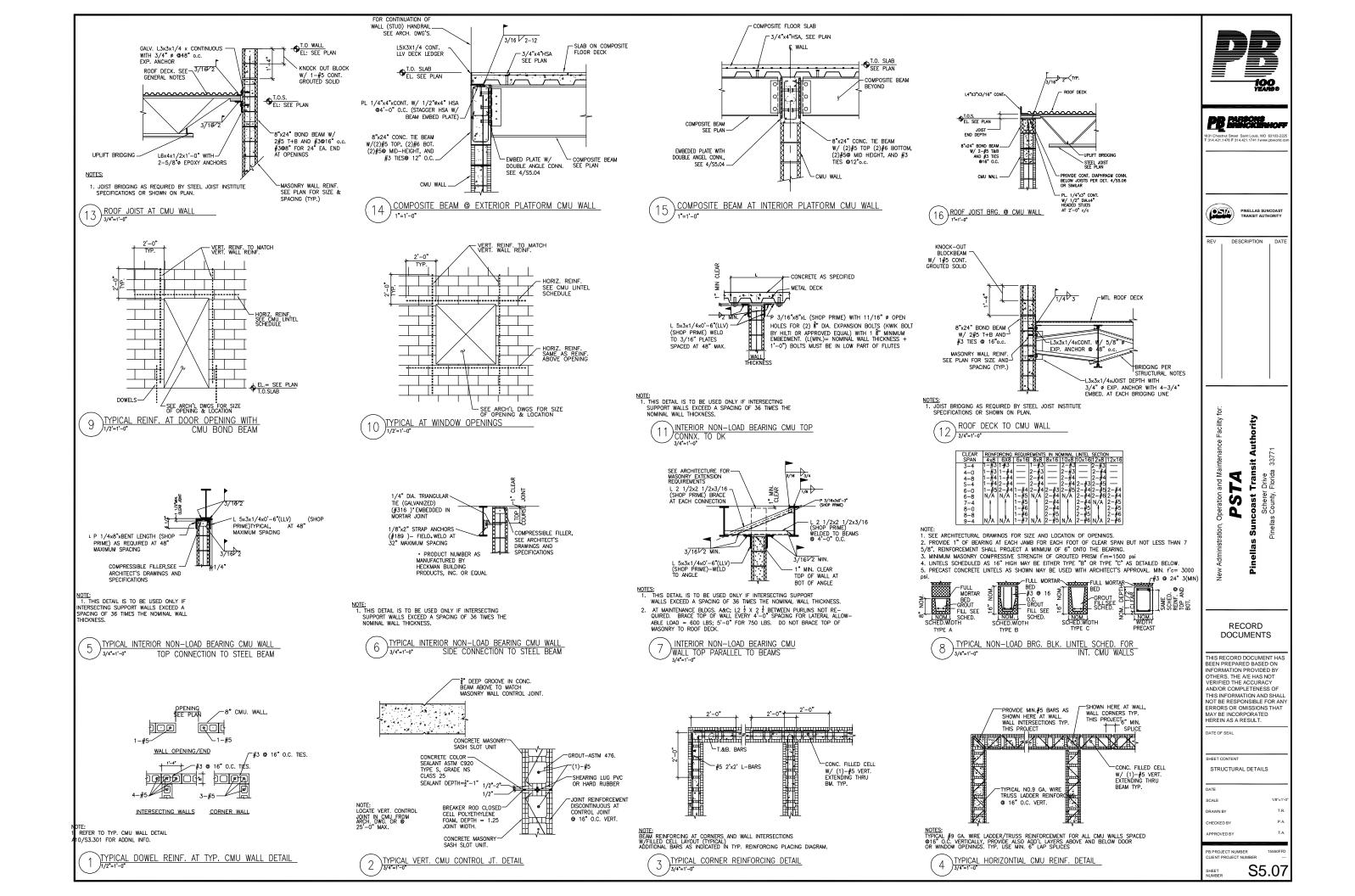


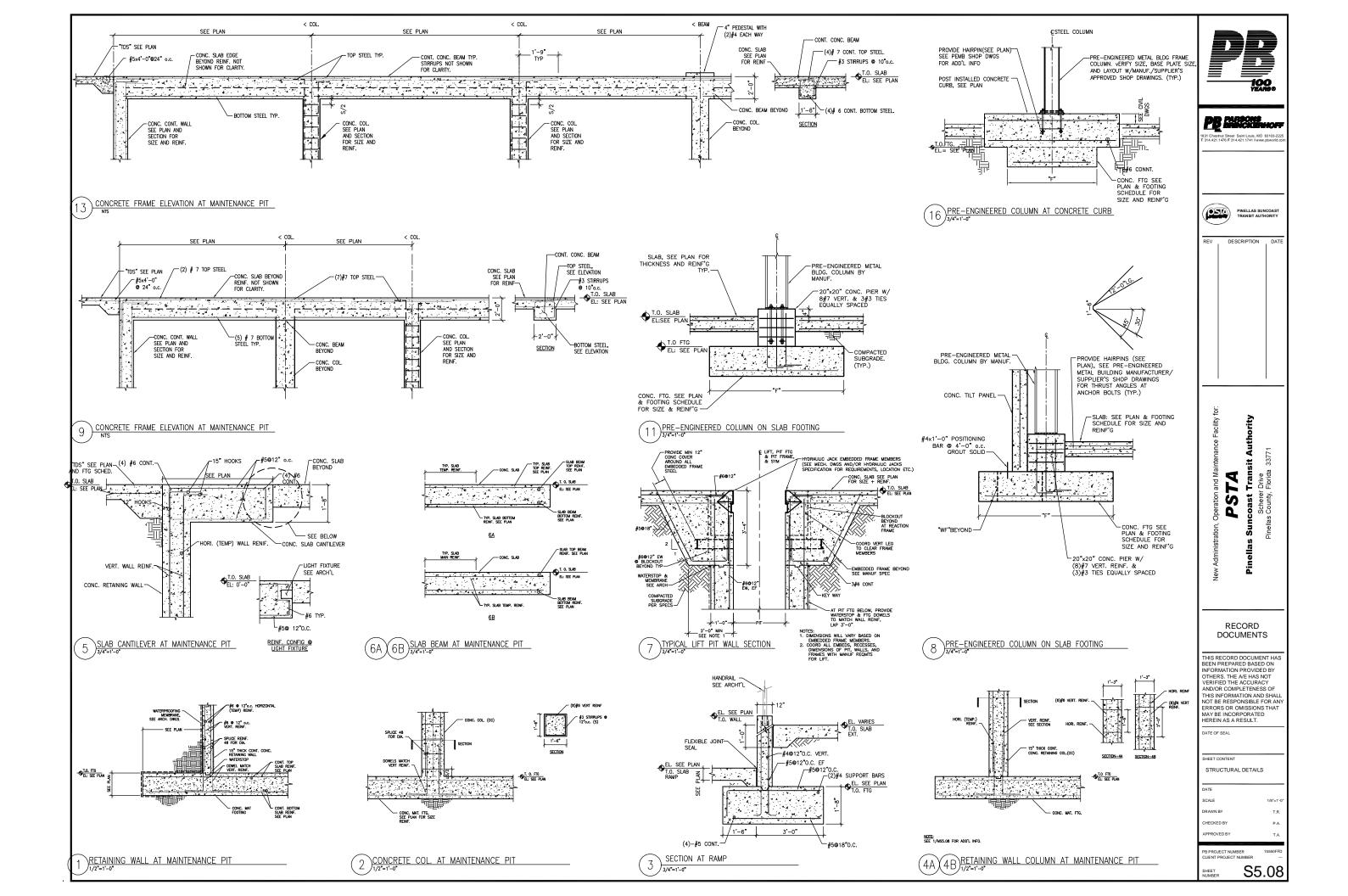


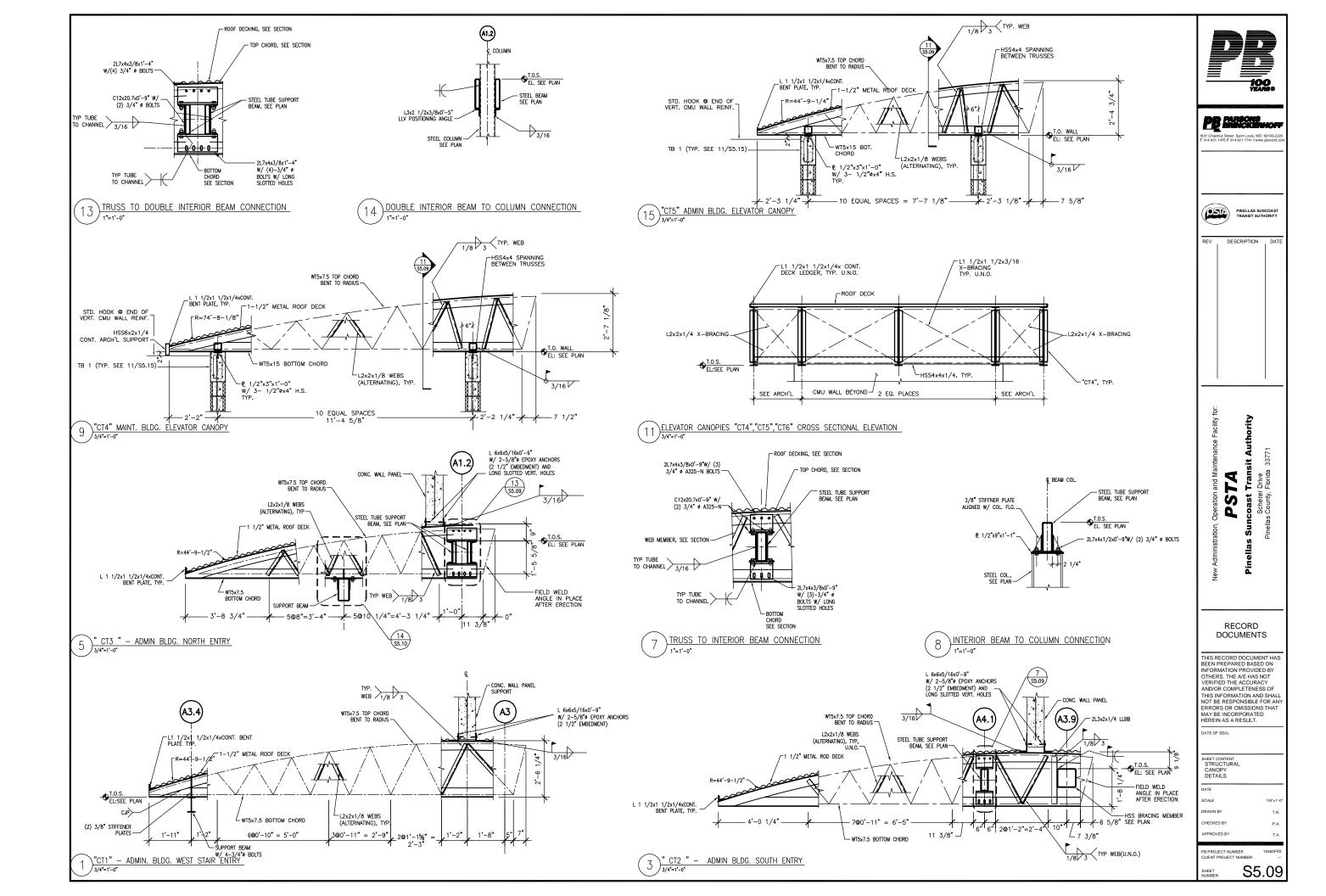


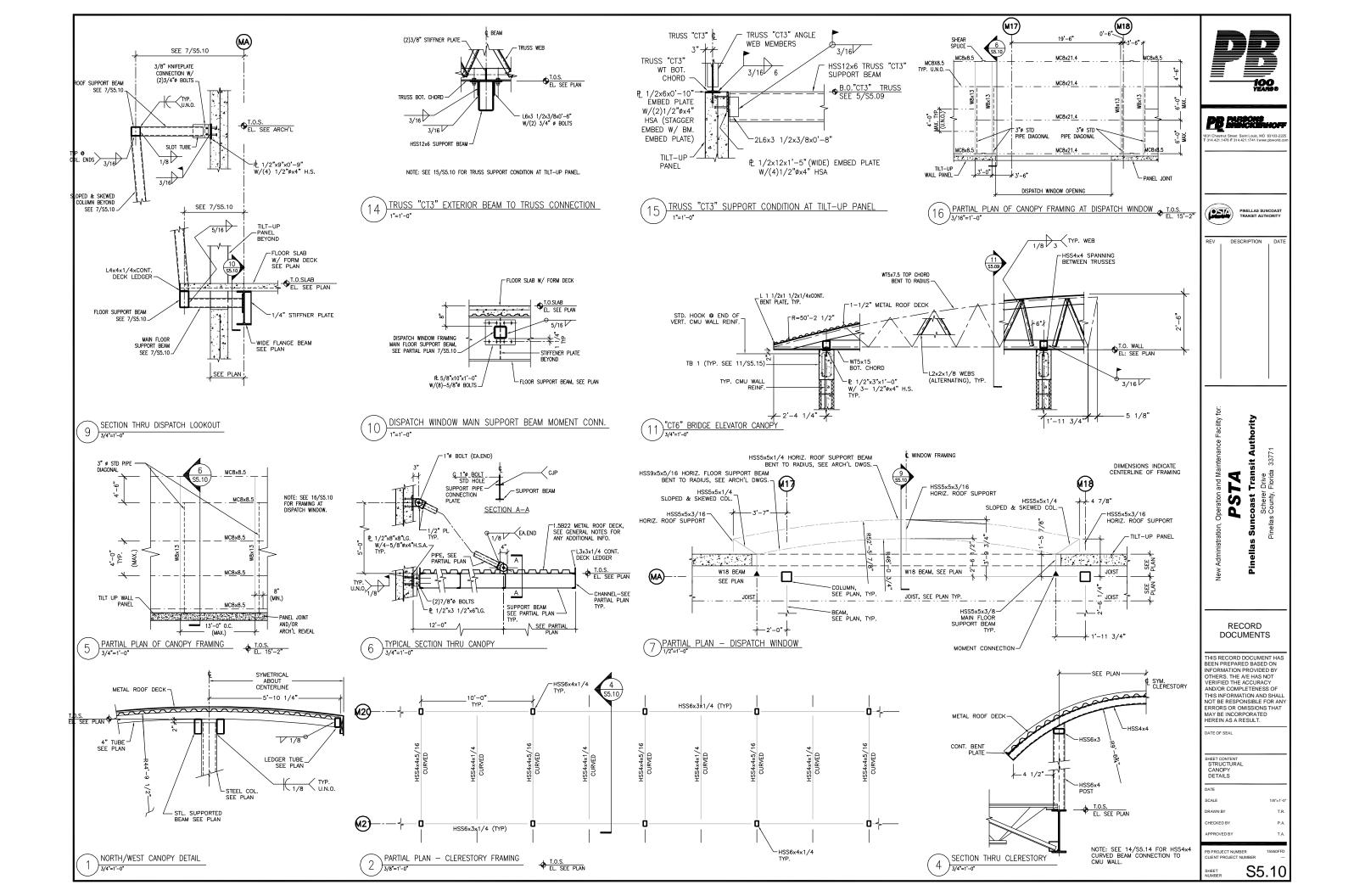


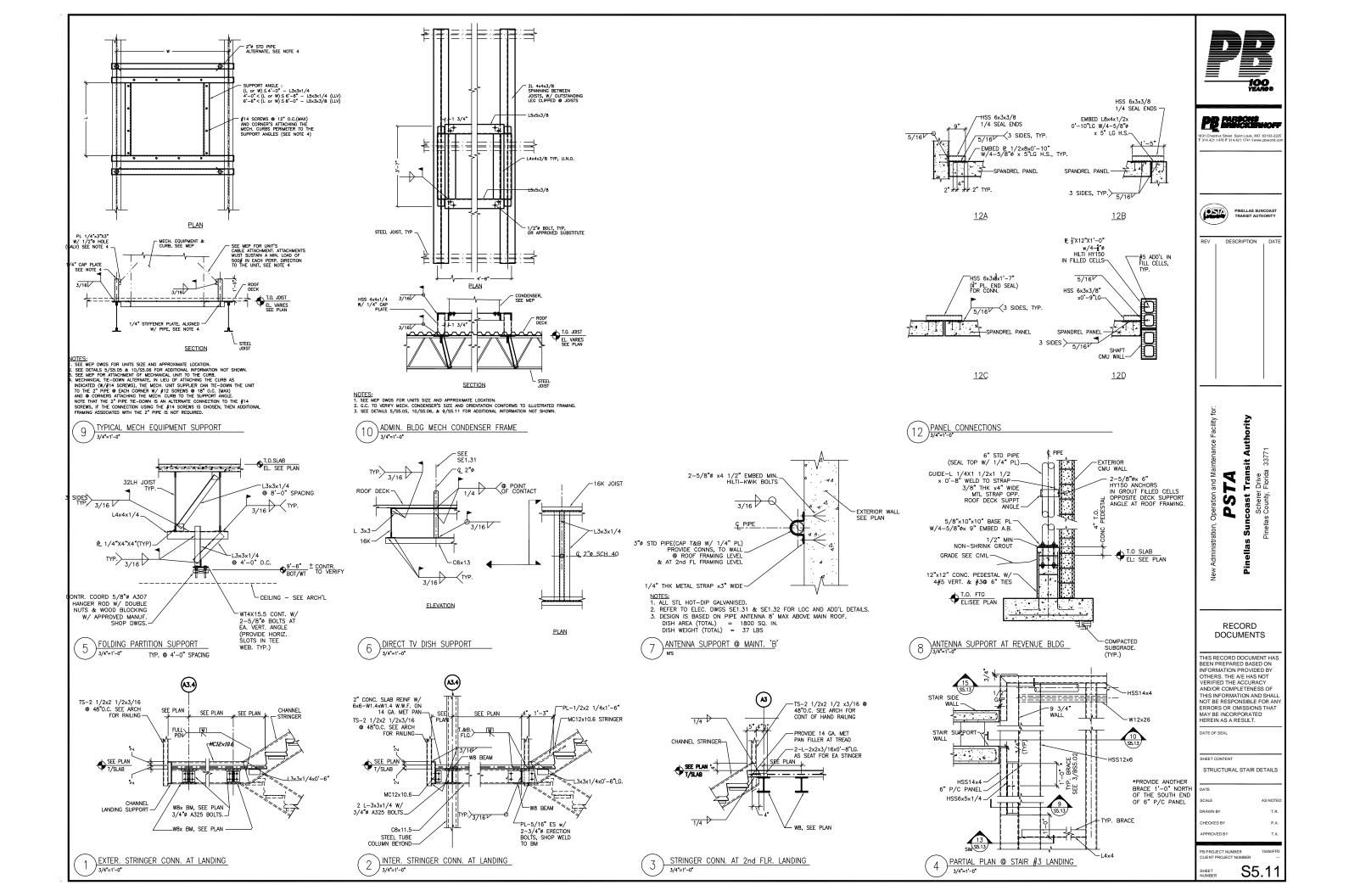


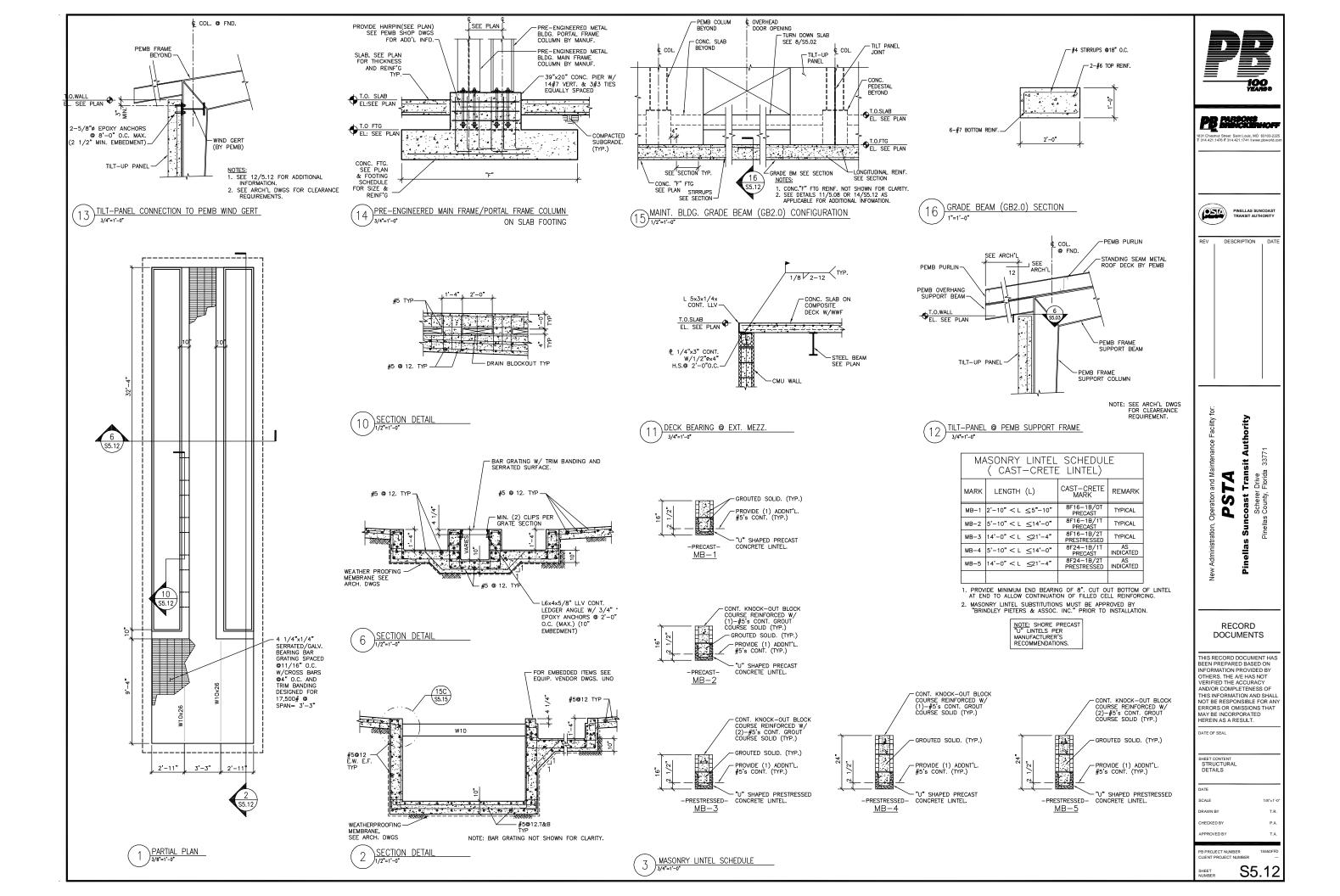


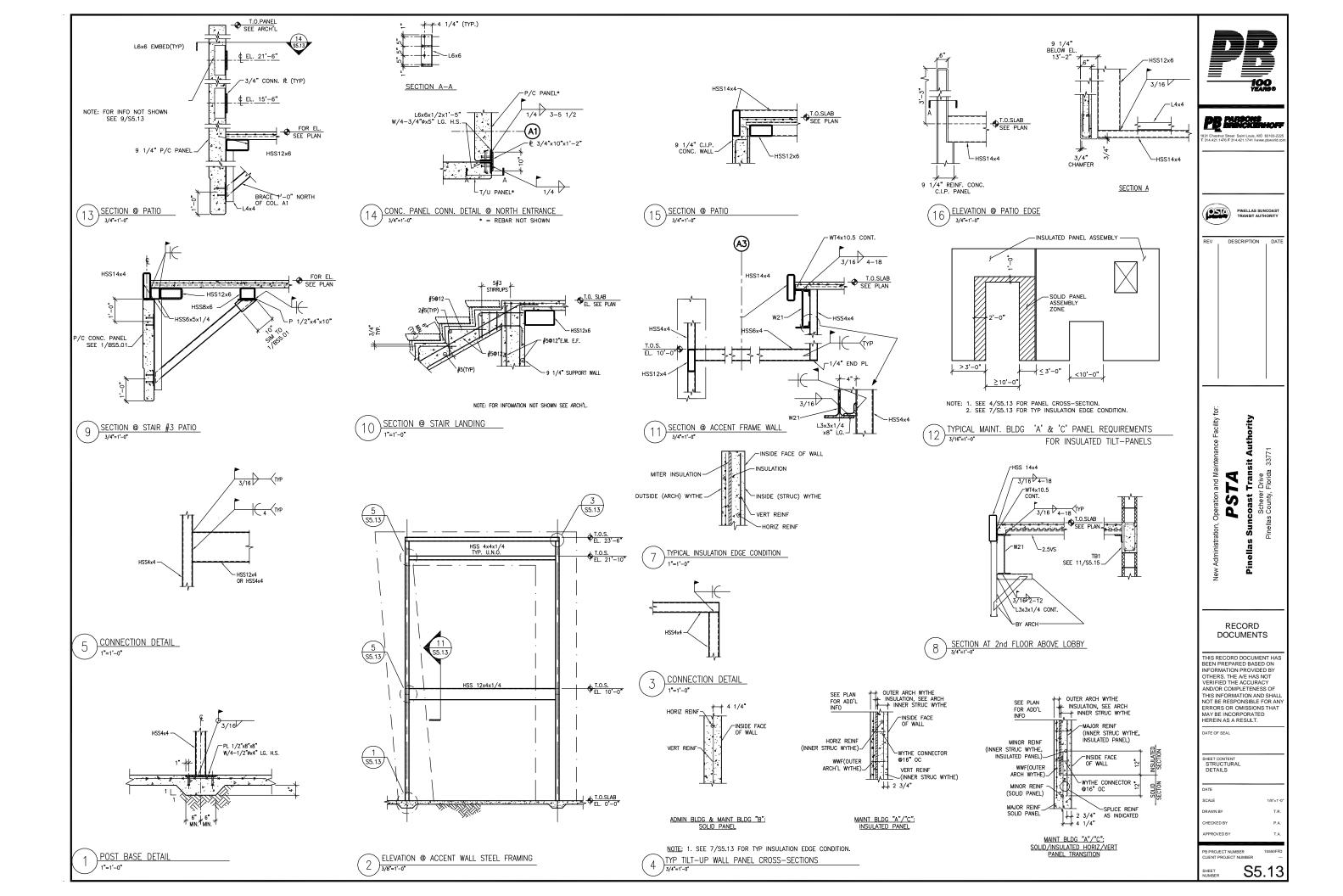


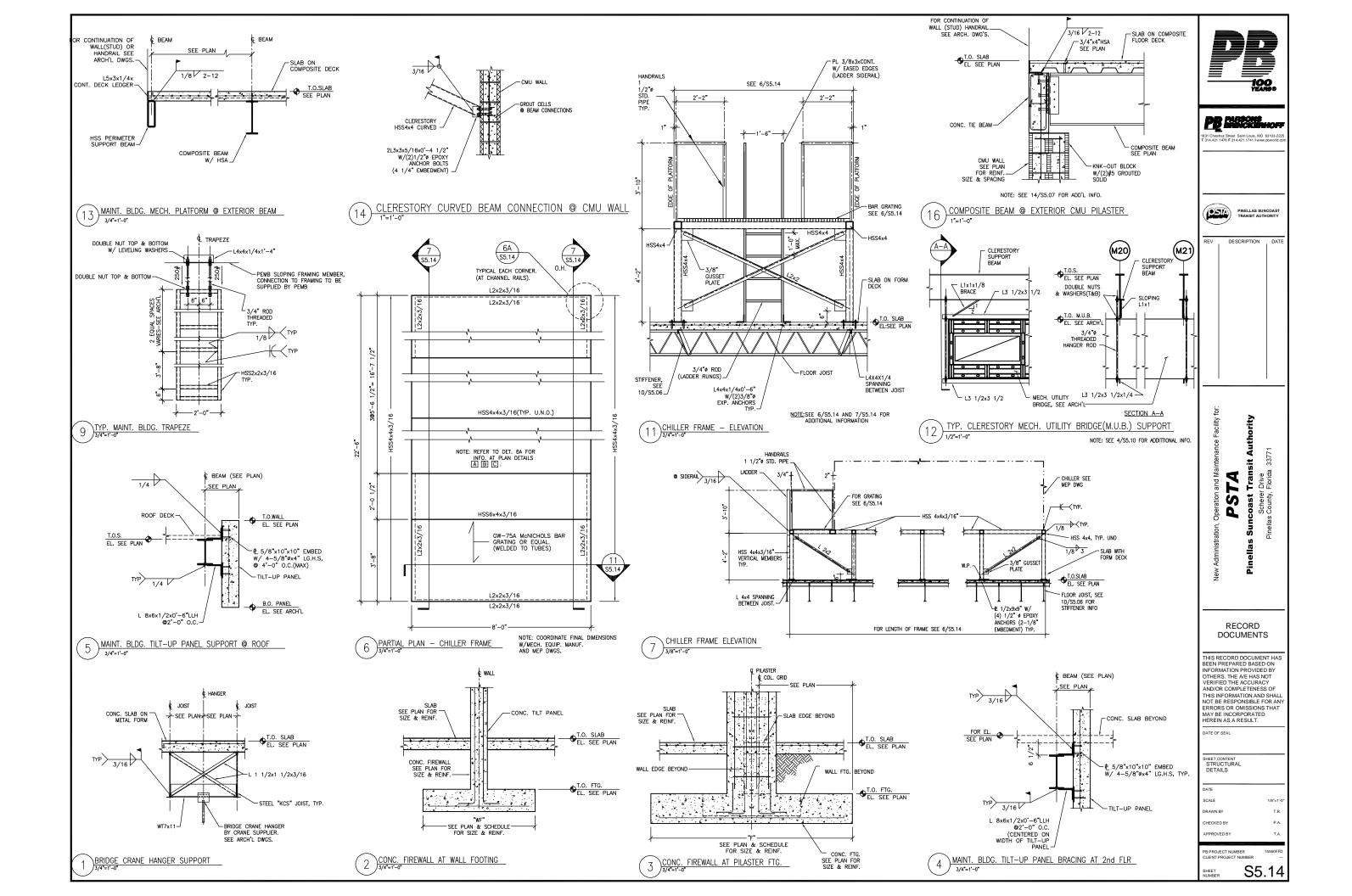


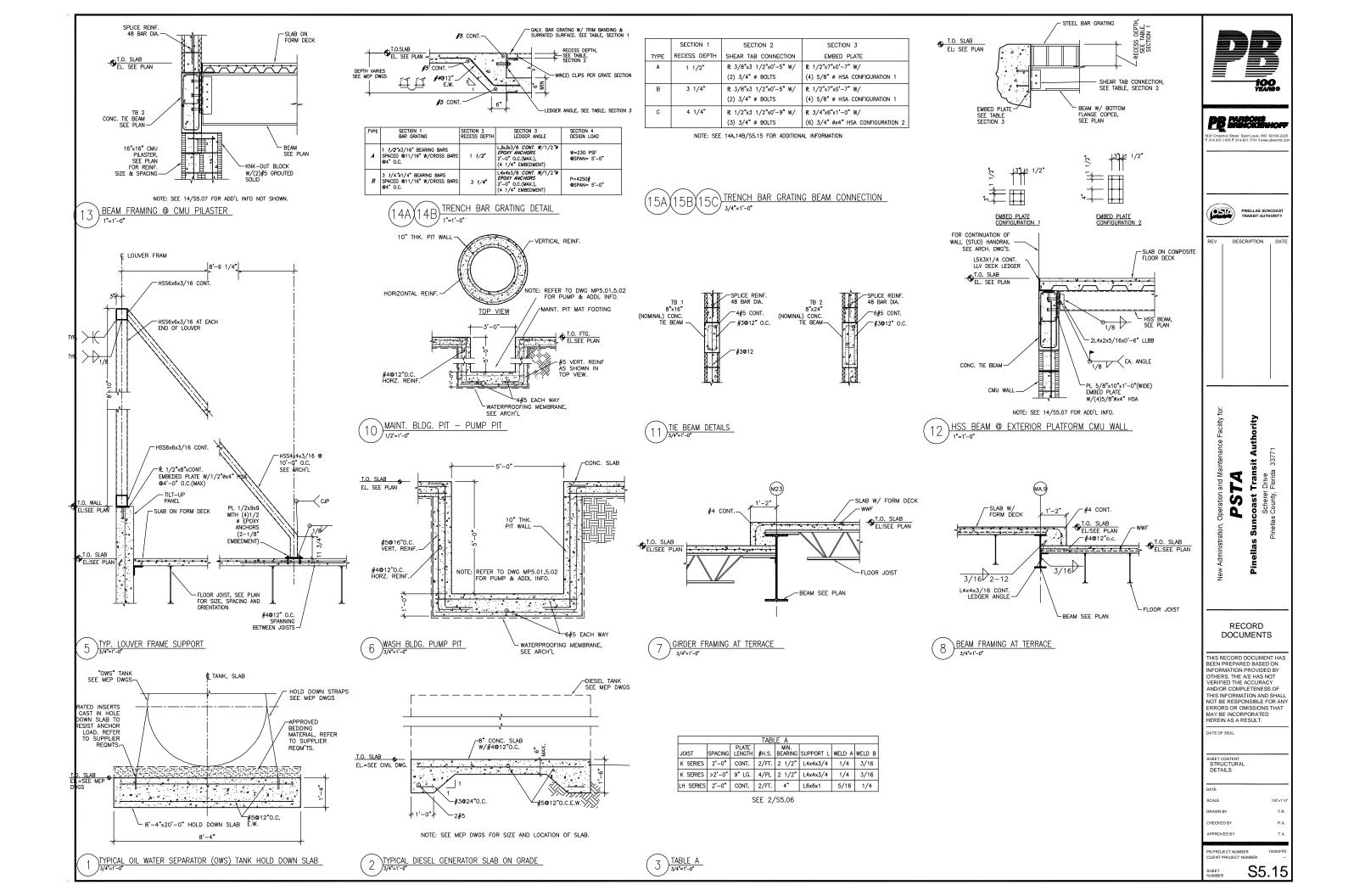




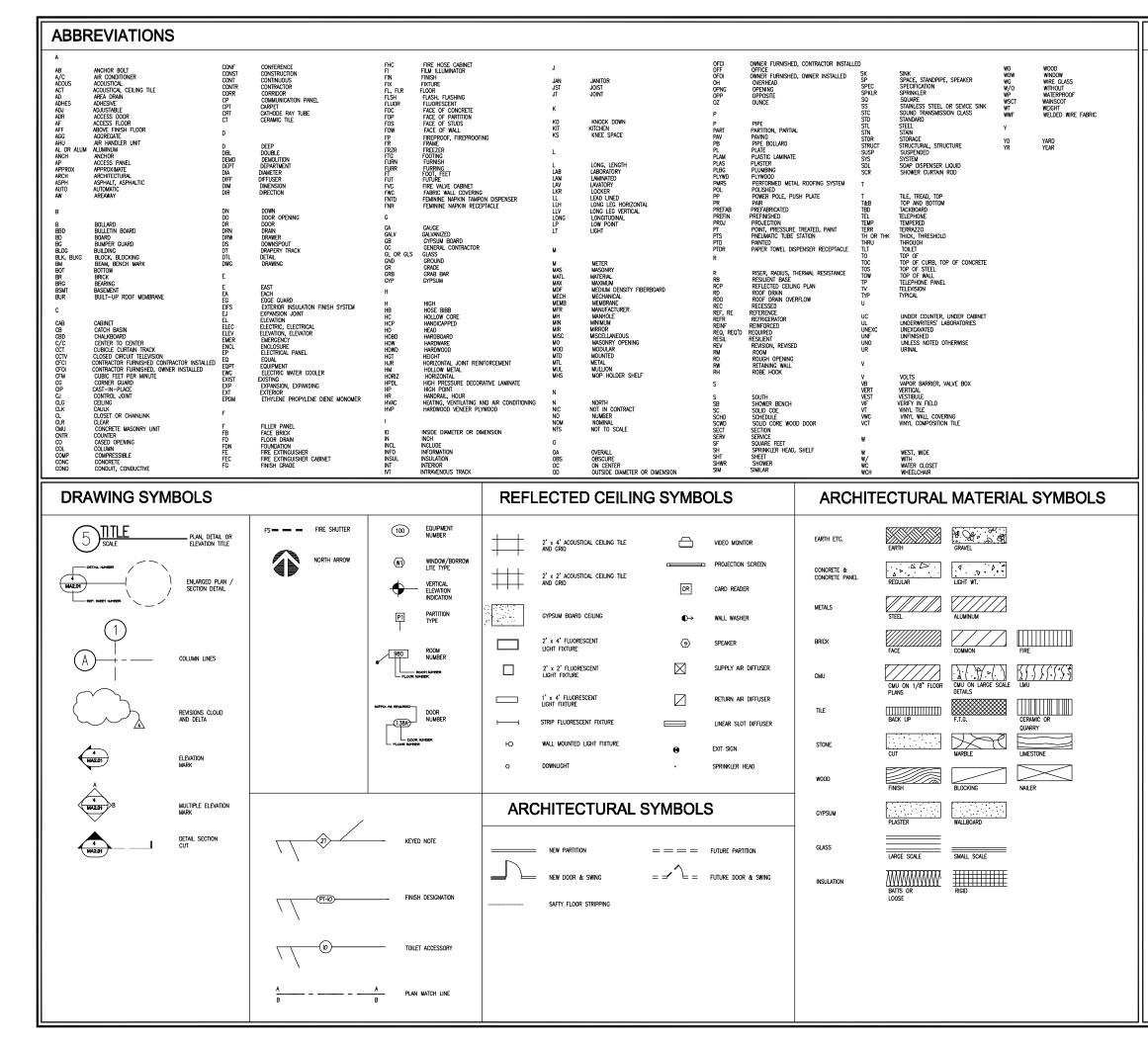








## Maintenance Architectural Drawing MA0.00- MA1.13



#### **GENERAL NOTES** ALL DIMENSIONS UNLESS OTHERWISE NOTED ARE TO FINISHED FACE OF PARTITIONS. 2. INTERIOR PARTITIONS SHALL BE 5/8 INCH GYPSUM WALL BOARD UNLESS OTHERWISE NOTED. = PROVIDE FIRE TREATED 2 INCH X 6 INCH NOMINAL WOOD BLOCKING RUNNING CONTINUOUSLY BETWEEN STUDS FOR ALL WALL HUNG EQUIPMENT OR OTHER TEMS ATTACHED TO GYPSUM WALL BOARD PARTITIONS PB BARCHERHOF PROVIDE DEFLECTION CHANNELS © ALL LOCATIONS WHERE STL. STUDS MEET BOTTOM OF STEEL JOIST & BEAM, UNLESS DETAILED OTHERWISE. et Saint Louis, MO 631 PROVIDE FIRE SAFING INSULATION AT ALL PIPE, DUCT, ELECTRICAL, ETC. PENETRATIOI THROUGH CONCRETE FLOOR SLABS AND AT FIRE AND SMOKE WALLS UNLESS INDICATED OTHERWISE, INTERIOR DOORS LOCATED IN GYPSUM BOARD PARTITIONS TO BE 6 INCHES FROM HINGE-SIDE JAMB TO ADJACENT WALLS. (AEQ) PINELLAS SUNCOAST TRANSIT AUTHORITY PROVIDE FIRE TREATED WOOD BLOCKING & NAILERS THROUGHOUT THE BUILDING AS REQ. TO MEET NON-COMBUSTABLE CONSTRUCTION PER FLORIDA CODE DESCRIPTION WHERE ADJOINING ROOMS HAVE DIFFERENT TYPES OF FLOORING OR SIMILAR FLOORING OF DIFFERENT COLORS, MAKE CHANGE UNDER CENTER LINE OF DOOR. ROOM FINISHES SHOWN IN THE ROOM FINISH SCHEDULE SHALL APPLY TO ALL ALCOVES, FREESTANDING COLUMNS, OR OTHER APPURTED SPACES ADJOINING THE SCHEDLED ROOM UNLESS OTHERWISE NOTED. 10. ALL EXPANSION JOINTS IN THE FINISHED AREA EXPOSED TO VIEW SHALL BE COVERED WITH EXPANSION JOINT COVERS APPROPRIATE FOR THE FINISHED SURFACE, WHETHER FLOOR, CEILING, OR WALL, OR JUNCTIONS THEREOF 11. INSTALL DOUBLE STUDS AT DOOR FRAME JAMBS. INSTALL STUD TRACKS AT EACH STORE OWNES. INSTALL STUD TRACKS AT EACH SIDE OF OPENING, AT FRAME HEAD HEIGHT AND BETWEEN STUDS AND ADJACENT STUDS. 12. REFER TO DRAWINGS FOR INDICATION OF PARTITIONS EXTENDING STUD FRAMING THROUGH THE CEILING TO THE STRUCTURE ABOVE. MAINTAIN CLEARANCE UNDER STRUCTURAL BUILDING MEMBERS TO AVOID DEFLECTION TRANSFER TO STUDS. PROVID PROVIDE EXTENDED LEG CEILING RUNNERS. 13. METAL STUD SPACING @ 16" O.C. UNLESS thority NOTED OTHERWISE ON DRAWINGS. 14. PROVIDE AND INSTALL CORNER BEADS, EDGE TRIM, JOINT MATERIALS AND FASTENERS TO INSURE PROPER INSTALLATION OF GYPSUM BOARD PLACE CORNER BEADS TE XETERNAL CORNERS. PLACE EDGE TRIM WHERE GYPSUM BOARD ABUTS Aut DISSIMILAR MATERIALS. 쁥 15. PROVIDE ACOUSTIC INSULATION IN PARTITIONS TA TIGHT WITHIN SPACES, AROUND CUT OPENINGS, BEHIND AND AROUND ELECTRICAL AND MECHANICAL ITEMS WITHIN OR BEHIND PARTITIONS, AND TIGHT TO ITEMS PASSING THROUGH Ś INSTALLATION OF SUSPENDED CEILING SYSTEM SHALL MEET LOCAL CODE REQUIREMENTS FOR SEISMIC BRACING Su 17. INSTALLATION OF ACOUSTIC INSULATION TO BE FRICTION FIT, GLASS FIBER TYPE, UNFACED. THICKNESS AS RECOMMENDED BY MANUFACTURER FOR THICKNESS OF WALL (2 INCH MINIMUM). Pinel 18. TAPE, FILL AND SAND EXPOSED JOINTS, EDGES AND CORNERS OF GYPSUM BOARD TO PRODUCE A SMOOTH SURFACE READY TO RECEIVE FINISHES 19. IF THERE ARE ANY DISCREPANCY WITH THE INFORMATION ON THE DRAWINGS OR WITH THE SPECIFICATIONS WANUAL, CONTACT THE ARCHITECT OR ENGINEER, DO NOT SCALE DRAWINGS. 20. PROVIDE LATERAL BRACING OF INTERIOR PARTITIONS THAT DO NOT EXTEND TO DECK ABOVE AS REQUIRED TO MEET LOCAL CODE REQUIREMENTS FOR SEISMIC BRACING RECORD 21. DETAIL SHOWINGS STUD FRAMING-NOT CODE OR STRUCT. RELATED FOR ILLUSTRATION PURPOSES ONLY. CONSTRUCTION ON SITE MAY VARY. DOCUMENTS ARCHITECTURAL ELEVATION: 0'-0" EQUALS: 15'-0" CIVIL FOR THE ADMIN. BUILDING, AND 11'-0" FOR THE MAINTENANCE BUILDING THIS RECORD DOCUMENT HAS BEEN PREPARED BASED ON INFORMATION PROVIDED BY OTHERS. THE A/E HAS NOT VERIFIED THE ACCURACY IN ALL DETAILS THAT ILLUSTRATE WOOD BLOCKING, NAILING OR SHEATHING SHALL BE FIRE RETARDANT TREATED WOOD. AND/OR COMPLETENESS OF AND/OR COMPLETENESS OF THIS INFORMATION AND SHAL NOT BE RESPONSIBLE FOR AN ERRORS OR OMISSIONS THAT MAY BE INCORPORATED HEREIN AS A RESULT. 24. PROVIDE PRECONSTRUCTION SOIL TREATMENT TERMICIDE APPLICATION TO INSURE THAT A CONTINUOUS HORIZONTAL AND VERTICAL TERMITICIDAL BARRIER OR TREATED ZONE I ESTABLISHED AROUND AND UNDER ALL BUILDING DATE OF SEAL CONSTRUCTION. APPLICATION OF TREATMENT SHALL INCLUDE BUT NOT BE LIMITED TO: UNDERGROUND SUPPORTED SLABS-ON-GRADE, FOOTINGS, ADJACENT SOIL ALONG BOTH SHEET CONTENT INSIDE AND OUTSIDE PERIMETER OF FOUNDATION WALLS. BOTH SIDES OF INTERIOR PARTITION WALLS, AROUND PLUMBING PIPES AND ELECTRICAL CONDUIT PENETRATING TH MAINTENANCE BUILDING GENERAL NOTES ABBREV. AND SYMBOLS SLAB, AND AROUND INTERIOR CLUMN FOOTERS AND PIERS. OUTSIDE PERIMETEROE FOUNDATIONS SHALL EXTEND FROM ATE GRADE TO BOTTOM OF FOOTING. EXTEND TREATMENT UNDER SCALE ALL EXTERIOR CONCRETE OR GRADE TO A MINIMUM OF 1'-0" OUTSIDE OF THE PRIMARY STRUCTURE SIDEWALLS. SEE DRAWN BY SPECIFICATIONS FOR FURTHER INFORMATION. COMPLETE CHECKED BY COMPLIANCE WITH FBC CHAPTER 18, SECTION 1816 MUST BE PPROVED B B PROJECT NUMBER MA0.00 SHEET

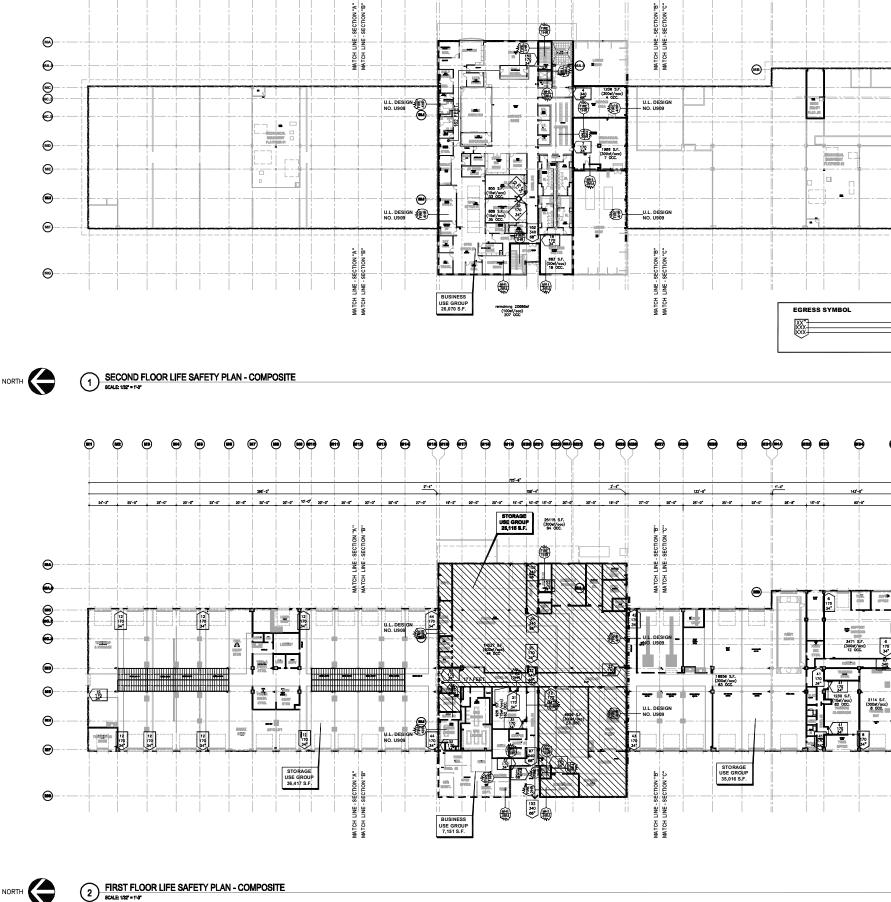
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DA

Drive Florida

GM

MG



FIRE RESIS 30 OR MOR BUSINESS, MIN. FIRE



M34

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MA

MB

<u>eeee</u>e

MD

ME

-

MF

MG

- ACTUAL OCCUPANTS TO EGRESS - Allowable occupants to egress - Egress width

.

20'-0" 19'-4"

2220 S.F. (300st/occ) B OCC.

1890 S.F. 785 S.F. (300ef/occ (300ef/occ) 7 OCC. 3 OCC.

7 170 34°

Tint .

8 170 34

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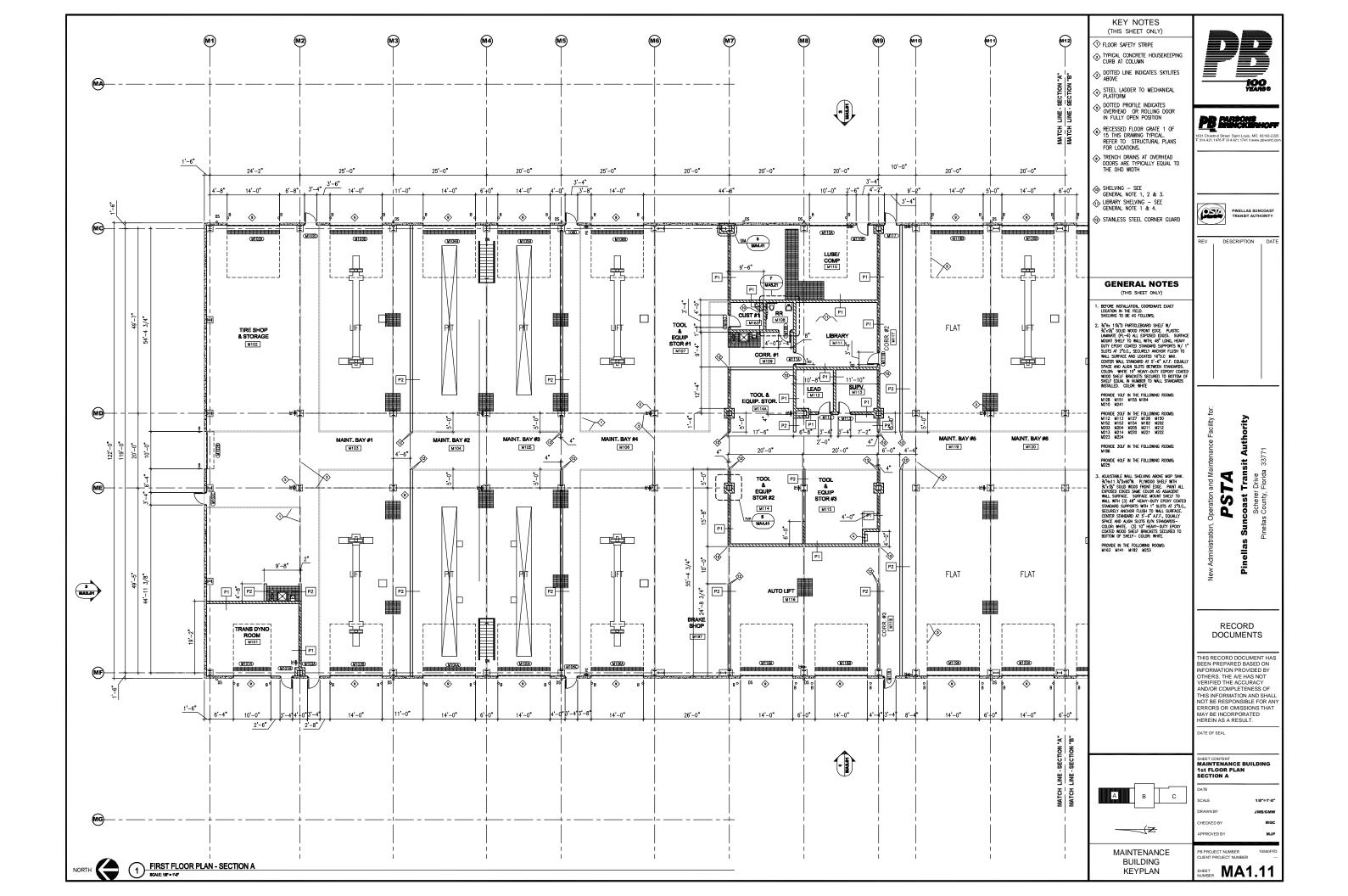
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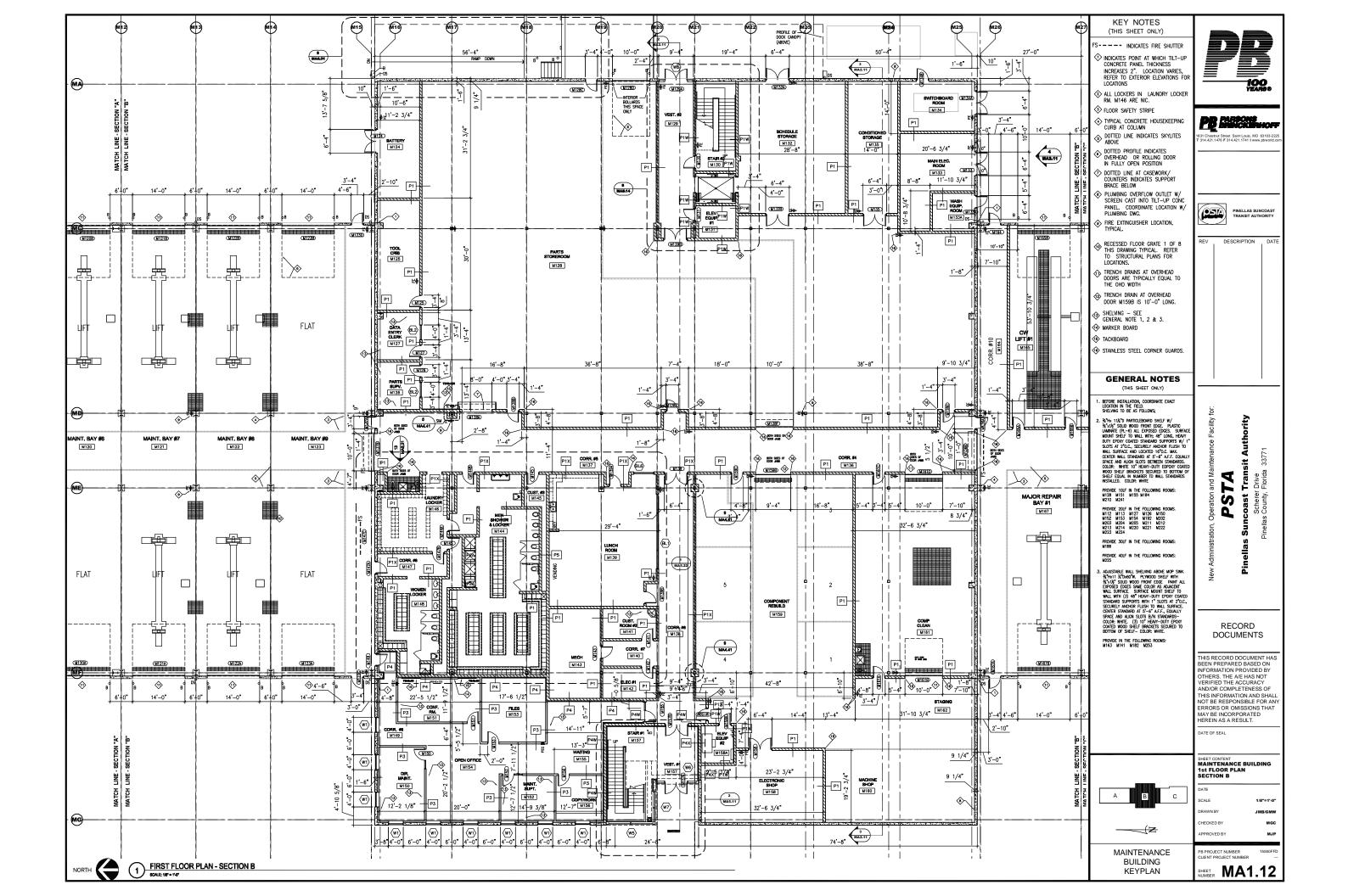
M1 M2

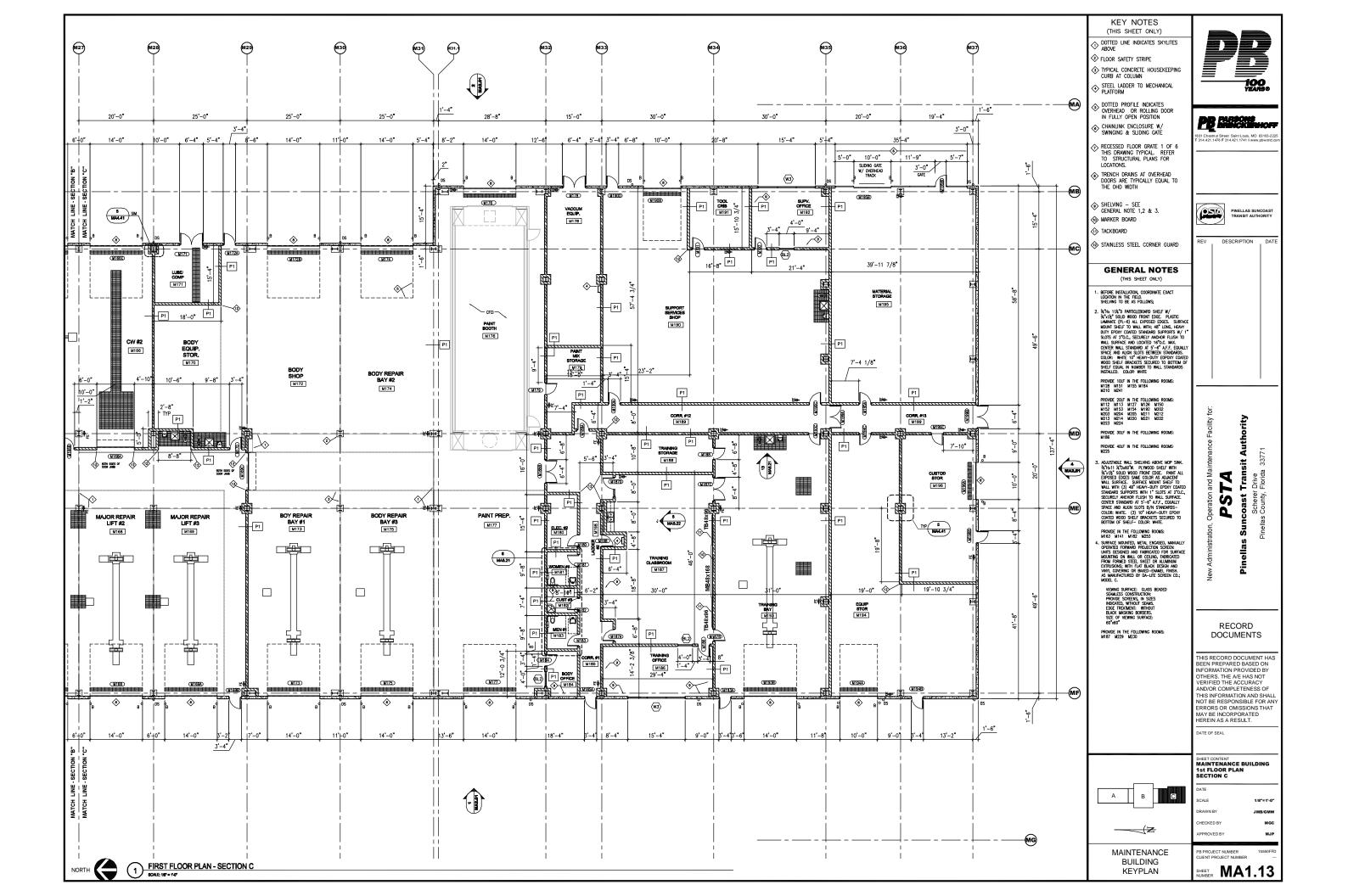
B	UILD	ING CODE	NFOF	RMAT	ION			
REQUI	REMENTS FO	ZED OPENINGS AND SKYLIG R WIND-BORNE DEBRIS IMP	ACT RESISTAN	CE STANDARD	OS FOR LARGE			
1996	OR MIAMI-D. DDITIONAL IN	TING THE REQUIREMENTS O ADE PA 201, 202 AND 203 JEORMATION	. REFEREN	CE STRUCTUR	AND ASTM E			
2. MODIFI	ED BITUMEN	MEMBRANE ROOF SYSTEM					100	
ACCOR	RDANCE WITH	NCE OF NON-BALLASTED LI FM 4450, FM 4470, UL 5	80, OR UL 1		IED IN		YEARSO	
		Y II (ASCE 7-98 TABLE 1		Vele	REFERENCE			
DESCRIPTION USE GROUP PRIMARY			ANALYSIS (S-2), REPAIR GARAGE (B), BUSINESS		SECTION 411.5	PB PAREONS		
CONSTRUCT	CONSTRUCTION TYPE			SS ECTED)	SECTION 305 SECTION 606	1831 Chesthut Street Saint Louis, MO 63103-2225		
BUSINESS, (	(B)		SPRINKLER	ACTUAL		T 314.421.1476 F 314.42	1.1741 I www.pbworld.com	
	BUILDING H		55 FEET 5 STORY	35 FEET 2 STORY	TABLE 500			
	AREA MULT		34,000 SF	SEE PLAN				
STORAGE, (S	5-2) BUILDING H	IEIGHT	ALLOWABLE 55 FEET	ACTUAL 31 FEET	TABLE 500			
	MAXIMUM S		4 STORY 48,000 SF	1 STORY SEE PLAN				
		RUCTION FIRE RESIST		•	JRS)		ELLAS SUNCOAST	
	SEPARATION			2	TABLE 704.1		ANGITAUTHORIT	
-	FIRE WALLS	AGE - SPECIAL OCCUPANCY	4		TABLE 600	REV DESC	RIPTION DATE	
	EARING WALLS	S DTHER COLUMNS	NONCOMBUSTIBLE			DEGO		
OR MORE T	HAN ONE FL							
COLUMNS S	UPPORTING F	ROOF ONLY	NONCOMBUSTIBLE NONCOMBUSTIBLE					
BEAMS, GIR SUPPORTING	DERS, TRUSS G COLUMNS (	ES, ARCHES DR MORE THAN ONE FLOOR	NONCOMBUSTIBLE					
	ONE FLOOR		1	IBUSTIBLE IBUSTIBLE				
FLOORS &	FLOOR/CEILIN	IG ASSEMBLIES	NONCON	IBUSTIBLE				
HORIZONTAL		FROM PROPERTY LINE for		BUSTIBLE				
EXTERIOR B	O FEET T	S, EXTERIOR NONBEARING WAI D 3 FEET	LLS, & GABLE	ENDS OF ROO	OF TABLE 600			
		EET TO 10 FEET	0% WALL	OPENINGS				
			10% WALL	OPENINGS				
	OVER 10 FEET TO 20 FEET		NONCOMBUSTIBLE 20% WALL OPENINGS					
	OVER 20 FEET TO 30 FEET			OMBUSTIBLE				
	OVER 30	FEET	NONCO	MBUSTIBLE				
			NO LIMIT WALL OPENINGS			ty for	≥	
	FIRE RESISTANCE RATING OF EXIT ACCESS CORRIDORS 30 OR MORE OCCUPANTS			0	TABLE 704.2.4	acilit	orit	
BUSINESS, S		PARTITIONS & OPENINGS	PARTITION	OPENING	TABLE 705.1.2	Ce F	th .	
	SHAFT ENCLOSURES (including stairs, elevators, exits)		1	1	TABLE 705.1.2	enar	<b>t A</b> 1 3771	
	LESS THAN 4 STORIES			_	beration and Maintenance Facility DSTA oast Transit Authority Scherer Drive County, Florida 33771			
	OCCUPANCY SEPARATION FIRE WALLS		2 4	1-1/2 3			<b>ra</b> Drive floric	
							Ity. F	
	MACHINERY RODM			3/4	SECTION 704.1.3.3			
_		CE, DEAD-END, EGRES	S WIDTHS &	& OCCUPA	NT LOADS	ð –	Sunc	
BUSINESS -	(B) LENGTH OF EXIT ACCESS TRAVEL		300 FEET		TABLE 1004	ratio	Pine	
	MAX.IMUM DEAD END CORRIDOR EGRESS WIDTH PER PERSON CORRIDOR EGRESS WIDTH PER PERSON STAIRS MINIMUM CORRIDOR WIDTH MINIMUM CLEAR OPENING OF EXIT DODRS		50 FEET 0.2 INCHES 0.3 INCHES 44 INCHES 32 INCHES			vew Administration, Or Pinellas Sunc		
						P. New	Ä	
STORAGE -	MINIMUM STAIR WIDTH MAGE - (S-2)		44 NCHES		TABLE 1004	<b>1</b>		
	LENGTH OF EXIT ACCESS TRAVEL MAX.IMUM DEAD END CORRIDOR		400 FEET					
	INC.INVIDUE ENCOURDANT ECRESS WIDTH PER PERSON CORRIDOR ECRESS WIDTH PER PERSON STARS MINIMUM CORRIDOR WIDTH MINIMUM CLEAR OPENING OF EXIT DODRS MINIMUM STAR WIDTH INVIDUE STAR WIDTH BUSINESS – (8)		0.2 INCHE		1	RECORD DOCUMENTS		
			0.3 INCHES	1				
			S 32 INCHES 44 NCHES					
MINIMUM OCC			100 GROSS SQ. FT.					
	STORAGE - (S-2) MEZZANINE SINGLE EXIT CRITERIA OCCUPANT LOAD LESS THAN		100 GROSS SQ. FT. 300 GROSS SQ. FT. 30		TABLE 1003.1	BEEN PREPARE	D DOCUMENT HAS ARED BASED ON	
MEZZANINE					SECTION 1005.7	INFORMATION PROVIDED BY OTHERS. THE A/E HAS NOT		
		RAVEL DISTANCE	100 FEET		1	VERIFIED THE A AND/OR COMPL	ETENESS OF	
	НО	JRLY PART	ITION	RAT	INGS	THIS INFORMAT NOT BE RESPON	VSIBLE FOR ANY	
GRAF			RIPTION		DEFFORME	MAY BE INCORF HEREIN AS A RE	ORATED	
				UM BOARD	(UL U419)	DATE OF SEAL		
		1-HOUR	CMU		(UL U905)	LOF OLAL		
		2-HOUR GYPSUM BOARD CMU		(UL U419) (UL U905)	SHEET CONTENT			
4 - HOUR FIRE WALL SEPAR			RATION			1st FLR. LIFE S 2nd FLR. LIFE	SAFETY PLAN	
						BUILDING CO	DE INFO.	
[[[[	2-HOUR FLOOR ASSEMBLY				(UL D780) DETAIL 11/MA4.41	DATE		
FE PO-			ALL OFFICE SPACE, SURFACE		DETAIL TT/MA4.41	SCALE	1/32"=1'-0"	
(FIRE EXTINGUIS SURFACE MOUNT	ыпшк T)	MOUNT IN MAINTENANCE AREA AND UTILITY ROOMS - REFERENCE ENLARGED FLOOR PLANS FOR LOCATIONS)				DRAWN BY	GMW	
FEC	HER Cabinet -Recessed)					CHECKED BY APPROVED BY	MGC	
MUUNT - SEMI-RECESSED)							mor	
•***		NOST REMOTE EGRESS PATH				PB PROJECT NUMBE		
•	-/					CLIENT PROJECT NU	MBER	
•							<b>A0.10</b>	



	PE CARCOLOGY OF BUILDING BUILD			
	REV DESCRIPTION DATE			
	New Administration, Operation and Maintenance Facility for: <b>PSTA Pinellas Suncoast Transit Authority</b> Scherer Dive Pinellas County, Florida 33771			
	RECORD DOCUMENTS THIS RECORD DOCUMENT HAS BEEN PREPARED BASED ON INFORMATION PROVIDED BY OTHERS. THE A/E HAS NOT VERIFIED THE A/E CAURACY AND/OR COMPLETENESS OF THIS INFORMATION AND SHALL NOT BE RESPONSIBLE FOR ANY ERRORS OR OMISSIONS THAT MAY BE INCORPORATED HEREIN AS A RESULT. DATE OF SEAL			
А В С	SHEET CONTENT 1st FLOOR COMPOSITE PLAN 2nd FLOOR COMPOSITE PLAN DATE SCALE 1/32"=1'-0" DRAWN BY GMW CHECKED BY MGC APPROVED BY MJP			
MAINTENANCE BUILDING KEYPLAN	PB PROJECT NUMBER 15550FFD CLIENT PROJECT NUMBER			







## Maintenance Architectural Drawing MA2.01C- MA3

